

U.S. Department of Energy—Grand Junction, Colorado

Calculation Cover Sheet Calc. No.: MOA-02-05-2007-4-04-00 **Discipline: Geotechnical** No. of Sheets: 6 Doc. No.: X0215800 **Properties** Location: Attachment 5, Volume II, Appendix N Project: Moab UMTRA Project Site: **Crescent Junction Disposal Site, Disposal Cell Design** Feature: Supplemental Geotechnical Properties of Tailings Materials from the Moab Processing Site Sources of Data: Laboratory test results from Shaw E & I, Inc. Boring and Test Pit Logs from the 2005 Geotechnical Investigation (Attachment 5 Vol. I, Appendix I) Sources of Formulae and References: Shaw E & I, Inc, November 7, 2006. Certificate of Analysis, Letter Report to General Engineering Laboratories. Shaw E & I, Inc. February 15, 2007. Certificate of Analysis, Letter Report to General Engineering Laboratories. Golder Associates, April 3, 2006. Results of the Bench Scale Testing Program on Cover Soils and Uranium Mill Tailings from the Moab Tailings Impoundment, Grand County, Utah, Draft Technical Memorandum to S.M. Stoller. Preliminary Calc. Final Calc. X Supersedes Calc. No. Author: Checked by: 5/25/07 31 Mmpo7 Name Date Date 31-07 6-5-Approved by: Name Date Name Date Name Date

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Problem Statement:

Preliminary site selection performed jointly by the U.S. Department of Energy (DOE) and the Contractor has identified a 2,300-acre withdrawal area in the Crescent Flat area just northeast of Crescent Junction, Utah, as a possible site for a final disposal cell for the Moab uranium mill tailings. The proposed disposal cell would cover approximately 250 acres.

Based on the preliminary site-selection process, the suitability of the Crescent Junction Disposal Site is being evaluated from several technical aspects, including geomorphic, geologic, hydrologic, seismic, geochemical, and geotechnical. The objective of this calculation is to present the supplemental test results of geotechnical testing performed to characterize the tailings at the Moab processing site.

These data will be incorporated into Attachment 5, Volume II of the *Remedial Action Plan and Site Design for Stabilization of Moab Title I Uranium Mill Tailings at Crescent Junction, Utah, Site* (RAP) and summarized in the Remedial Action Selection (RAS) report for the Moab Site.

Method of Solution:

Samples were collected during November and December 2005 by mixing either bulk samples obtained from the ten backhoe test pits (600-series, also referred to as the GABT series) completed in early December 2005, or selected Shelby tube and/or split spoon samples from the November/December 2005 geotechnical drilling program (700-series). Figure 1 shows the location where the tailings samples were collected. A description of the discrete samples used to obtain composite samples, along with results from settlement and compaction tests, is contained in Appendix A. Further test results, including Atterberg limits, grain size distribution, hydraulic conductivity, triaxial shear strength, consolidation, and capillary-moisture relationships are presented in Appendix B and Appendix C at the conclusion of this calculation.

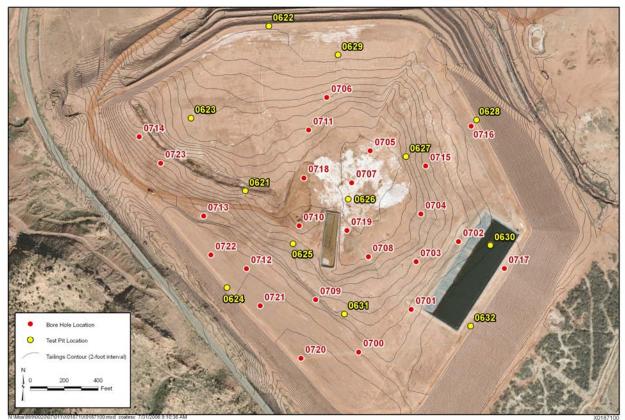


Figure 1. Location of Tailings Samples

Assumptions:

Not applicable

Calculation:

Appendixes A, B, and C contain laboratory results from the supplemental testing, and Table 1 summarizes the test results.

Discussion:

Soil and tailings samples from this sampling effort were sent to Shaw E & I, Inc. Geotechnical Laboratory (hereafter referred to as the laboratory) on May 4, 2006.

The laboratory issued to Stoller Corporation a Certificate of Analysis on November 7, 2006. Upon review of the data, three deficiencies were noted as follows:

- Confined Undrained Triaxial Shear Strength with Pore Pressure Measurements (ASTM D 4767) tests were requested with effective confining pressures of 20, 40, and 60 pounds per square inch (psi). Test results presented in Appendix B show that tests were performed with an effective confining pressure of 2 psi. The laboratory is currently reanalyzing these tests. This calculation set will be updated when these test results are available.
- 2. Hydraulic Conductivity using Flexible Wall Permeameter (ASTM D 5084) tests were requested with effective confining pressures of 20, 40, and 60 psi. Test results presented in Appendix B show that tests were performed with an effective confining pressure of 2 psi. The laboratory is currently reanalyzing these tests. This calculation set will be updated when these test results are available.
- 3. Capillary-Moisture Relationships were analyzed using a potentiometer. These test results show very high vapor suctions, especially at high moisture contents. The laboratory is currently reanalyzing these tests using ASTM D 5298 (Measurement of Soil Potential Suction) and using filter paper. This calculation set will be updated when these test results are available.

Conclusion and Recommendations:

Not applicable

Computer Source:

Not applicable.

Bench		Atterberg Limits			/ses	Sample Prep Dry Unit Weight (pcf)/			xial Shear ength (2)		Volume	Maximum	Optimum Moisture	Cottlad Davaget	
Test Sample No.	Soil Type	(LL/PL/PI) ASTM D 4318	% Gravel	% Sand	% Silt	% Clay	Water Content (%)/Confining Pressure (psi)	Hydraulic Conductivity (1)	c, psf	Eff. Friction Angle, degrees	Coefficient of Consol., Cc	Moisture Content at 15-bar (3)	Dry Density (pcf) (Standard Proctor) (4)	Content (Standard Proctor) (4)	Settled Percent Compaction (4)
							106.3 / 7.0% / 2.5	4.7E-06							
GABT-01	Cover Soil	NP	4	73	18	5	106.3 / 7.0 / 2.5	7.6E-06					117.7	11.9%	82.0%
							106.3 / 7.0 / 2.5	1.1E-05							
GABT-02	Cover Soil	NP	3	80	14	3							109.2	13.8%	85.8%
							90.5 / 14.4 / 2.25	2.7E-04							
GABT-03	Sand Tailings	NP	1	83	15	2	90.5 / 14.4 / 2.26	3.8E-04	0	34.5			106.3	12.7%	79.3%
							90.5 / 14.4 / 2.27	7.9E-05							
							88.2 / 17.5 / 2.25	1.7E-04							
GABT-04	Sand Tailings	NP	0	76	21	3	88.2 / 17.5 / 2.26	1.3E-05	0	36.5	0.15	6.1	103.9	15.6%	82.2%
							88.2 / 17.5 / 2.27	1.8E-05							
							101.7 / 15.3 / 2.25	3.1E-04							
GABT-05	Sand Tailings	NP	3	76	17	5	101.7 / 15.3 / 2.26	2.2E-04	0	38.3			113.3	13.1%	90.9%
							101.7 / 15.3 / 2.27	2.1E-04							
GABT-06	Sand Tailings	NP	1	83	13	4					0.07	24.4	107.3	14.6%	82.6%
	Transition				40	0	96.3 / 20.5 / 2.5	1.2E-05							
	Transition Tailings	31/22/9	31/22/9 1	1	49	42	8	96.3 / 20.5 / 2.6	1.4E-05	0	47.2			107.3	18.4%
	. cgo						96.3 / 20.5 / 2.7	1.3E-05							
							101.4 / 17.9 / 2.25	3.2E-05							
GABT-08	Sand Tailings	NP	7	72	19	9	101.4 / 17.9 / 2.26	2.1E-05	0	37.1			112.8	16.0%	83.3%
							101.4 / 17.9 / 2.27	7.4E-05							
							91.8 / 23.0 / 2.5	6.4E-05							
GABT-09	Transition	23/20/3	0	42	50	8	91.8 / 23.0 / 2.6	6.9E-05	0	36.3	0.20	24.4	102.0	21.1%	87.9%
	Tailings						91.8 / 23.0 / 2.7	7.1E-05							
GABT-10	Transition Tailings	19/17/2	0	70	24	6					0.17	>50.5	107.8	18.7%	94.6%
GABT-11	Slimes Tailings	56/27/29	0	22	53	25					0.38	27.6	96.0	27.8%	68.5%
							83.6 / 20.9 / 2.5	8.4E-05							
GABT-12	Slimes Tailings	35/19/16	0	41	47	12	83.6 / 20.9 / 2.6	2.1E-05	0	50.8			101.6	22.5%	39.3%
	rainigs						83.6 / 20.9 / 2.7	1.9E-05							
GABT-13	Slimes Tailings	49/23/23	0	12	63	25					0.34	25.1	95.0	28.7%	84.9%
				1			81.2 / 22.8 / 3.0	2.7E-06							
GABT-14	Slimes	43/22/21	0	16 6	62	22	81.2 / 22.8 / 3.1	1.8E-06	0	37.6			101.5	20.9%	76.6%
	Tailings		-				81.2 / 22.8 / 3.2	1.8E-06	_	-			-		

(1) Hydraulic Conductivity tests performed at low confining pressures. Tests are currently being reanalyzed at estimated post-construction confining pressures. Data will be presented when available.
 (2) Triaxial shear strength tests performed at low confining pressures. Tests are currently being reanalyzed at estimated post-construction confining pressures. Data will be presented when available.
 (3) Capillary-Moisture Relationships analyzed with WP4 Potentiometer. Tests are currently being reanalyzed using ASTM D 5298 (filter paper method) Data will be presented when available.
 (4) Test results from Golder Associates, Inc. (GAI), 2006. Results to Bench Scale Testing Program on Cover Soils and Uranium Mill Tailings from the Moab Tailings Impoundment, Grand County, Utah, Draft technical memorandum, Lakewood, Colorado, April 3.

End of current text

Appendix A

Results of the Bench Scale Testing Program on Cover Soils and Uranium Mill Tailings from the Moab Tailings Impoundment, Grand County, Utah

April 3, 2006

Note: The attached Technical Memorandum was issued in draft form on April 3, 2006. The memorandum was reviewed with no comments and is considered a final version.

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DRAFT TECHNICAL MEMORANDUM

TO:	Greg Lord, S.M. Stoller	DATE:	April 3, 2006
FR:	James M. Johnson, Golder Associates Inc. Luis A. Quirindongo, Golder Associates Inc. Ron DiDonato, Golder Associates Inc.	OUR REF:	053-2269.2050

RE: RESULTS OF THE BENCH SCALE TESTING PROGRAM ON COVER SOILS AND URANIUM MILL TAILINGS FROM THE MOAB TAILINGS IMPOUNDMENT, GRAND COUNTY, UTAH

INTRODUCTION

As part of our approved work program on the Moab Project, personnel from Golder Associates Inc. (Golder) completed an onsite bench scale testing program during the period of March 15 through March 23, 2006.

The objective of the bench scale testing program was to advance the tailings characterization efforts started during the August 2005 preliminary tailings test program, adjusted to meet the evolving needs and goals of the tailings relocation materials handling evaluation and design.

During this period Golder completed the following tests:

- As sampled moisture content testing of the cover soils and tailings materials after compositing (ASTM D2216) 14 tests;
- Standard Proctor compaction testing (ASTM D698) 14 tests;
- Loose density testing (no ASTM standard) 14 tests; and
- Settled density tests (no ASTM standard) 14 tests.

Each of the 14 samples was created by mixing either bulk samples obtained from the ten backhoe test pits completed in early December 2005, or selected Shelby tube and/or split spoon samples from the November/December 2005 geotechnical drilling and sampling program.

TESTING PROGRAM DESCRIPTION

Sample Preparation

The bench scale testing program was designed to make use of cover soils and tailings samples remaining onsite, exclusive of the samples already selected and shipped for offsite geotechnical laboratory testing. The samples pre-selected for use in this program are listed in the March 14, 2006 Golder letter describing the proposed bench scale testing program. At the start of the sample preparation phase of the work, Golder personnel noted that some of the pre-selected samples were no

Greg Lord		April 3, 2006
S.M. Stoller	-2-	053-2269

longer available onsite or were different material types than originally classified. Therefore, the list of samples to be used in this testing program was modified based on review of the available samples and material types. The final sample list is presented in Table 1.

From March 15 through March 17, Messrs. Luis Quirindongo and Jeff Robison of Golder visited the site to begin the preparation of the samples for the testing program. Each group of samples was mixed to create a composite sample, with splits set aside for Proctor compaction and loose/settled density testing. The sample for Proctor compaction testing was set out to air dry (over the weekend of March 18-19, 2006). The samples for loose and settled density testing were placed either in buckets or bags to preserve the as-sampled moisture content.

Sample Testing

During the sample preparation period, moisture content samples were obtained and tested to determine the initial, or as-sampled moisture content. During the period of March 20 through 23, Messrs. Luis Quirindongo and Ron DiDonato were present on site to perform the standard Proctor compaction and loose/settled density testing. During the course of the week, sample processing, moisture conditioning, and testing were performed. All testing was completed by March 23.

Natural Moisture Content Testing

• Fourteen moisture content tests were performed following ASTM D2216 procedures.

Standard Proctor Compaction Testing

• Fourteen compaction tests were performed following ASTM D698, Method A procedures.

Loose Settled Density Testing

- Fourteen loose settled density tests were performed. These are non-standard tests designed to collect data which we expect to be useful indicators of material characteristics following excavation and during and after transport. ASTM D4253 Standard Method for Maximum Index Density and Unit Weight of Soil Using Vibratory Table and ASTM D4254 Standard Method for Maximum Index Density and Unit Weight of Soil and Calculation of Relative Density were used as guidelines. The procedures developed by Golder for these tests are as follows:
 - For loose density testing, a standard Proctor mold with a known volume was used. Material at its natural moisture content was placed in the mold as loosely as possible, struck off at the top of the mold, and the weight of the mold plus wet soil was recorded.
 - For the settled density test, the same mold and material was used. To prevent any loss of mass and obtain a smoother recording surface, a cap weighing 1.34 pounds was placed on top of the sample before proceeding with testing. To settle the sample, the mold with the sample was dropped on a concrete floor from a height of 1 to 3 inches, 100 times. After every 25 drops, the side of the mold was tapped with a

hammer approximately 16 to 20 times. After shaking the sample, height changes and moisture content were measured and recorded.

TEST RESULTS

During sample preparation and testing, materials were mixed to represent four typical types found at the site: cover soils, sand tailings, transition tailings, and slimes tailings. Golder recommends that leftover sample materials be tested for classification properties in an offsite laboratory to either confirm the visual classification presented in this technical memorandum or provide a basis for modifying the classification. The range of measurements obtained for the four primary material types are presented below.

Natural Moisture Content:

- Cover Soil 6.5%
- Sand Tailings 1.1 to 9.4%
- Transition Tailings 16.5 to 37.0%
- Slimes Tailings 37.5 to 52.3%

Proctor Maximum Dry Density:

- Cover Soil 109.2 to 117.7 pcf
- Sand Tailings 103.9 to 107.3 pcf
- Transition Tailings 102.0 to 113.3 pcf
- Slimes Tailings 95.0 to 101.6 pcf

Proctor Optimum Moisture Content:

- Cover Soil 11.9 to 13.8%
- Sand Tailings 12.7 to 15.6%
- Transition Tailings 13.1 to 21.1%
- Slimes Tailings 20.9 to 28.7%

Loose Wet Density:

- Cover Soil 71.5 to 78.7 pcf
- Sand Tailings 63.7 to 66.8 pcf
- Transition Tailings 71.6 to 103.1 pcf
- Slimes Tailings 49.0 to 93.8 pcf

Loose Dry Density:

- Cover Soil 66.9 to 73.1 pcf
- Sand Tailings 57.7 to 61.7 pcf
- Transition Tailings 53.2 to 83.9 pcf
- Slimes Tailings 33.2 to 64.3 pcf

Settled Wet Density:

- Cover Soil 100.9 to 103.3 pcf
- Sand Tailings 93.1 to 95.9 pcf
- Transition Tailings 107.3 to 126.4 pcf
- Slimes Tailings 58.9 to 114.2 pcf

Settled Dry Density:

- Cover Soil 93.7 to 96.6 pcf
- Sand Tailings 84.3 to 88.6 pcf
- Transition Tailings 84.6 to 102.9 pcf
- Slimes Tailings 39.9 to 80.7 pcf

Percent Vertical Compression (under dynamic loading):

- Cover Soil 22.0 to 30.8%
- Sand Tailings 30.3 to 31.5%
- Transition Tailings 18.4 to 39%
- Slimes Tailings 16.9 to 39.6%

Percent Compaction (Settled Dry Density as a percentage of the standard Proctor maximum dry density):

- Cover Soil 82.0 to 85.8%
- Sand Tailings 79.3 to 82.6%
- Transition Tailings 78.8 to 94.6%
- Slimes Tailings 39.3 to 84.9%

All test results are summarized on Table 2. The bench scale test data sheets are included in Attachment 1.

TABLES

TABLE 1SAMPLE SELECTION MIX SUMMARY

Bench Test Sample No.	Borehole or Test Pit	Depth or Interval	Mixed Description
GABT-01	GATP-05 GATP-11		SAND (SP), little silt, trace gravel, fine grained, reddish brown, cover soil
GABT-02	GATP-08 GATP-09	0-5' 0-5'	SAND (SP), some silt, poorly graded, yellowish brown, cover soil
GABT-03	GATP-04	5-10' 10-15'	SAND (SP), some silt, poorly graded, moist, yellowish brown, sand tailings
GABT-04	GATP-08	15-20' 5-10'	SAND (SP), trace to little silt, trace clay, poorly graded, moist, light brown, sand tailings
GIBTOT	GITT 00	10-15' 15-20	on 12 (or), duce to have shi, duce only, poonly graded, moist, nght orown, sand tanings
GABT-05	GATP-06	5-10' 10-15'	Clayey SAND (SC), poorly graded, saturated, reddish brown, transition tailings
GABT-06	GATP-09	15-20' 5-10' 10-15'	SAND (SP), little to some silt, saturated, reddish brown, sand tailings
GABT-07	GATP-05	15-20' 10-15'	Sandy SILT (ML), moist, reddish brown, transition tailings
GABT-08	GATP-07 GATP-10		Sandy SILT (ML), moist, little to some clay, reddish brown, transition tailings
GABT-09	GATP-11 700	10-15' 12-14' 24-26'	Sandy CLAY (CL), some silt, medium plasticity, grayish brown, moist, transition tailings
		40-42' 42-44' 44-46'	
GABT-10	707	6-8' 8-10' 10-12' 12-14'	Sandy CLAY (CL), some silt, medium plasticity, dark gray, wet, transition tailings
	713	18-20' 20-22' 13.5-15.5'	
GABT-11	703	18-20' 33-35' 38-40' 43-45' 58-60' 63-65' 68-70'	SILT (ML), low plasticity, moist, dark brown, slimes tailings
GABT-12	709	13-15' 23-25' 28-30' 48-50' 53-55' 63-65'	Silty CLAY (CL), medium plasticity, moist, grayish brown, slimes tailings
GABT-13	700		CLAY (CL), low to medium plasticity, moist, grayish brown, slimes tailings
	707	34-36' 50-52' 52-54'	
GABT-14	708	20-22' 30-32' 38-40' 44-46' 59-60' 60-62'	CLAY (CH), high plasticity, moist, dark gray, slimes tailings

TABLE 2 BENCH TESTING RESULTS SUMMARY

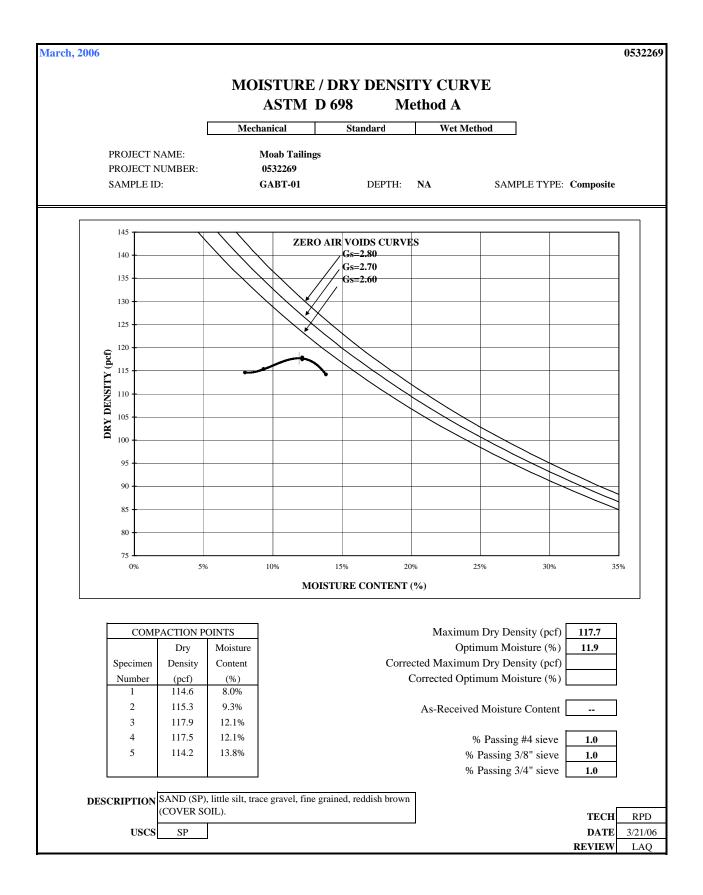
Bench Test Sample No.	Initial Moisture Content (Taken March 17, 2006)	Maximum Dry Density (pcf)	Optimum Moisture Content	Loose Wet Density (pcf)	Loose Dry Density (pcf)	Settled Wet Density (pcf)	Settled Dry Density (pcf)	Vertical Percent Settlement	Settled Percent Compaction	Comments
GABT-01	6.5%	117.7	11.9%	71.5	66.9	103.3	96.6	30.8%	82.0%	Cover Soil
GABT-02	6.5%	109.2	13.8%	78.7	73.1	100.9	93.7	22.0%	85.8%	Cover Soil
GABT-03	9.4%	106.3	12.7%	63.7	57.7	93.1	84.3	31.5%	79.3%	Sand Tailings
GABT-04	1.1%	103.9	15.6%	66.1	59.3	95.1	85.4	30.5%	82.2%	Sand Tailings
GABT-05	19.9%	113.3	13.1%	103.1	83.9	126.4	102.9	18.4%	90.9%	Transition Tailings
GABT-06	5.9%	107.3	14.6%	66.8	61.7	95.9	88.6	30.3%	82.6%	Sand Tailings
GABT-07	35.4%	107.3	18.4%	72.7	53.2	115.7	84.6	37.1%	78.8%	Transition Tailings
GABT-08	16.5%	112.8	16.0%	71.6	62.7	107.3	93.9	33.3%	83.3%	Transition Tailings
GABT-09	37.0%	102.0	21.1%	71.8	54.7	117.6	89.7	39.0%	87.9%	Transition Tailings
GABT-10	30.7%	107.8	18.7%	82.5	66.2	127.0	102.0	35.1%	94.6%	Transition Tailings
GABT-11	49.4%	96.0	27.8%	58.3	40.6	94.5	65.7	38.3%	68.5%	Slimes Tailings
GABT-12	40.8%	101.6	22.5%	49.0	33.2	58.9	39.9	16.9%	39.3%	Slimes Tailings
GABT-13	37.5%	95.0	28.7%	68.9	48.7	114.2	80.7	39.6%	84.9%	Slimes Tailings
GABT-14	52.3%	101.5	20.9%	93.8	64.3	113.4	77.7	17.3%	76.6%	Slimes Tailings

ATTACHMENT 1

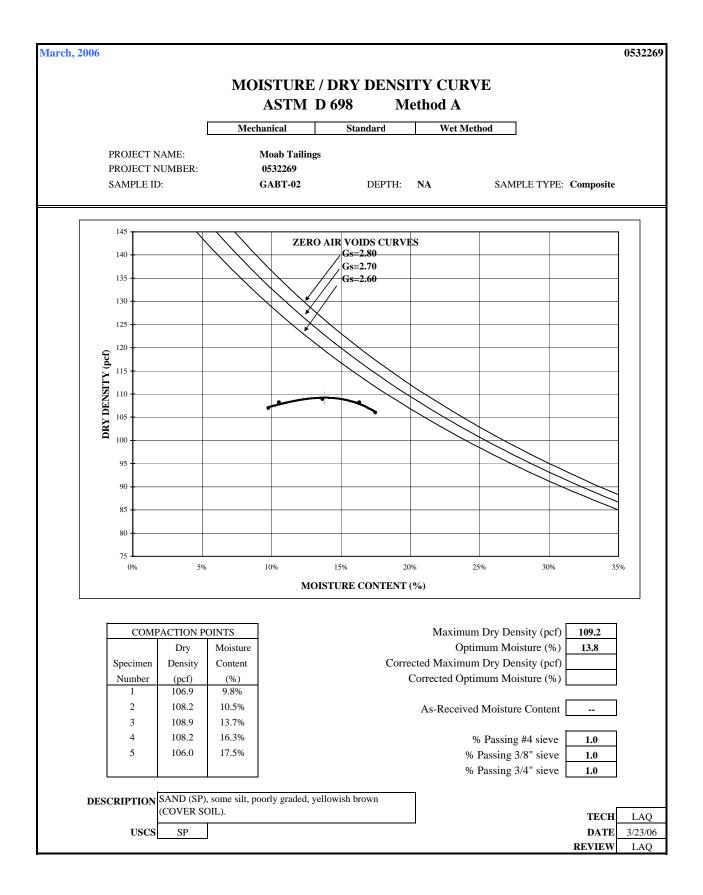
DATA SHEETS

STANDARD PROCTOR COMPACTION TESTING

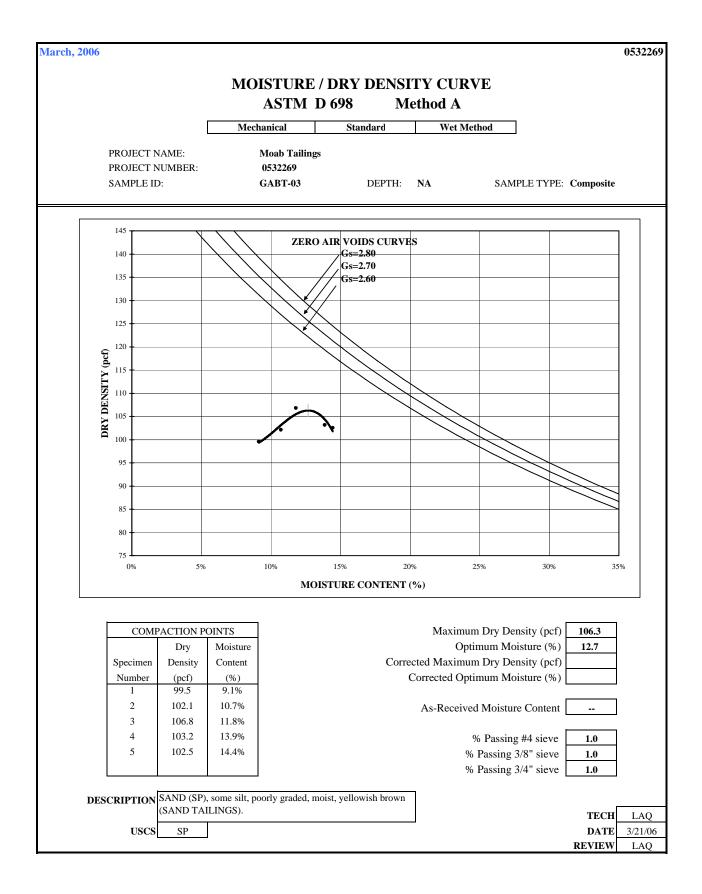
			ASTN	M D 698	& 1557	1			
PROJECT NAME PROJECT NUMBER	М	oab Tailings 0532269]	TEST TYPE	ASTM D 698		Method A	\exists
SAMPLE IDENTITY	GABT-01		NA]					
SAMPLE TYPE	Composite	тур	E COMPAC	TOR F	PREPARATIO	N	Method A.	20% OR LESS RE	TAINED ON #4
Mold Number	217		Mechanical		Wet Method	1	Method A.	2070 OK LESS KE	
Mold Weight (gm)	0.00	I		4		4	Method B:	>20% RETAINED	
Mold Diameter (in.)	4.000		Т	YPE PROCT	OR			20% OR LESS RET	AINED ON 3/8"
Mold Height (in.) Mold Volume (cu.ft.)	4.575 0.0333			Standard	1		Method C:	> 20% RETAINED	ON 3/8" AND
word volume (cumu)	0.0555		5.5 -lbf. R	ammer with 1	2 inch drop		Method C.	< 30% RETAINED	
WATER CONTENT		COARSE FRACTION	TOTAL SAMPLE		Total weigh	t before proce	ssing and per	cent retained	
Wt Tare & Soil Wet	(W1)		36.60			veight, wet (co		0.00	
Wt Tare & Soil Dry	(W2)		34.40	-		veight, dry (co		0.00	
Wt Tare Wt Moisture	(W3) (W4=W1-W2)		0.40 2.20		-	retained on #4 etained on 3/8'		0.00	
Wt Dry Soil	(W5=W2-W3)		34.00		0	etained on 3/4		0.00	
Water Content (dec)	(wc=W4/W5)		0.0647		0	retained on #4		0.00%	
Water Content (%)	(W4/W5)*100		6.47%			etained on 3/8"		0.00%	
					Percent re	etained on 3/4"	' sieve (dry)	0.00%	
POINT RESULTS (FINE)	[1	2	3	4	5	6	7	
Wt. Soil & Mold	(W1)	1867.70	1903.50	1994.80	1987.60	1961.80			
Weight of Mold	(W2)	0.00	0.00	0.00	0.00	0.00			
Wt. of Wet Soil Wet Density, wd (pcf)	(W3=W1-W2) (W3/453.6*Vm)	1867.70 123.76	1903.50 126.13	1994.80 132.18	1987.60 131.70	1961.80 129.99			
wet Density, wu (per)	(113/455.0 111)	125.70	120.15	152.10	151.70	12).))			
WATER CONTENTS	_								
Wt. Tare & Soil Wet	(W4)	310.30	355.20	302.10	391.20	435.20			
Wt. Tare & Soil Dry Wt. Tare	(W5) (W6)	287.80 6.60	325.40 6.60	270.10 6.60	349.60 6.60	383.10 6.70			
Wt. Moisture	(W7=W4-W5)	22.50	29.80	32.00	41.60	52.10			
Wt. Dry Soil	(W8=W5-W6)	281.20	318.80	263.50	343.00	376.40			
	-								
Water Content (%)	(W7/W8)*100	8.00%	9.35%	12.14%	12.13%	13.84%			
Dry Density (pcf)	(wd/(1+wc))	114.6	115.3	117.9	117.5	114.2			
Maximum Dry Density (po	ef)		117.7	DE	SCRIPTION	SAND (SP), li	ittle silt, trace	gravel, fine grained	, reddish brown
Optimum Moisture Conter	. ,		11.9			(COVER SOI	L).		
Corrected Maximum Dry Corrected Optimum Moist				-	USCS	SP			
Corrected Optimum Mois	ture (70)			1	0505	51			
Specific Gravity And Abso		Aggregate - A	STM C 127			-	AS-RECEIVE	D	
Weight of Oven Dry Sampl	ίο γ	A				МО	ISTURE CON	TENT	
Weight of Saturated-Surface Weight of Saturated Sample	• • •	B 2m) C						Ţ	
Absorption of Oversize Par		gm) C (B-A)/A]*100				1			
Bulk Specific Gravity		A/(B-C)]			
AVEDACE ABCODE		1		1					
AVERAGE ABSORPTION AVERAGE BULK SPECI				4				TE	
A VERAGE DULK SPECI	GRAVIII			1				REVI	



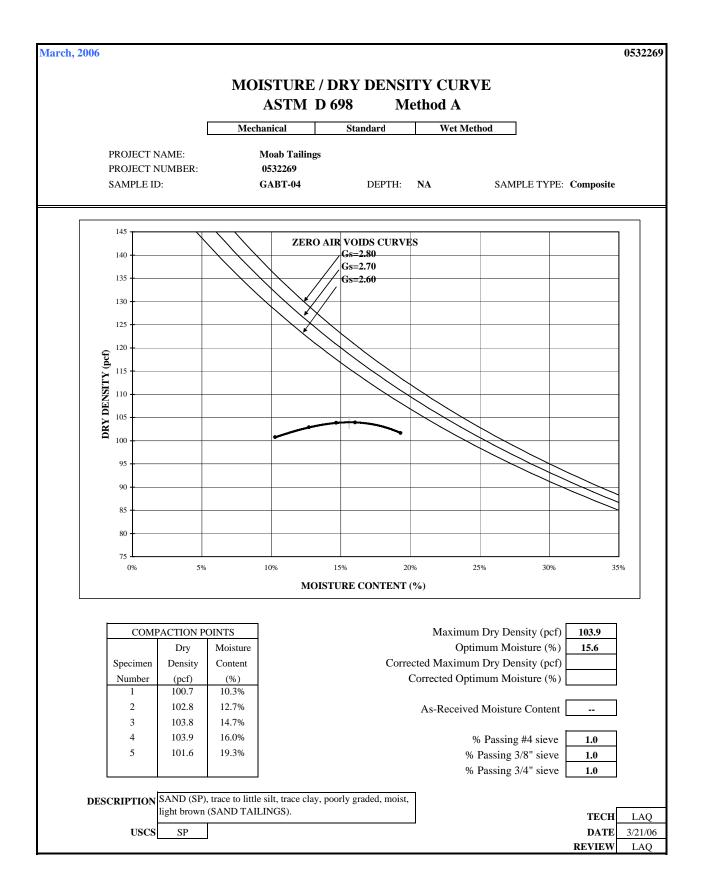
			ASTN	M D 698	8 & 1557	,			
PROJECT NAME	N	Ioab Tailings		1	TEST TYPE	ASTM D 698		Method A	
PROJECT NUMBER		0532269							
SAMPLE IDENTITY	GABT-02		NA						
SAMPLE TYPE	Composite			-					
		ТҮР	E COMPAC	FOR I	PREPARATIC	<u>N</u> N	Method A:	20% OR LESS RE	TAINED ON #4
Mold Number	217		Mechanical		Wet Method	J			
Mold Weight (gm)	0.00						Method B:	> 20% RETAINED	
Mold Diameter (in.)	4.000		Т	YPE PROCT	OR			20% OR LESS RET	AINED ON 3/8"
Mold Height (in.) Mold Volume (cu.ft.)	4.575 0.0333			Standard	1		Made	. 200/ DETAINED	ON 2/811 AND
wiola volume (cu.tt.)	0.0355	·	5.5 -lbf R	ammer with 1	12 inch dron		Method C:	> 20% RETAINED < 30% RETAINED	
					12 men ur op				0113/4
WATER CONTENT		COARSE FRACTION	TOTAL SAMPLE	7		t before proces			
Wt Tare & Soil Wet	(W1)		62.90			veight, wet (coa		0.00	
Wt Tare & Soil Dry	(W2)		59.10	4		veight, dry (coa		0.00	
Wt Tare	(W3)		0.40		-	retained on #4		0.00	
Wt Moisture Wt Dry Soil	(W4=W1-W2) (W5=W2-W3)		3.80	-	0	etained on 3/8"		0.00	
Water Content (dec)	$(w_5=w_2-w_3)$ $(w_c=W_4/W_5)$		58.70 0.0647		0	etained on 3/4'' retained on #4		0.00%	
Water Content (%)	(W4/W5)*100		6.47%	-		etained on 3/8"	• •	0.00%	
Water Content (70)	(((4,(15)) 100		0.4770	1		etained on 3/4"		0.00%	
							,		
POINT RESULTS (FINE)	[1	2	3	4	5	6	7	
Wt. Soil & Mold	(W1)	1771.80	1804.70	1868.40	1899.50	1879.00			
Weight of Mold	(W2)	0.00	0.00	0.00	0.00	0.00			
Wt. of Wet Soil	(W3=W1-W2)	1771.80	1804.70	1868.40	1899.50	1879.00			
Wet Density, wd (pcf)	(W3/453.6*Vm)	117.40	119.58	123.81	125.87	124.51			
WATER CONTENTS									
Wt. Tare & Soil Wet	(W4)	290.50	290.80	333.30	368.10	400.00			
Wt. Tare & Soil Dry	(W5)	265.20	263.70	294.00	317.40	341.50			
Wt. Tare	(W6)	6.60	6.60	6.70	6.70	6.60			
Wt. Moisture	(W7=W4-W5)	25.30	27.10	39.30	50.70	58.50			
Wt. Dry Soil	(W8=W5-W6)	258.60	257.10	287.30	310.70	334.90			
				•					
Water Content (%)	(W7/W8)*100	9.78%	10.54%	13.68%	16.32%	17.47%			
Dry Density (pcf)	(wd/(1+wc))	106.9	108.2	108.9	108.2	106.0			
Maximum Dry Density (po			109.2	DI	ESCRIPTION	S	· 1	ly graded, yellowisl	n brown
Optimum Moisture Conte			13.8			(COVER SOIL	.).		
Corrected Maximum Dry	, ta i				USCO	CD			
Corrected Optimum Mois	ture (%)]	USCS	SP			
Specific Gravity And Abso	rption of Coarse	Aggregate - A	STM C 127				AS-RECEIVE	D	
Weight of Oven Dry Samp	le (gm)	А				-	STURE CON		
Weight of Saturated-Surfa	ίο /	B				іГ		Ţ [.]	
Weight of Saturated Samp	• • •	gm) C		1		1 -		<u> </u>	
Absorption of Oversize Par		[(B-A)/A]*100]			
Bulk Specific Gravity		A/(B-C)]			
				_		-			
AVERAGE ABSORPTION								TE	
AVERAGE BULK SPECI	FIC GRAVITY]				DA	
								REVI	EW LAQ



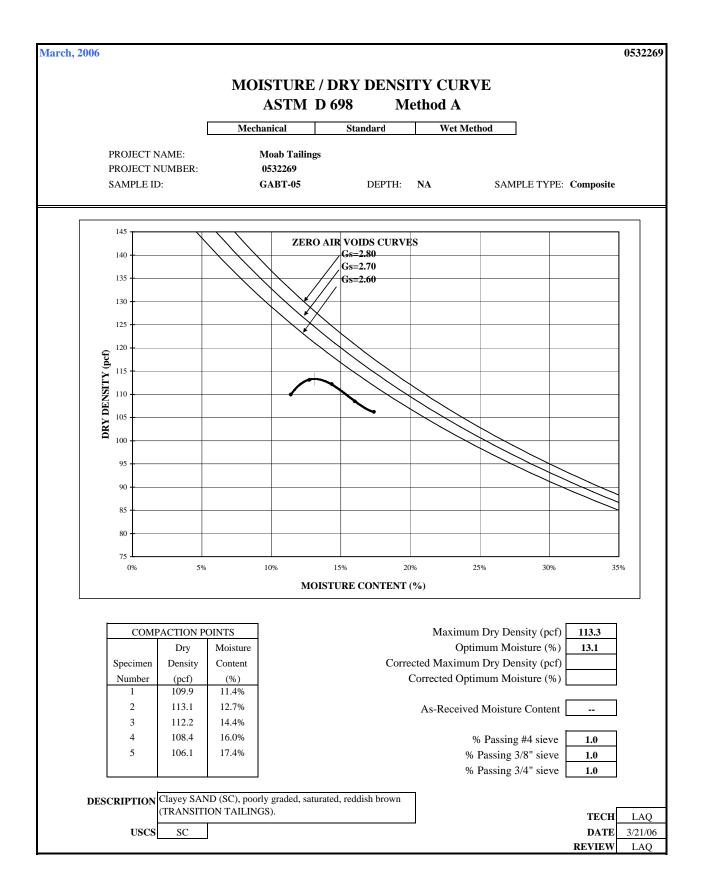
			ASTN	M D 698	8 & 1557	1			
PROJECT NAME	M	loab Tailings		1	TEST TYPE	ASTM D 698		Method A	
PROJECT NUMBER		0532269							
SAMPLE IDENTITY	GABT-03		NA						
SAMPLE TYPE	Composite			•		•			
		ТҮР	E COMPAC	FOR I	PREPARATIC	DN	Method A:	20% OR LESS RE	TAINED ON #4
Mold Number	217		Mechanical		Wet Method				
Mold Weight (gm)	0.00						Method B:	> 20% RETAINED	
Mold Diameter (in.)	4.000		Т	YPE PROCT	OR			20% OR LESS RET	TAINED ON 3/8"
Mold Height (in.)	4.575			Standard	<u> </u>				
Mold Volume (cu.ft.)	0.0333						Method C:	> 20% RETAINEI	
			5.5 -101. R	ammer with 1	12 inch drop			< 30% RETAINEI) ON 3/4"
WATER CONTENT		COARSE FRACTION	TOTAL SAMPLE	-	Total weigh	t before proces	ssing and per	cent retained	
Wt Tare & Soil Wet	(W1)		42.30		Total v	weight, wet (co	arse & fine)	0.00	
Wt Tare & Soil Dry	(W2)		38.70	4		veight, dry (coa		0.00	
Wt Tare	(W3)		0.40	-	-	retained on #4		0.00	
Wt Moisture	(W4=W1-W2)		3.60	-	0	etained on 3/8"		0.00	
Wt Dry Soil	(W5=W2-W3)		38.30	-	0	etained on 3/4"	. ,	0.00	
Water Content (dec)	(wc=W4/W5)		0.0940	-		retained on #4 etained on 3/8''		0.00%	
Water Content (%)	(W4/W5)*100		9.40%	J		etained on 3/8		0.00%	
				•					
POINT RESULTS (FINE)	l	1	2	3	4	5	6	7	
Wt. Soil & Mold	(W1)	1639.30	1705.60	1802.10	1772.60	1770.10			
Weight of Mold	(W2)	0.00	0.00	0.00	0.00	0.00			
Wt. of Wet Soil	(W3=W1-W2)	1639.30	1705.60	1802.10	1772.60	1770.10			
Wet Density, wd (pcf)	(W3/453.6*Vm)	108.62	113.02	119.41	117.46	117.29			
WATER CONTENTS									
Wt. Tare & Soil Wet	(W4)	245.90	274.40	236.90	252.10	260.30			
Wt. Tare & Soil Dry	(W5)	225.90	248.50	212.60	222.20	228.30			
Wt. Tare	(W6)	6.60	6.60	6.70	6.60	6.60			
Wt. Moisture	(W7=W4-W5)	20.00	25.90	24.30	29.90	32.00			
Wt. Dry Soil	(W8=W5-W6)	219.30	241.90	205.90	215.60	221.70			
Water Content (%)	(W7/W8)*100	9.12%	10.71%	11.80%	13.87%	14.43%			
Dry Density (pcf)	(wd/(1+wc))	99.5	102.1	106.8	103.2	102.5			
Maximum Dry Density (po	cf)		106.3	DI	ESCRIPTION	SAND (SP), so	ome silt, poor	ly graded, moist, y	ellowish brown
Optimum Moisture Conte	nt (%)		12.7			(SAND TAILI	INGS).		
Corrected Maximum Dry	y Density (pcf)								
Corrected Optimum Mois	ture (%)				USCS	SP			
Specific Gravity And Abso	rption of Coarse	Aggregate - A	STM C 127				AS DECEIVE		
Weight of Oven Dry Samp	le (am)	Α		1		7	AS-RECEIVE		
Weight of Saturated-Surfa	ιų ,	A B				мо			
Weight of Saturated Samp	• • • •	gm) C				4 L		T	
Absorption of Oversize Par		(B-A)/A]*100				1			
-			L	1	<u> </u>	י ר			
Bulk Specific Gravity		A/(B-C)			1	1			
AVERAGE ABSORPTION]				TE	CH LAQ
AVERAGE BULK SPECI	FIC GRAVITY]					TE 3/21/06
								REVI	EW LAQ



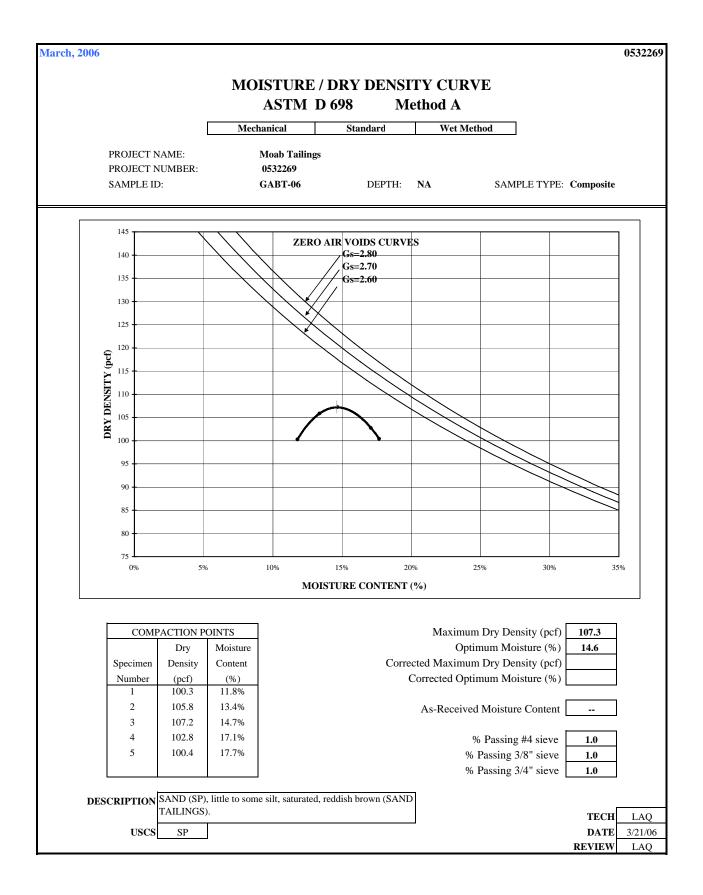
			ASTN	M D 698	& 1557	1			
PROJECT NAME PROJECT NUMBER	М	oab Tailings 0532269]	TEST TYPE	ASTM D 698		Method A	\exists
SAMPLE IDENTITY	GABT-04		NA]					
SAMPLE TYPE	Composite	тур	E COMPAC	TOR F	PREPARATIC	N	Method A.	20% OR LESS RET	AINED ON #4
Mold Number	217	111	Mechanical		Wet Method		Methou A.	20 % OK LESS KET	AINED ON #4
Mold Weight (gm)	0.00			4		4	Method B:	>20% RETAINED	ON #4 AND
Mold Diameter (in.)	4.000		Т	YPE PROCT	OR			20% OR LESS RETA	AINED ON 3/8"
Mold Height (in.) Mold Volume (cu.ft.)	4.575 0.0333			Standard	<u>]</u>		Mathad Ci	> 20% RETAINED	ON 2/8" AND
word vorume (cu.rt.)	0.0555		5.5 -lbf. R	ammer with 1	2 inch drop		Method C.	< 30% RETAINED	
					<u>^</u>				
WATER CONTENT		COARSE FRACTION	TOTAL SAMPLE		Total weigh	t before proce	essing and per	ccent retained	
Wt Tare & Soil Wet	(W1)		45.50]	Total v	veight, wet (co	arse & fine)	0.00	
Wt Tare & Soil Dry	(W2)		45.00			veight, dry (co		0.00	
Wt Tare Wt Moisture	(W3) (W4=W1-W2)		0.40 0.50		-	retained on # etained on 3/8		0.00	
Wt Dry Soil	(W5=W2-W3)		44.60	-	0	etained on 3/4		0.00	
Water Content (dec)	(wc=W4/W5)		0.0112		0	retained on #4	. ,	0.00%	
Water Content (%)	(W4/W5)*100		1.12%	1	Percent re	etained on 3/8	' sieve (dry)	0.00%	
					Percent re	etained on 3/4	' sieve (dry)	0.00%	
POINT RESULTS (FINE)	[1	2	3	4	5	6	7	
Wt. Soil & Mold	(W1)	1676.00	1749.40	1797.00	1819.60	1830.00			
Weight of Mold	(W2)	0.00	0.00	0.00	0.00	0.00			
Wt. of Wet Soil	(W3=W1-W2)	1676.00	1749.40	1797.00	1819.60	1830.00			
Wet Density, wd (pcf)	(W3/453.6*Vm)	111.06	115.92	119.07	120.57	121.26			
WATER CONTENTS									
Wt. Tare & Soil Wet	(W4)	295.50	248.00	254.40	353.50	264.50			
Wt. Tare & Soil Dry	(W5)	268.60	220.80	222.70	305.60	222.80			
Wt. Tare Wt. Moisture	(W6) (W7=W4-W5)	6.60 26.90	6.80	6.70 21.70	6.70 47.90	6.80 41.70			
Wt. Dry Soil	(W7 = W4 - W5) (W8 = W5 - W6)	26.90	27.20 214.00	31.70 216.00	298.90	216.00			
wu biy son		202.00	21 1100	210.00	270.70	210.00			
Water Content (%)	(W7/W8)*100	10.27%	12.71%	14.68%	16.03%	19.31%			
Dry Density (pcf)	(wd/(1+wc))	100.7	102.8	103.8	103.9	101.6			
Maximum Dry Density (po Optimum Moisture Conter	nt (%)		103.9 15.6	DE	SCRIPTION		race to little si SAND TAILIN	ilt, trace clay, poorly NGS).	graded, moist,
Corrected Maximum Dry Corrected Optimum Moist				4	USCS	SP			
corrected optimum mois	uite (70)			1	0505	51	1		
Specific Gravity And Absor	•		STM C 127	1	1	1	AS-RECEIVE		
Weight of Oven Dry Sampl Weight of Saturated-Surface	.0	A B			 	МС	DISTURE CONT	FENT	
Weight of Saturated-Surface Weight of Saturated Sample	• • •	gm) C			+	4		Т	
Absorption of Oversize Par	۰ <u>د</u>	(B-A)/A]*100			+	1			
Bulk Specific Gravity	(- / /	A/(B-C)]			
AVERAGE ABSORPTION	J	I		1				TEC	H IAO
AVERAGE ABSORPTION AVERAGE BULK SPECII				4				DAT	_
				1				REVIE	



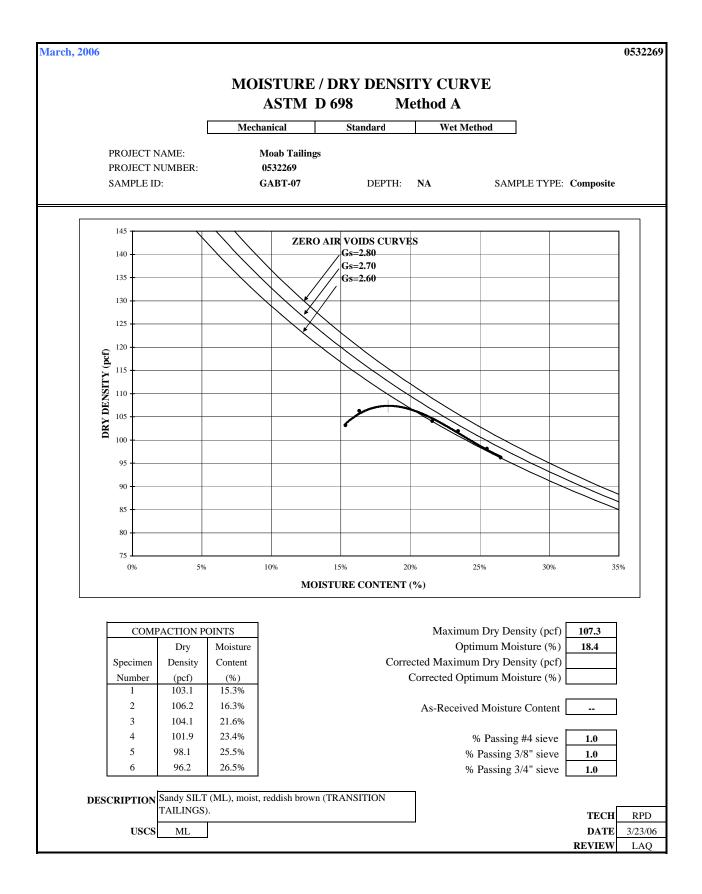
			ASTN	M D 698	& 1557	1			
PROJECT NAME PROJECT NUMBER	М	oab Tailings 0532269]	TEST TYPE	ASTM D 698		Method A	\exists
SAMPLE IDENTITY	GABT-05		NA]					
SAMPLE TYPE	Composite	тур	E COMPAC	ΓΩΡ Ι	PREPARATIC	N	Mathad A.	20% OR LESS RET	AINED ON #4
Mold Number	217	111	Mechanical		Wet Method		Methou A.	20 % OK LESS KE I	AINED ON #4
Mold Weight (gm)	0.00	ļ		4		4	Method B:	> 20% RETAINED	ON #4 AND
Mold Diameter (in.)	4.000		Т	YPE PROCT	OR			20% OR LESS RET	AINED ON 3/8"
Mold Height (in.) Mold Volume (cu.ft.)	4.575 0.0333			Standard	1		Mathad C:	> 20% RETAINED	ON 2/8" AND
word volume (cu.n.)	0.0555		5.5 -lbf. R	ammer with 1	2 inch drop		Method C.	< 30% RETAINED	
					<u>^</u>				
WATER CONTENT		COARSE FRACTION	TOTAL SAMPLE		Total weigh	t before proce	ssing and per	cent retained	
Wt Tare & Soil Wet	(W1)		73.80	-		weight, wet (co		0.00	
Wt Tare & Soil Dry	(W2)		61.60	-		veight, dry (co		0.00	
Wt Tare Wt Moisture	(W3) (W4=W1-W2)		0.40 12.20		-	retained on # tained on 3/8		0.00	
Wt Dry Soil	(W5=W2-W3)		61.20		0	etained on 3/4		0.00	
Water Content (dec)	(wc=W4/W5)		0.1993		0	retained on #4		0.00%	
Water Content (%)	(W4/W5)*100		19.93%			etained on 3/8'		0.00%	
					Percent re	etained on 3/4'	' sieve (dry)	0.00%	
POINT RESULTS (FINE)	[1	2	3	4	5	6	7	
Wt. Soil & Mold	(W1)	1847.20	1923.60	1935.80	1898.20	1880.30			
Weight of Mold	(W2)	0.00	0.00	0.00	0.00	0.00			
Wt. of Wet Soil Wet Density, wd (pcf)	(W3=W1-W2) (W3/453.6*Vm)	1847.20 122.40	1923.60 127.46	1935.80 128.27	1898.20 125.78	1880.30 124.59			
wer bensity, wu (per)	(113/45510 111)	122.40	127.40	120.27	125.70	124.57			
WATER CONTENTS	-					-	-		
Wt. Tare & Soil Wet	(W4)	285.30	291.90	312.40	272.10	272.00			
Wt. Tare & Soil Dry Wt. Tare	(W5) (W6)	256.80 6.60	259.70 6.60	274.20 8.10	235.70 8.10	232.70 6.60			
Wt. Moisture	(W7=W4-W5)	28.50	32.20	38.20	36.40	39.30			
Wt. Dry Soil	(W8=W5-W6)	250.20	253.10	266.10	227.60	226.10			
	-			I	I	I	I		
Water Content (%)	(W7/W8)*100	11.39% 109.9	12.72%	14.36%	15.99%	17.38%			
Dry Density (pcf)	(wd/(1+wc))	109.9	113.1	112.2	108.4	106.1			
Maximum Dry Density (po			113.3	DE	SCRIPTION			graded, saturated, re	ddish brown
Optimum Moisture Conte Corrected Maximum Dry			13.1	-		(TRANSITIO	N TAILINGS).	
Corrected Optimum Mois					USCS	SC			
_		I		•					
Specific Gravity And Abso	rption of Coarse	Aggregate - A	STM C 127				AS DECENT		
Weight of Oven Dry Samp	le (gm)	Α				мо	AS-RECEIVE		
Weight of Saturated-Surfa	.0	B				1410		1	
Weight of Saturated Samp	le in Water (g	gm) C]		-	
Absorption of Oversize Par	rticles (%) [(B-A)/A]*100				J			
Bulk Specific Gravity		A/(B-C)]			
AVERAGE ABSORPTION	N	1	[1				TEC	H LAQ
AVERAGE ADSORPTION AVERAGE BULK SPECI				1				DAT	_
				4				REVIE	



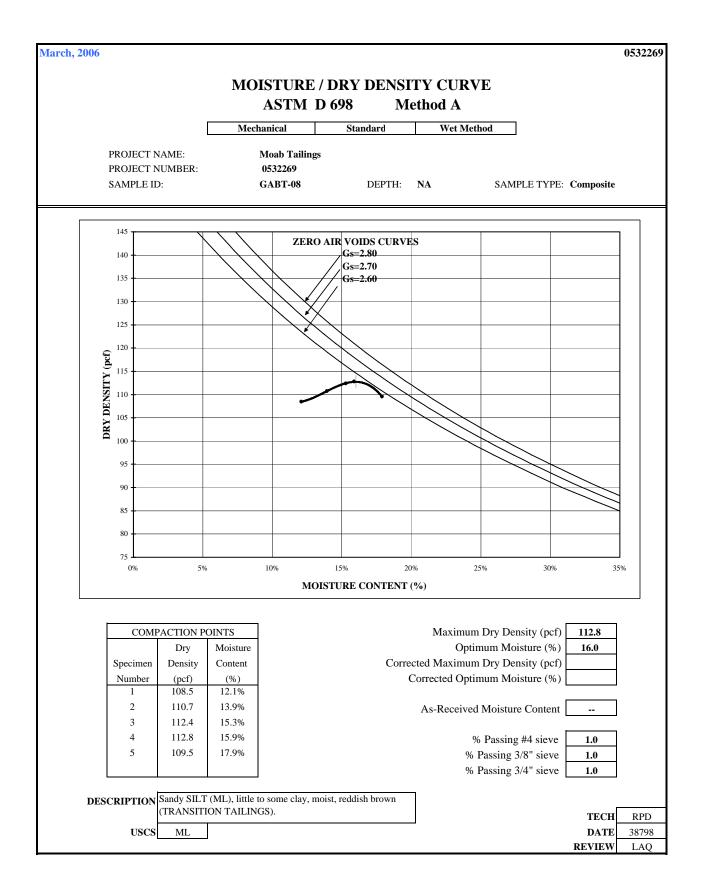
			AST	TM D 69	8 & 155	7				
PROJECT NAME		Ioab Tailings		1	TEST TYPE	ASTM D 698		Method A		
PROJECT NUMBER		0532269			1201 1112					
SAMPLE IDENTITY	GABT-06		NA	-						
SAMPLE TYPE	Composite			4		J				
		ТҮР	E COMPAC	FOR F	REPARATIC	DN	Method A:	20% OR LESS	RETAINED O	N #4
Mold Number	217		Mechanical]	Wet Method]				
Mold Weight (gm)	0.00	I		4		4	Method B:	> 20% RETAIN	ED ON #4 AN	D
Mold Diameter (in.)	4.000		Т	YPE PROCT	OR			20% OR LESS	RETAINED O	N 3/8''
Mold Height (in.)	4.575			Standard	T					
Mold Volume (cu.ft.)	0.0333				1		Method C:	> 20% RETAIN	ED ON 3/8" A	ND
			5.5 -lbf. R	ammer with 1	2 inch drop			< 30% RETAIN	ED ON 3/4"	
		60 L D 67	TOTAL							
WATER CONTENT		COARSE	TOTAL		Total weigh	t before proce	essing and per	rcent retained		
		FRACTION	SAMPLE	1	T ()	• • • • • • •	0 6			
Wt Tare & Soil Wet	(W1)		56.10	4		weight, wet (co		0.00		
Wt Tare & Soil Dry	(W2)		53.00	-		veight, dry (co		0.00		
Wt Tare	(W3)		0.40	4	0	retained on #		0.00		
Wt Moisture	(W4=W1-W2)		3.10	4	0	etained on 3/8		0.00		
Wt Dry Soil	(W5=W2-W3)		52.60	4	0	etained on 3/4		0.00		
Water Content (dec)	(wc=W4/W5)		0.0589	_		retained on #		0.00%		
Water Content (%)	(W4/W5)*100		5.89%]		etained on 3/8		0.00%		
					Percent re	etained on 3/4	sieve (dry)	0.00%		
POINT RESULTS (FINE		1	2	3	4	5	6	7		
Wt. Soil & Mold	(W1)	1691.40	1811.30	1855.20	1815.50	1783.30				
Weight of Mold	(W2)	0.00	0.00	0.00	0.00	0.00				
Wt. of Wet Soil	(W3=W1-W2)	1691.40	1811.30	1855.20	1815.50	1783.30				
Wet Density, wd (pcf)	(W3/453.6*Vm)	112.08	120.02	122.93	120.30	118.17				
WATER CONTENTS										
Wt. Tare & Soil Wet	(W4)	253.40	303.00	379.80	361.20	392.50				
Wt. Tare & Soil Dry	(W5)	227.40	268.00	332.00	309.50	334.50				
Wt. Tare	(W6)	6.70	6.60	6.50	6.70	6.60				
Wt. Moisture	(W7=W4-W5)	26.00	35.00	47.80	51.70	58.00				
Wt. Dry Soil	(W8=W5-W6)	220.70	261.40	325.50	302.80	327.90				
				I			T	· · · · · ·		
Water Content (%)	(W7/W8)*100	11.78%	13.39%	14.69%	17.07%	17.69%		┥───┤		
Dry Density (pcf)	(wd/(1+wc))	100.3	105.8	107.2	102.8	100.4				
				1	GODIDITION					(1.1.) TD
Maximum Dry Density (107.3 14.6	DF	SCRIPTION		little to some s	ilt, saturated, red	idish brown (SAND
Optimum Moisture Cont Corrected Maximum D			14.0	_		TAILINGS).				
Corrected Optimum Moi	• • •				USCS	SP				
Specific Gravity And Abs	orption of Coarse	Aggregate - A	STM C 127							
Weight of Oyen Dr. C.				1	1	٦ . <i></i>	AS-RECEIVED			
Weight of Oven Dry Sam		A				м	DISTURE CONT			
Weight of Saturated-Surf	• • •	B C				-		1		
Weight of Saturated Sam		gm) C				-				
Absorption of Oversize Pa	articles (%)	[(B-A)/A]*100		1		J				
Bulk Specific Gravity		A/(B-C)				ו				
r v				•		-				
AVERAGE ABSORPTIC				4					TECH	LAQ
AVERAGE BULK SPEC	IFIC GRAVITY			J					DATE REVIEW	3/21/06 LAQ
										~~~~~



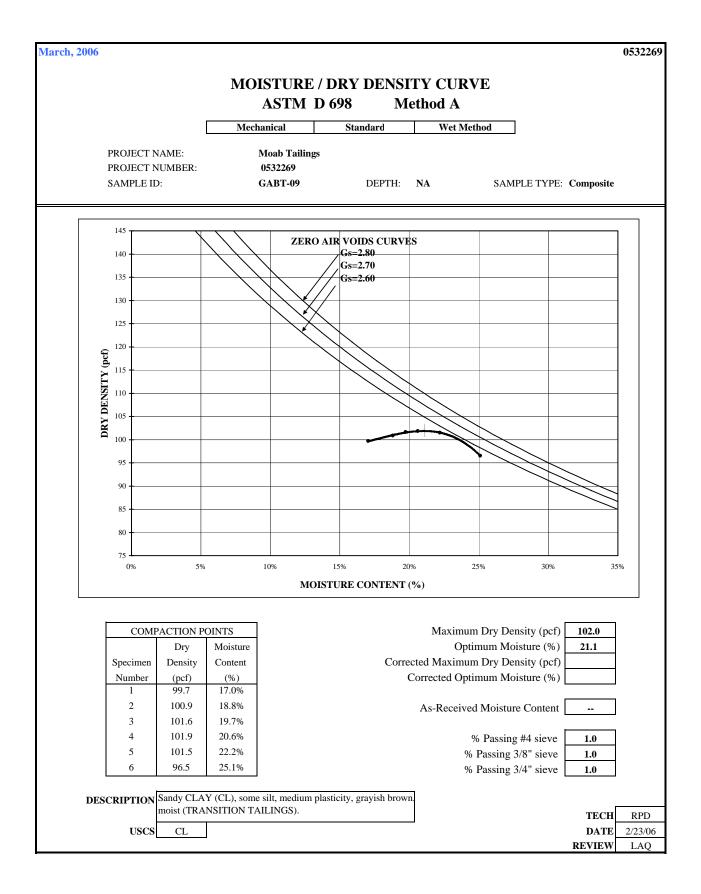
			ASTN	M D 698	8 & 1557	1			
PROJECT NAME PROJECT NUMBER		loab Tailings 0532269		]	TEST TYPE	ASTM D 698		Method A	
SAMPLE IDENTITY SAMPLE TYPE	GABT-07 Composite	ТҮР	NA E COMPAC	I FOR F	PREPARATIC	DN	Method A:	20% OR LESS RETAI	NED ON #4
Mold Number Mold Weight (gm)	217 0.00		Mechanical	]	Wet Method	]	Method B:	> 20% RETAINED ON	
Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	4.000 4.575 0.0333		Т	YPE PROCT Standard			Method C:	20% OR LESS RETAIN > 20% RETAINED ON	
			5.5 -lbf. R	ammer with 1	2 inch drop			< 30% RETAINED ON	3/4''
WATER CONTENT		COARSE FRACTION	TOTAL SAMPLE	-	Total weigh	t before proce	essing and per	rcent retained	
Wt Tare & Soil Wet Wt Tare & Soil Dry	(W1) (W2)		56.60 41.90	-		veight, wet (co veight, dry (co		0.00	
Wt Tare	(W3)		0.40		-	retained on #		0.00	
Wt Moisture Wt Dry Soil	(W4=W1-W2) (W5=W2-W3)		14.70 41.50	-	0	etained on 3/8 etained on 3/4		0.00	
Water Content (dec)	(wc=W4/W5)		0.3542		0	retained on #	. ,	0.00%	
Water Content (%)	(W4/W5)*100		35.42%			etained on 3/8		0.00%	
					Percent re	etained on 3/4	" sieve (dry)	0.00%	
POINT RESULTS (FINE)	[	1	2	3	4	5	6	7	
Wt. Soil & Mold	(W1)	1794.70	1864.60	1909.10	1898.50	1858.50	1836.10		
Weight of Mold Wt. of Wet Soil	(W2) (W3=W1-W2)	0.00 1794.70	0.00 1864.60	0.00 1909.10	0.00 1898.50	0.00 1858.50	0.00 1836.10		
Wet Density, wd (pcf)	(W3=W1-W2) (W3/453.6*Vm)	118.92	123.55	1909.10	125.80	123.15	121.67		
	· · · ·			•		•	•	<u> </u>	
WATER CONTENTS Wt. Tare & Soil Wet	(W4)	330.10	279.60	369.50	368.40	384.40	409.50		
Wt. Tare & Soil Dry	(W5)	287.10	241.30	305.10	299.70	307.60	325.10		
Wt. Tare	(W6)	6.60	6.70	6.60	6.60	6.60	6.60		
Wt. Moisture	(W7=W4-W5)	43.00	38.30	64.40	68.70	76.80	84.40		
Wt. Dry Soil	(W8=W5-W6)	280.50	234.60	298.50	293.10	301.00	318.50		
Water Content (%)	(W7/W8)*100	15.33%	16.33%	21.57%	23.44%	25.51%	26.50%		
Dry Density (pcf)	(wd/(1+wc))	103.1	106.2	104.1	101.9	98.1	96.2		
Maximum Dry Density (po			107.3	DE	SCRIPTION		ML), moist, re	eddish brown (TRANSI	TION
Optimum Moisture Conter Corrected Maximum Dry			18.4	4		TAILINGS).			
Corrected Optimum Mois					USCS	ML			
Specific Gravity And Abso	rption of Coarse	Aggregate - A	STM C 127	-			AS-RECEIVE	D	
Weight of Oven Dry Sampl Weight of Saturated-Surfac Weight of Saturated Sampl Absorption of Oversize Par	ce-Dry (gm) le in Water (j	A B gm) C ((B-A)/A]*100				МС	AS-RECEIVE		
Bulk Specific Gravity		(B-A)/A] 100 A/(B-C)				]			
AVERAGE ABSORPTION	N	1		1				ТЕСН	RPD
AVERAGE ABSORPTION AVERAGE BULK SPECI				]				DATE REVIEW	3/23/06 LAQ



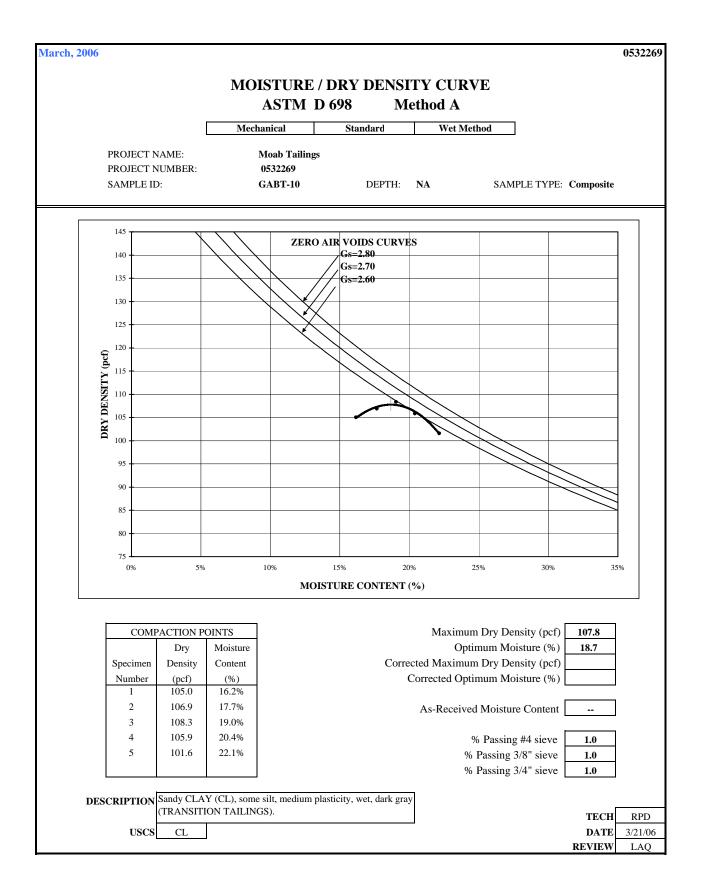
ASTM D 698 & 1557											
PROJECT NAME	Moab Tailings			TEST TYPE ASTM D 698			Method A				
PROJECT NUMBER		0532269									
SAMPLE IDENTITY	GABT-08		NA	1							
SAMPLE TYPE	Composite			•							
		ТҮР	E COMPAC	FOR I	PREPARATIC	ON Method A	: 20% OR LESS RETAIN	NED ON #4			
Mold Number	217		Mechanical		Wet Method						
Mold Weight (gm)	0.00					Method B	: > 20% RETAINED ON	#4 AND			
Mold Diameter (in.)	4.000		Т	YPE PROCT	OR		20% OR LESS RETAIN	ED ON 3/8"			
Mold Height (in.)	4.575			Standard	1						
Mold Volume (cu.ft.)	0.0333	_	55 H.C D.		(2 to a boli down	Method C	: > 20% RETAINED ON				
5.5 -lbf. Rammer with 12 inch drop <30% RETAINED ON 3/4"											
WATER CONTENT		COARSE FRACTION	TOTAL SAMPLE	-		t before processing and p					
Wt Tare & Soil Wet	(W1)		53.50	-		weight, wet (coarse & fine					
Wt Tare & Soil Dry	(W2)		46.00	-		veight, dry (coarse & fine)					
Wt Tare	(W3)		0.40	-	-	retained on #4 sieve (wet)					
Wt Moisture	(W4=W1-W2)		7.50		0	etained on 3/8" sieve (wet)					
Wt Dry Soil Water Content (dec)	(W5=W2-W3)		45.60 0.1645	-	0	etained on 3/4" sieve (wet) retained on #4 sieve (dry)					
Water Content (dec)	(wc=W4/W5) (W4/W5)*100		16.45%			etained on 3/8" sieve (dry)					
water Content (76)	(****/***5)*100		10.43%	1		etained on 3/4" sieve (dry)					
					T Crecilit Te	cumed on 5/4 sieve (dry)	0.0076				
POINT RESULTS (FINE)	I	1	2	3	4	5 6	7				
Wt. Soil & Mold	(W1)	1834.70	1904.00	1955.40	1971.90	1948.90					
Weight of Mold	(W2)	0.00	0.00	0.00	0.00	0.00					
Wt. of Wet Soil	(W3=W1-W2)	1834.70	1904.00	1955.40	1971.90	1948.90					
Wet Density, wd (pcf)	(W3/453.6*Vm)	121.57	126.16	129.57	130.66	129.14					
WATER CONTENTS											
Wt. Tare & Soil Wet	(W4)	262.40	274.80	395.00	286.10	462.60					
Wt. Tare & Soil Dry	(W5)	234.80	242.00	343.50	247.80	393.40					
Wt. Tare	(W6)	6.60	6.40	6.60	6.60	6.60					
Wt. Moisture	(W7=W4-W5)	27.60	32.80	51.50	38.30	69.20					
Wt. Dry Soil	(W8=W5-W6)	228.20	235.60	336.90	241.20	386.80					
				-							
Water Content (%)	(W7/W8)*100	12.09%	13.92%	15.29%	15.88%	17.89%					
Dry Density (pcf)	(wd/(1+wc))	108.5	110.7	112.4	112.8	109.5					
Maximum Dry Density (p	cf)		112.8	DI	ESCRIPTION	Sandy SILT (ML), little to	some clay, moist, reddish	ı brown			
Optimum Moisture Conte	nt (%)		16.0			(TRANSITION TAILING	· · · · · · · · · · · · · · · · · · ·				
Corrected Maximum Dry	y Density (pcf)										
<b>Corrected Optimum Mois</b>	ture (%)				USCS	ML					
Specific Gravity And Absorption of Coarse Aggregate - ASTM C 127											
		1		r		AS-RECEIV					
Weight of Oven Dry Sample (gm) A						NTENT					
Weight of Saturated-Surface-Dry (gm) B											
Weight of Saturated Sample in Water     (gm)     C       Absorption of Oversize Particles (%)     [(B-A)/A]*100											
Absorption of Oversize Par	rucies (%)	[(B-A)/A]*100				J					
Bulk Specific Gravity		A/(B-C)				]					
	T	1		1				DPT			
AVERAGE ABSORPTION AVERAGE BULK SPECI				4			TECH	RPD			
AVERAGE BULK SPECI	FIU GKAVII Y			1			DATE REVIEW	3/22/06 LAQ			



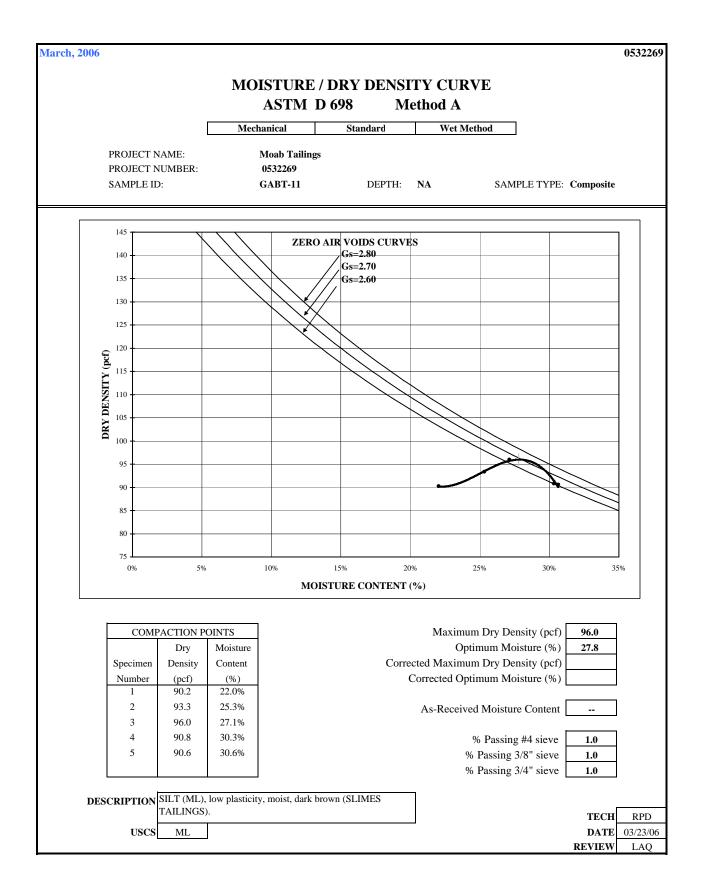
ASTM D 698 & 1557											
PROJECT NAME	Moab Tailings			TEST TYPE ASTM D 698			Method A				
PROJECT NUMBER		0532269									
SAMPLE IDENTITY	GABT-09		NA								
SAMPLE TYPE	Composite			-							
		ТҮР	E COMPAC	FOR I	REPARATIC	DN	Method A:	20% OR LESS RET	AINED ON #4		
Mold Number	217		Mechanical		Wet Method						
Mold Weight (gm)	0.00						Method B:	> 20% RETAINED	ON #4 AND		
Mold Diameter (in.)	4.000		Т	YPE PROCT	OR			20% OR LESS RETA	INED ON 3/8"		
Mold Height (in.)	4.575			Standard	<u> </u>						
Mold Volume (cu.ft.)	0.0333	·	<b>55</b> 0 6 D		<u></u>		Method C:	> 20% RETAINED			
5.5 -lbf. Rammer with 12 inch drop <30% RETAINED ON 3/4"											
WATER CONTENT		COARSE     TOTAL     Total weight before processing and percent retained       FRACTION     SAMPLE									
Wt Tare & Soil Wet	(W1)		39.30			veight, wet (co		0.00			
Wt Tare & Soil Dry	(W2)		28.80	1		veight, dry (co		0.00			
Wt Tare	(W3)		0.40		-	retained on #		0.00			
Wt Moisture	(W4=W1-W2)		10.50	-	0	etained on 3/8		0.00			
Wt Dry Soil	(W5=W2-W3)		28.40	-	0	etained on 3/4	. ,	0.00			
Water Content (dec)	(wc=W4/W5)		0.3697 36.97%	-		retained on #	•	0.00%			
Water Content (%)	(W4/W5)*100		30.97%	J		etained on 3/8' etained on 3/4'		0.00%			
					I ertent re	taineu on 5/4	sieve (ury)	0.00%			
POINT RESULTS (FINE)	[	1	2	3	4	5	6	7			
Wt. Soil & Mold	(W1)	1760.40	1808.40	1836.80	1853.90	1871.10	1821.90				
Weight of Mold	(W2)	0.00	0.00	0.00	0.00	0.00	0.00				
Wt. of Wet Soil	(W3=W1-W2)	1760.40	1808.40	1836.80	1853.90	1871.10	1821.90				
Wet Density, wd (pcf)	(W3/453.6*Vm)	116.65	119.83	121.71	122.84	123.98	120.72				
WATER CONTENTS											
Wt. Tare & Soil Wet	(W4)	328.20	287.60	451.70	399.20	357.80	409.00				
Wt. Tare & Soil Dry	(W5)	281.60	243.10	378.30	332.10	294.00	328.30				
Wt. Tare	(W6)	8.10	6.60	6.50	6.50	6.50	6.80				
Wt. Moisture	(W7=W4-W5)	46.60	44.50	73.40	67.10	63.80	80.70				
Wt. Dry Soil	(W8=W5-W6)	273.50	236.50	371.80	325.60	287.50	321.50				
Water Content (%)	(W7/W8)*100	17.04%	18.82%	19.74%	20.61%	22.19%	25.10%				
Dry Density (pcf)	(wd/(1+wc))	99.7	100.9	101.6	101.9	101.5	96.5				
Maximum Dry Density (pe	,		102.0	DF	SCRIPTION			ilt, medium plasticity	grayish brown		
Optimum Moisture Conte			21.1	-		moist (TRAN	SITION TAIL	LINGS).			
Corrected Maximum Dry				4			r				
Corrected Optimum Mois	ture (%)			]	USCS	CL	]				
Specific Gravity And Absorption of Coarse Aggregate - ASTM C 127 AS-RECEIVED											
Weight of Oven Dry Sample (gm) A				MOISTURE CON							
Weight of Saturated-Surface-Dry (gm) B				İ		1		T			
Weight of Saturated Samp	le in Water (	gm) C				]		-			
Absorption of Oversize Par	rticles (%)	[(B-A)/A]*100				]					
Bulk Specific Gravity		A/(B-C)				]					
		I		_		-					
AVERAGE ABSORPTION								TEC			
AVERAGE BULK SPECI	FIC GRAVITY							DAT			
								REVIE	W LAQ		



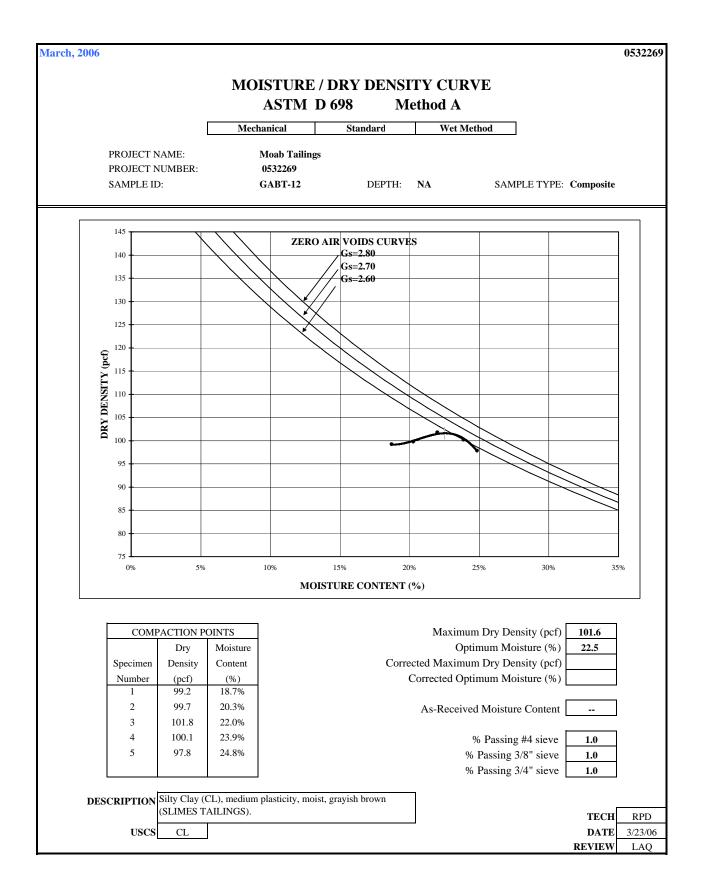
ASTM D 698 & 1557									
PROJECT NAME	N	Ioab Tailings		1	TEST TYPE	ASTM D 698		Method A	
PROJECT NUMBER		0532269			1201 1112			Method /1	
SAMPLE IDENTITY	GABT-10		NA						
SAMPLE TYPE	Composite			•					
		ТҮР	E COMPAC	FOR I	PREPARATIC	<b>DN</b>	Method A:	20% OR LESS RE	TAINED ON #4
Mold Number	217		Mechanical		Wet Method				
Mold Weight (gm)	0.00						Method B:	> 20% RETAINED	O ON #4 AND
Mold Diameter (in.)	4.000		Т	YPE PROCT	OR			20% OR LESS RET	TAINED ON 3/8"
Mold Height (in.)	4.575			Standard	1				
Mold Volume (cu.ft.)	0.0333		<b>55</b> 0 6 D				Method C:	> 20% RETAINED	
			5.5 -IDI. K	ammer with 1				< 30% RETAINEI	J UN 3/4**
WATER CONTENT		COARSE FRACTION	TOTAL SAMPLE	-	Total weigh	t before process	ing and per	cent retained	
Wt Tare & Soil Wet	(W1)		48.50			veight, wet (coar		0.00	
Wt Tare & Soil Dry	(W2)		37.20	-		veight, dry (coar		0.00	
Wt Tare	(W3)		0.40	-	-	retained on #4 s		0.00	
Wt Moisture	(W4=W1-W2)		11.30	-	0	etained on 3/8" s		0.00	
Wt Dry Soil Water Content (dec)	(W5=W2-W3)		36.80 0.3071	-	0	etained on 3/4" s retained on #4 s	. ,	0.00	
Water Content (dec)	(wc=W4/W5) (W4/W5)*100		30.71%			retained on #4 s	• • /	0.00%	
water Content (76)	(**4/**5)*100		50.71%	1					
Percent retained on 3/4" sieve (dry) 0.00%									
POINT RESULTS (FINE)	I	1	2	3	4	5	6	7	
Wt. Soil & Mold	(W1)	1841.40	1897.30	1945.90	1923.40	1872.10			
Weight of Mold	(W2)	0.00	0.00	0.00	0.00	0.00			
Wt. of Wet Soil	(W3=W1-W2)	1841.40	1897.30	1945.90	1923.40	1872.10			
Wet Density, wd (pcf)	(W3/453.6*Vm)	122.02	125.72	128.94	127.45	124.05			
WATER CONTENTS									
Wt. Tare & Soil Wet	(W4)	374.50	379.20	392.60	295.20	373.90			
Wt. Tare & Soil Dry	(W5)	323.30	323.30	330.90	246.30	307.30			
Wt. Tare	(W6)	6.70	6.70	6.80	6.60	6.50			
Wt. Moisture	(W7=W4-W5)	51.20	55.90	61.70	48.90	66.60			
Wt. Dry Soil	(W8=W5-W6)	316.60	316.60	324.10	239.70	300.80			
Water Content (%)	(W7/W8)*100	16.17%	17.66%	19.04%	20.40%	22.14%			
Dry Density (pcf)	(wd/(1+wc))	105.0	106.9	108.3	105.9	101.6			
Maximum Dry Density (po	cf)		107.8	DI	ESCRIPTION	Sandy CLAY (C	CL), some si	lt, medium plastici	ty, wet, dark gray
<b>Optimum Moisture Conte</b>	nt (%)		18.7			(TRANSITION	TAILINGS	).	
Corrected Maximum Dry									
Corrected Optimum Mois	ture (%)			J	USCS	CL			
Specific Gravity And Abso	rption of Coarse	Aggregate - A	STM C 127						
Weight of Oven Dry Samp	le (am)	Α		1		-	S-RECEIVEI TURE CONT		
Weight of Saturated-Surfa		A B				MOIS			
Weight of Saturated Samp	• • •	gm) C		1		┨ └─		L	
Absorption of Oversize Par		[(B-A)/A]*100				1			
Bulk Specific Gravity		A/(B-C)		·	1	1			
Burk Specific Gravity		A(D-C)			1	J			
AVERAGE ABSORPTION	N			1				TE	CH RPD
AVERAGE BULK SPECI				1					<b>TE</b> 3/21/06
				•				REVI	



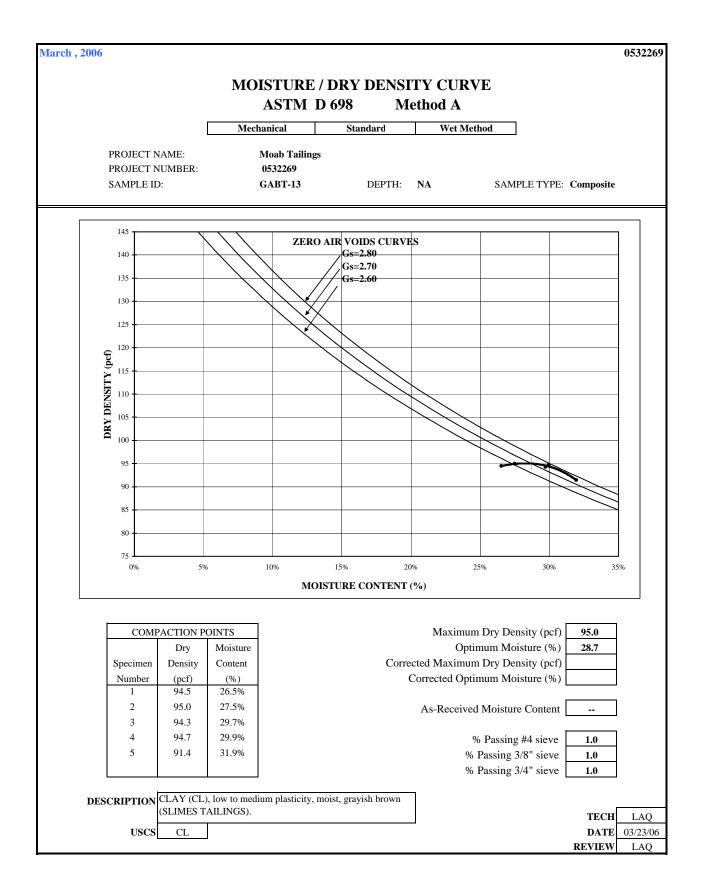
ASTM D 698 & 1557									
PROJECT NAME PROJECT NUMBER	M	oab Tailings 0532269		]	TEST TYPE	ASTM D 698		Method A	
SAMPLE IDENTITY SAMPLE TYPE	GABT-11 Composite		NA						
Mold Number Mold Weight (gm)	217 0.00	ТҮР	E COMPACT Mechanical		PREPARATIC Wet Method	) ]		20% OR LESS RETA > 20% RETAINED C	
Mold Diameter (in.) Mold Height (in.)	4.000 4.575		Т	YPE PROCT Standard	or T		Method B.	20% OR LESS RETA	
Mold Volume (cu.ft.)	0.0333		5.5 -lbf. Ra	ammer with 1	2 inch drop		Method C:	> 20% RETAINED C < 30% RETAINED C	
WATER CONTENT		COARSE	TOTAL SAMPLE		Total weigh	t before proce	essing and per	rcent retained	
Wt Tare & Soil Wet	(W1)		52.70	]		veight, wet (co		0.00	
Wt Tare & Soil Dry Wt Tare	(W2) (W3)		35.40 0.40	-		veight, dry (co retained on #4		0.00	
Wt Moisture	(W4=W1-W2)		17.30		Weight re	tained on 3/8	" sieve (wet)	0.00	
Wt Dry Soil Water Content (dec)	(W5=W2-W3) (wc=W4/W5)		35.00 0.4943	-	0	tained on 3/4' retained on #4	. ,	0.00	
Water Content (dec)	(W2=W4/W5) (W4/W5)*100		49.43%			tained on 3/8		0.00%	
				1	Percent re	tained on 3/4'	' sieve (dry)	0.00%	
POINT RESULTS (FINE)		1	2	3	4	5	6	7	
Wt. Soil & Mold	(W1)	1661.90	1765.00	1841.40	1785.60	1785.40			
Weight of Mold Wt. of Wet Soil	(W2) (W3=W1-W2)	0.00 1661.90	0.00 1765.00	0.00 1841.40	0.00 1785.60	0.00 1785.40			
Wet Density, wd (pcf)	(W3/453.6*Vm)	110.12	116.95	122.02	118.32	118.31			
WATER CONTENTS									
Wt. Tare & Soil Wet	(W4)	462.20	390.60	316.30	404.10	333.20			
Wt. Tare & Soil Dry	(W5)	379.90	313.00	250.20	311.60	256.60			
Wt. Tare	(W6)	6.60	6.60	6.60	6.60	6.60			
Wt. Moisture Wt. Dry Soil	(W7=W4-W5) (W8=W5-W6)	82.30 373.30	77.60 306.40	66.10 243.60	92.50 305.00	76.60 250.00			
wt. Dry Son	(118-113-110)	373.30	500.40	243.00	305.00	230.00			
Water Content (%)	(W7/W8)*100	22.05%	25.33%	27.13%	30.33%	30.64%			
Dry Density (pcf)	(wd/(1+wc))	90.2	93.3	96.0	90.8	90.6			
				1					
Maximum Dry Density (pc Optimum Moisture Conter			96.0 27.8	DE	SCRIPTION	SILT (ML), lo TAILINGS).	ow plasticity, i	moist, dark brown (SI	IMES
Corrected Maximum Dry	. ,		21.0			TAILINGS).			
Corrected Optimum Moist				]	USCS	ML			
Specific Gravity And Absor	rption of Coarse A	Aggregate - A	STM C 127				AS-RECEIVE	D	
Weight of Oven Dry Sampl	le (gm)	Α				МО	ISTURE CON		
Weight of Saturated-Surfac	• • •	В						]	
Weight of Saturated Sampl Absorption of Oversize Par	.0	m) C B-A)/A]*100							
Bulk Specific Gravity	(/0) [(	A/(B-C)				]			
						1			
AVERAGE ABSORPTION				-				TEC	
AVERAGE BULK SPECII	TIC GRAVITY			1				DAT REVIEV	



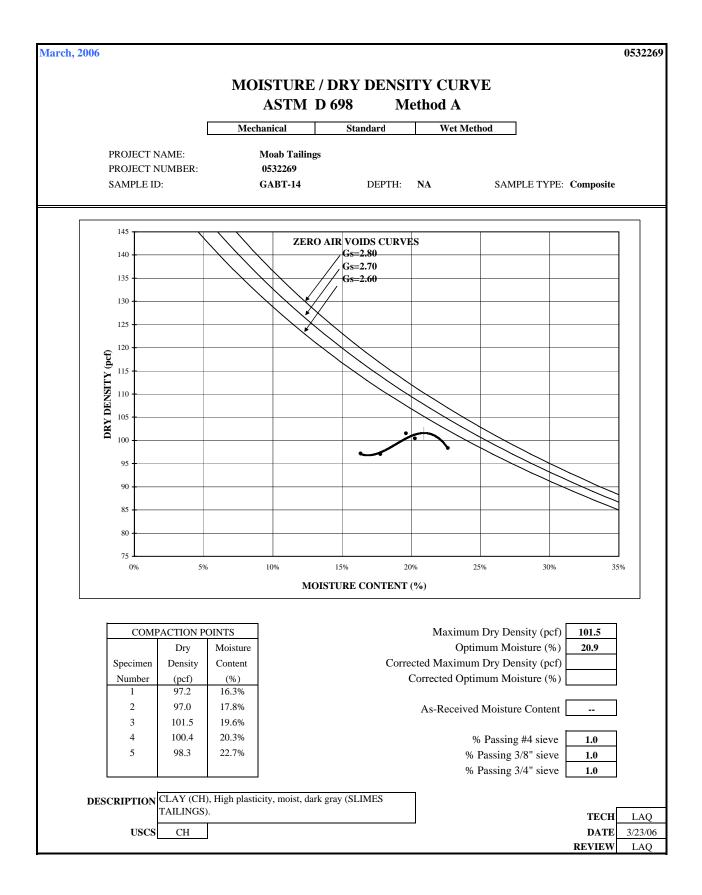
ASTM D 698 & 1557									
PROJECT NAME PROJECT NUMBER		oab Tailings 0532269			TEST TYPE	ASTM D 698		Method A	
SAMPLE IDENTITY SAMPLE TYPE	GABT-12 Composite	TYP	NA E COMPAC'	FOR I	PREPARATIC	DN	Method A:	20% OR LESS RET	AINED ON #4
Mold Number Mold Weight (gm)	217 0.00	[	Mechanical	]	Wet Method	]	Method B:	> 20% RETAINED	
Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	4.000 4.575 0.0333		Т	YPE PROCT Standard	or ]		Method C:	20% OR LESS RETA > 20% RETAINED	
			5.5 -lbf. R	ammer with 1	2 inch drop			< 30% RETAINED	
WATER CONTENT		COARSE FRACTION	TOTAL SAMPLE		Total weigh	t before proce	essing and per	ccent retained	
Wt Tare & Soil Wet	(W1)		47.30	-		veight, wet (co		0.00	
Wt Tare & Soil Dry Wt Tare	(W2) (W3)		33.70 0.40			veight, dry (co retained on #		0.00	
Wt Moisture	(W4=W1-W2)		13.60		-	etained on 3/8		0.00	
Wt Dry Soil	(W5=W2-W3)		33.30		Weight re	etained on 3/4	" sieve (wet)	0.00	
Water Content (dec)	(wc=W4/W5)		0.4084			retained on #		0.00%	
Water Content (%)	(W4/W5)*100		40.84%			etained on 3/8		0.00%	
					Percent re	etained on 3/4	' sieve (dry)	0.00%	
POINT RESULTS (FINE)		1	2	3	4	5	6	7	
Wt. Soil & Mold	(W1)	1777.40	1810.50	1873.90	1872.10	1843.20			
Weight of Mold	(W2)	0.00	0.00	0.00	0.00	0.00			
Wt. of Wet Soil Wet Density, wd (pcf)	(W3=W1-W2) (W3/453.6*Vm)	1777.40 117.78	1810.50 119.97	1873.90 124.17	1872.10 124.05	1843.20 122.14			
wet Density, wu (per)	(**********	117.76	11).)7	124.17	124.05	122.14			
WATER CONTENTS									
Wt. Tare & Soil Wet	(W4)	289.60	287.80	286.20	293.50	403.60			
Wt. Tare & Soil Dry	(W5)	245.00	240.40	235.80	238.20	324.60			
Wt. Tare	(W6)	6.70	6.60	6.60	6.60	6.60			
Wt. Moisture Wt. Dry Soil	(W7=W4-W5) (W8=W5-W6)	44.60 238.30	47.40 233.80	50.40 229.20	55.30 231.60	79.00 318.00		<u> </u>	
wt. Dry Son	(***********	238.30	255.80	229.20	251.00	518.00			
Water Content (%)	(W7/W8)*100	18.72%	20.27%	21.99%	23.88%	24.84%			
Dry Density (pcf)	(wd/(1+wc))	99.2	99.7	101.8	100.1	97.8			
Maximum Dry Density (po Optimum Moisture Conte	nt (%)		101.6 22.5	DF	SCRIPTION	Silty Clay (Cl (SLIMES TA		asticity, moist, grayis	h brown
Corrected Maximum Dry Corrected Optimum Mois				]	USCS	CL	]		
Specific Gravity And Abso	orption of Coarse A	.ggregate - A	STM C 127				AS-RECEIVE	D	
Weight of Oven Dry Samp	le (gm)	Α				MC	ISTURE CON		
Weight of Saturated-Surfa	• · • · ·	В				]		]	
Weight of Saturated Samp	.0	m) C				4			
Absorption of Oversize Particles (%) [(B-A)/A]*100									
Bulk Specific Gravity		A/(B-C)				]			
AVERAGE ABSORPTION	N	]		1				TEC	H RPD
AVERAGE BULK SPECI	FIC GRAVITY			]				DAT REVIE	



ASTM D 698 & 1557									
PROJECT NAME PROJECT NUMBER	М	oab Tailings 0532269		]	TEST TYPE	ASTM D 698		Method A	7
SAMPLE IDENTITY	GABT-13		NA						
SAMPLE TYPE	Composite	TVD	E COMPAC		PREPARATIO	N	Mathod A.	200/ OD I ESS DET	AINED ON #4
Mold Number	217	111	Mechanical		Wet Method		Method A:	20% OR LESS RET	AINED ON #4
Mold Weight (gm)	0.00	I		1		1	Method B:	> 20% RETAINED	ON #4 AND
Mold Diameter (in.)	4.000		Т	YPE PROCT	OR			20% OR LESS RETA	INED ON 3/8"
Mold Height (in.) Mold Volume (cu.ft.)	4.575 0.0333			Standard	<u> </u>		Mathad Cu	> 200/ DETAINED	NN 2/8" A ND
wiola volume (cu.n.)	0.0333		5.5 -lbf. R	ammer with 1	2 inch drop		Method C:	> 20% RETAINED < 30% RETAINED	
					F				
WATER CONTENT		COARSE FRACTION	TOTAL SAMPLE		Total weigh	t before proce	ssing and per	cent retained	
Wt Tare & Soil Wet	(W1)		46.60		Total v	veight, wet (co	arse & fine)	0.00	
Wt Tare & Soil Dry	(W2)		34.00			veight, dry (co		0.00	
Wt Tare Wt Moisture	(W3) (W4=W1-W2)		0.40		-	retained on #4 tained on 3/8'		0.00 0.00	
Wt Dry Soil	(W5=W2-W3)		12.60 33.60		0	tained on 3/8		0.00	
Water Content (dec)	(wc=W4/W5)		0.3750		0	retained on #4	. ,	0.00%	
Water Content (%)	(W4/W5)*100		37.50%		Percent re	tained on 3/8"	sieve (dry)	0.00%	
					Percent re	tained on 3/4"	sieve (dry)	0.00%	
POINT RESULTS (FINE)	[	1	2	3	4	5	6	7	
Wt. Soil & Mold	(W1)	1804.20	1827.40	1845.30	1857.20	1820.60			
Weight of Mold	(W2)	0.00	0.00	0.00	0.00	0.00			
Wt. of Wet Soil Wet Density, wd (pcf)	(W3=W1-W2) (W3/453.6*Vm)	1804.20 119.55	1827.40 121.09	1845.30 122.27	1857.20 123.06	1820.60 120.64			
wet Delisity, wu (per)	(₩3/433.0* ₩Ш)	119.55	121.09	122.27	125.00	120.04			
WATER CONTENTS	_								
Wt. Tare & Soil Wet	(W4)	269.40	252.00	314.80	287.00	275.60			
Wt. Tare & Soil Dry	(W5)	214.30	199.10	244.20	222.40	210.50			
Wt. Tare Wt. Moisture	(W6) (W7=W4-W5)	6.70 55.10	6.70 52.90	6.70 70.60	6.70 64.60	6.70 65.10			
Wt. Dry Soil	(W8=W5-W6)	207.60	192.40	237.50	215.70	203.80			
·				1		1			
Water Content (%)	(W7/W8)*100	26.54%	27.49%	29.73%	29.95%	31.94%			
Dry Density (pcf)	(wd/(1+wc))	94.5	95.0	94.3	94.7	91.4			
Maximum Dry Density (po			95.0	DF	ESCRIPTION			n plasticity, moist, gr	ayish brown
Optimum Moisture Conte			28.7			(SLIMES TAI	ILINGS).		
Corrected Maximum Dry Corrected Optimum Mois					USCS	CL			
Corrected Optimum Mois	ture (70)			1	0505	CL			
Specific Gravity And Abso			STM C 127				AS-RECEIVE	D	
Weight of Oven Dry Samp	ίο γ	A				MO	ISTURE CON	TENT	
Weight of Saturated-Surfa Weight of Saturated Samp	• • •	B 2m) C						T	
Weight of Saturated Sample in Water     (gm)     C       Absorption of Oversize Particles (%)     [(B-A)/A]*100									
Bulk Specific Gravity		A/(B-C)				]			
AVEDACE ABCORDETON	NT .	I		1					
AVERAGE ABSORPTION AVERAGE BULK SPECI								TEC DA1	_
A , ERAGE BULK SI EUL				1				REVIE	



ASTM D 698 & 1557									
PROJECT NAME	N	Ioab Tailings		1	TEST TYPE	ASTM D 698		Method A	
PROJECT NUMBER		0532269							
SAMPLE IDENTITY	GABT-14		NA						
SAMPLE TYPE	Composite			-					
		TYP	E COMPAC	TOR I	PREPARATIC	DN	Method A:	20% OR LESS RE	TAINED ON #4
Mold Number	217		Mechanical		Wet Method				
Mold Weight (gm)	0.00						Method B:	> 20% RETAINED	
Mold Diameter (in.)	4.000		Т	YPE PROCT	OR			20% OR LESS RE	TAINED ON 3/8"
Mold Height (in.)	4.575			Standard	<u> </u>				
Mold Volume (cu.ft.)	0.0333	_					Method C:	> 20% RETAINED	
			5.5 -101. R	ammer with 1	2 inch drop			< 30% RETAINE	D ON 3/4"
WATER CONTENT		COARSE FRACTION	TOTAL SAMPLE	-	Total weigh	t before proce	ssing and per	cent retained	
Wt Tare & Soil Wet	(W1)		68.80		Total v	veight, wet (co	arse & fine)	0.00	
Wt Tare & Soil Dry	(W2)		45.30	1		veight, dry (co		0.00	
Wt Tare	(W3)		0.40		-	retained on #4		0.00	
Wt Moisture	(W4=W1-W2)		23.50		0	etained on 3/8"		0.00	
Wt Dry Soil	(W5=W2-W3)		44.90	-	0	etained on 3/4"		0.00	
Water Content (dec)	(wc=W4/W5)		0.5234	_		retained on #4 etained on 3/8''		0.00%	
Water Content (%)	(W4/W5)*100		52.34%	J		etained on 3/8		0.00%	
					I ertent re	tameu on 3/4	sieve (ury)	0.00%	
POINT RESULTS (FINE)		1	2	3	4	5	6	7	
Wt. Soil & Mold	(W1)	1706.30	1724.10	1832.90	1822.50	1819.80			
Weight of Mold	(W2)	0.00	0.00	0.00	0.00	0.00			
Wt. of Wet Soil	(W3=W1-W2)	1706.30	1724.10	1832.90	1822.50	1819.80			
Wet Density, wd (pcf)	(W3/453.6*Vm)	113.06	114.24	121.45	120.76	120.59			
WATER CONTENTS									
Wt. Tare & Soil Wet	(W4)	356.50	285.60	232.80	414.70	418.70			
Wt. Tare & Soil Dry	(W5)	307.40	243.50	195.70	345.90	342.60			
Wt. Tare	(W6)	6.80	6.70	6.60	6.70	6.80			
Wt. Moisture	(W7=W4-W5)	49.10	42.10	37.10	68.80	76.10			
Wt. Dry Soil	(W8=W5-W6)	300.60	236.80	189.10	339.20	335.80			
-				•					
Water Content (%)	(W7/W8)*100	16.33%	17.78%	19.62%	20.28%	22.66%			
Dry Density (pcf)	(wd/(1+wc))	97.2	97.0	101.5	100.4	98.3			
Maximum Dry Density (p	cf)		101.5	DE	ESCRIPTION	CLAY (CH), I	High plasticity	y, moist, dark gray	(SLIMES
<b>Optimum Moisture Conte</b>	ent (%)		20.9			TAILINGS).			
Corrected Maximum Dry	y Density (pcf)								
Corrected Optimum Mois	ture (%)			J	USCS	CH			
Specific Gravity And Abso	orption of Coarse	Aggregate - A	STM C 127					_	
	• • •			1	1	-	AS-RECEIVE		
Weight of Oven Dry Samp	ιų ,	A				MO	STURE CON	T	
Weight of Saturated-Surfa	• • •	B C				4 l		T	
Weight of Saturated Samp		gm) C				4			
Absorption of Oversize Particles (%) [(B-A)/A]*100									
Bulk Specific Gravity		A/(B-C)				]			
				-					
AVERAGE ABSORPTIO				4					CH LAQ
AVERAGE BULK SPECI	FIC GRAVITY			J				D/ REVI	ATE         3/23/06           EW         LAQ



LOOSE AND SETTLED DENSITY TESTING

LOOSE & SETTLED DENSITY							
PROJECT NAME PROJECT NUMBER SAMPLE IDENTITY SAMPLE TYPE	Moab Tailings 053-2269 GABT-01 N/A Composite						
Mold Number Mold Weight (lb) Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	A3       9.572       4.000       4.573       0.0333	WATER CONTENT Wt Tare & Soil Wet Wt Tare & Soil Dry Wt Tare Wt Moisture Wt Moisture Wt Dry Soil Water Content (dec) Water Content (%)	(W1) (W2) (W3) (W4=W1-W2) (W5=W2-W3) (wc=W4/W5) (W4/W5)*100	488.90 457.60 6.80 31.30 450.80 0.0694 6.94%			
LOOSE DENSITY Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	11.950 2.378 71.51 66.86						
SETTLE DENSITY A verage Settlement (in.) Settled Height (in.) Settlement Volume (cu.ft.) Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	1.406           3.167           0.0230           11.950           2.378           103.26           96.56	Settleme	nt (in.): 1.375 1.438 1.406 1.375 1.469 1.375				
Loose Wet Density (pcf) Loose Dry Density (pcf) Settled Wet Density (pcf) Settled Dry Density (pcf) Vertical Percent Settlement Percent Compaction	71.506 66.864 103.263 96.558 30.75% 82.04%	Sample Description: SAND (SP), little silt, tra	ace gravel, fine grained, re	eddish brown (COVER SOIL) TECH RPD DATE 3/23/06			
* ASTM D-4254 Standard Me	ed, but follow recommendations from: ethod for Maximum Index Density and Unit ethod for Maximum Index Density and Unit ming optimum values from Proctor Testing	Weight of Soil and Calculation of Relati	ve Density	REVIEW LAQ			

LOOSE & SETTLED DENSITY							
PROJECT NAME PROJECT NUMBER SAMPLE IDENTITY SAMPLE TYPE	Moab Tailings 053-2269 GABT-02 N/A Composite						
Mold Number Mold Weight (lb) Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	A3 9.572 4.000 4.573 0.0333	WATER CONTENT Wt Tare & Soil Wet Wt Tare & Soil Dry Wt Tare Wt Moisture Wt Dry Soil Water Content (dec) Water Content (%)	(W1) (W2) (W3) (W4=W1-W2) (W5=W2-W3) (wc=W4/W5) (W4/W5)*100	443.20         411.90         6.60         31.30         405.30         0.0772         7.72%			
LOOSE DENSITY Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	12.190 2.618 78.72 73.08						
SETTLE DENSITY A verage Settlement (in.) Settled Height (in.) Settlement Volume (cu.ft.) Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	1.005 3.568 0.0259 12.190 2.618 100.91 93.67	Settleme	nt (in.): 1.000 1.000 1.063 1.031 1.000 0.938				
Loose Wet Density (pcf) Loose Dry Density (pcf) Settled Wet Density (pcf) Settled Dry Density (pcf) Vertical Percent Settlement Percent Compaction	78.723         73.079         100.907         93.673         21.98%         85.78%	Sample Description: SAND (SP), some silt, p	oorly graded, yellowish b	<b>TECH</b> RPD <b>DATE</b> 3/23/06			
* ASTM D-4254 Standard Me	ed, but follow recommendations from: ethod for Maximum Index Density and Unit ' ethod for Maximum Index Density and Unit ' ming optimum values from Proctor Testing.		ve Density	REVIEW LAQ			

LOOSE & SETTLED DENSITY								
PROJECT NAME PROJECT NUMBER SAMPLE IDENTITY SAMPLE TYPE	Moab Tailings 053-2269 GABT-03 N/A Composite							
Mold Number Mold Weight (lb) Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	A3 9.572 4.000 4.573 0.0333	WATER CONTENT Wt Tare & Soil Wet Wt Tare & Soil Dry Wt Tare Wt Moisture Wt Dry Soil Water Content (dec) Water Content (%)	(W1) (W2) (W3) (W4=W1-W2) (W5=W2-W3) (wc=W4/W5) (W4/W5)*100	387.60         351.60         6.70         36.00         344.90         0.1044         10.44%				
LOOSE DENSITY Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	11.692 2.120 63.75 57.72							
SETTLE DENSITY A verage Settlement (in.) Settled Height (in.) Settlement Volume (cu.ft.) Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	1.443 3.130 0.0228 11.692 2.120 93.13 84.33	Settleme	nt (in.): 1.375 1.438 1.438 1.500 1.531 1.375					
Loose Wet Density (pcf) Loose Dry Density (pcf) Settled Wet Density (pcf) Settled Dry Density (pcf) Vertical Percent Settlement Percent Compaction	63.748 57.723 93.133 84.330 31.55% 79.33%	Sample Description: SAND (SP), some silt, p TAILINGS).	oorly graded, moist, yellov	vish brown (SAND TECH RPD DATE 3/23/06				
* ASTM D-4254 Standard Me	ed, but follow recommendations from: ethod for Maximum Index Density and Unit ethod for Maximum Index Density and Unit ming optimum values from Proctor Testing.		ve Density	<b>REVIEW</b> LAQ				

LOOSE & SETTLED DENSITY								
PROJECT NAME PROJECT NUMBER SAMPLE IDENTITY SAMPLE TYPE	Moab Tailings       053-2269       GABT-04       N/A       Composite							
Mold Number Mold Weight (lb) Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	B3           9.578           4.000           4.573           0.0333	WATER CONTENT Wt Tare & Soil Wet Wt Tare & Soil Dry Wt Tare Wt Moisture Wt Dry Soil Water Content (dec) Water Content (%)	(W1) (W2) (W3) (W4=W1-W2) (W5=W2-W3) (wc=W4/W5) (W4/W5)*100	364.00           327.70           8.10           36.30           319.60           0.1136           11.36%				
LOOSE DENSITY Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	11.775 2.197 66.06 59.33							
SETTLE DENSITY A verage Settlement (in.) Settled Height (in.) Settlement Volume (cu.ft.) Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	1.396 3.177 0.0231 11.775 2.197 95.09 85.39	Settlemer	nt (in.): 1.313 1.375 1.625 1.500 1.313 1.250					
Loose Wet Density (pcf) Loose Dry Density (pcf) Settled Wet Density (pcf) Settled Dry Density (pcf) Vertical Percent Settlement Percent Compaction	66.064           59.326           95.093           85.394           30.53%           82.19%	Sample Description: SAND (SP), trace to little (SAND TAILINGS).	e silt, trace clay, poorly g	TECH LAQ DATE 3/23/06				
* ASTM D-4254 Standard Me	ed, but follow recommendations from: ethod for Maximum Index Density and Unit V ethod for Maximum Index Density and Unit V ming optimum values from Proctor Testing.		<i>r</i> e Density	REVIEW RPD				

LOOSE & SETTLED DENSITY							
PROJECT NAME PROJECT NUMBER SAMPLE IDENTITY SAMPLE TYPE	Moab Tailings       053-2269       GABT-05     N/A       Composite						
Mold Number Mold Weight (lb) Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	A3           9.572           4.000           4.573           0.0333	WATER CONTENT Wt Tare & Soil Wet Wt Tare & Soil Dry Wt Tare Wt Moisture Wt Dry Soil Water Content (dec) Water Content (%)	(W1)           (W2)           (W3)           (W4=W1-W2)           (W5=W2-W3)           (wc=W4/W5)           (W4/W5)*100	440.20           359.70           6.50           80.50           353.20           0.2279           22.79%			
LOOSE DENSITY Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	13.000 3.428 103.08 83.95						
SETTLE DENSITY A verage Settlement (in.) Settled Height (in.) Settlement Volume (cu.ft.) Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	0.844 3.729 0.0271 13.000 3.428 126.40 102.94	Settleme	ent (in.): 0.875 0.750 0.875 0.875 0.385 0.938 0.750				
Loose Wet Density (pcf) Loose Dry Density (pcf) Settled Wet Density (pcf) Settled Dry Density (pcf) Vertical Percent Settlement Percent Compaction	103.080 83.947 126.405 102.942 18.45% 90.86%	Sample Description: Clayey SAND (SC), poo TAILINGS).	orly graded, saturated, reddis	sh brown (TRANSITION TECH RPD DATE 3/23/06			
* ASTM D-4254 Standard Me	lethod for Maximum Index Density and Unit W lethod for Maximum Index Density and Unit W uming optimum values from Proctor Testing.		ve Density	REVIEW LAQ			

LOOSE & SETTLED DENSITY								
PROJECT NAME PROJECT NUMBER SAMPLE IDENTITY SAMPLE TYPE	Moab Tailings 053-2269 GABT-06 N/A Composite							
Mold Number Mold Weight (lb) Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	B3 9,578 4,000 4,573 0.0333	WATER CONTENT Wt Tare & Soil Wet Wt Tare & Soil Dry Wt Tare Wt Moisture Wt Dry Soil Water Content (dec) Water Content (%)	(W1) (W2) (W3) (W4=W1-W2) (W5=W2-W3) (wc=W4/W5) (W4/W5)*100	307.30           284.80           8.10           22.50           276.70           0.0813           8.13%				
LOOSE DENSITY Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	11.800 2.222 66.82 61.79							
SETTLE DENSITY Average Settlement (in.) Settled Height (in.) Settlement Volume (cu.ft.) Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	1.386           3.188           0.0232           11.800           2.222           95.86           88.65	Settlemen	nt (in.): 1.313 1.500 1.375 1.500 1.375 1.250					
Loose Wet Density (pcf) Loose Dry Density (pcf) Settled Wet Density (pcf) Settled Dry Density (pcf) Vertical Percent Settlement Percent Compaction	66.815 61.791 95.858 88.649 30.30% 82.62%	Sample Description: SAND (SP), little to som	e silt, saturated, reddish br	own (SAND TAILINGS). TECH LAQ DATE 3/23/06				
* ASTM D-4254 Standard Me	d, but follow recommendations from: thod for Maximum Index Density and Unit thod for Maximum Index Density and Unit ning optimum values from Proctor Testing.	Weight of Soil and Calculation of Relativ	/e Density	<b>REVIEW</b> RPD				

LOOSE & SETTLED DENSITY							
PROJECT NAME PROJECT NUMBER SAMPLE IDENTITY SAMPLE TYPE	Moab Tailings       053-2269       GABT-07       N/A       Composite						
Mold Number Mold Weight (lb) Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	A3 9.572 4.000 4.573 0.0333	WATER CONTENT Wt Tare & Soil Wet Wt Tare & Soil Dry Wt Tare Wt Moisture Wt Dry Soil Water Content (dec) Water Content (%)	(W1) (W2) (W3) (W4=W1-W2) (W5=W2-W3) (wc=W4/W5) (W4/W5)*100	512.20           376.50           8.10           135.70           368.40           0.3683           36.83%			
LOOSE DENSITY Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	11.991 2.419 72.74 53.16						
SETTLE DENSITY Average Settlement (in.) Settled Height (in.) Settlement Volume (cu.ft.) Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	1.698           2.875           0.0209           11.991           2.419           115.71           84.56	Settlemer	nt (in.): 1.625 1.625 1.813 1.750 1.813 1.563				
Loose Wet Density (pcf) Loose Dry Density (pcf) Settled Wet Density (pcf) Settled Dry Density (pcf) Vertical Percent Settlement Percent Compaction	72.739 53.158 115.706 84.559 37.13% 78.81%	Sample Description: Sandy SILT (ML), moist	, reddish brown (TRANS	<b>TECH</b> RPD <b>DATE</b> 3/23/06			
* ASTM D-4254 Standard Me	ed, but follow recommendations from: ethod for Maximum Index Density and Unit V ethod for Maximum Index Density and Unit V ming optimum values from Proctor Testing.		re Density	REVIEW LAQ			

	LOOSE & SETTLED DENSITY				
PROJECT NAME PROJECT NUMBER SAMPLE IDENTITY SAMPLE TYPE	Moab Tailings       053-2269       GABT-07a     N/A       Composite				
Mold Number Mold Weight (lb) Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	A3           9.572           4.000           4.573           0.0333	WATER CONTENT Wt Tare & Soil Wet Wt Tare & Soil Dry Wt Tare Wt Moisture Wt Dry Soil Water Content (dec) Water Content (%)	(W1) (W2) (W3) (W4=W1-W2) (W5=W2-W3) (wc=W4/W5) (W4/W5)*100	556.80           433.40           6.70           123.40           426.70           0.2892           28.92%	
LOOSE DENSITY Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	11.698 2.126 63.93 49.59				
SETTLE DENSITY Average Settlement (in.) Settled Height (in.) Settlement Volume (cu.ft.) Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	2.136 2.437 0.0177 11.698 2.126 119.95 93.04	Settlemen	ent (in.): 2.188 2.188 2.063 2.125 2.188 2.063		
Loose Wet Density (pcf) Loose Dry Density (pcf) Settled Wet Density (pcf) Settled Dry Density (pcf) Vertical Percent Settlement Percent Compaction	63.929 49.588 119.953 93.045 46.71% 86.71%	Sample Description: Sandy SILT (ML), moist	it, reddish brown (TRANSI	TION TAILINGS). TECH RPD DATE 3/23/06	
* ASTM D-4254 Standard Me	lethod for Maximum Index Density and Unit W lethod for Maximum Index Density and Unit W uming optimum values from Proctor Testing.		ve Density	<b>REVIEW</b> LAQ	

	LOOSE & SETTLED DENSITY				
PROJECT NAME PROJECT NUMBER SAMPLE IDENTITY SAMPLE TYPE	Moab Tailings 053-2269 GABT-08 N/A Composite				
Mold Number Mold Weight (lb) Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	B3           9.578           4.000           4.573           0.0333	WATER CONTENT Wt Tare & Soil Wet Wt Tare & Soil Dry Wt Tare Wt Moisture Wt Dry Soil Water Content (dec) Water Content (%)	(W1)           (W2)           (W3)           (W4=W1-W2)           (W5=W2-W3)           (wc=W4/W5)           (W4/W5)*100	334.80           294.00           6.60           40.80           287.40           0.1420           14.20%	
LOOSE DENSITY Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	11.959 2.381 71.60 62.70				
SETTLE DENSITY A verage Settlement (in.) Settled Height (in.) Settlement Volume (cu.ft.) Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	1.521 3.052 0.0222 11.959 2.381 107.27 93.94	Settleme	nt (in.): 1.750 1.750 1.375 1.375 1.500 1.375		
Loose Wet Density (pcf) Loose Dry Density (pcf) Settled Wet Density (pcf) Settled Dry Density (pcf) Vertical Percent Settlement Percent Compaction	71.597 62.696 107.272 93.936 33.26% 83.28%	Sample Description: Sandy SILT (ML), little TAILINGS).	to some clay, moist, redd	ish brown (TRANSITION TECH LAQ DATE 3/23/06	
* ASTM D-4254 Standard Me	ed, but follow recommendations from: ethod for Maximum Index Density and Unit ethod for Maximum Index Density and Unit ming optimum values from Proctor Testing.		ve Density	REVIEW RPD	

LOOSE & SETTLED DENSITY				
PROJECT NAME PROJECT NUMBER SAMPLE IDENTITY SAMPLE TYPE	Moab Tailings 053-2269 GABT-09 N/A Composite			
Mold Number Mold Weight (lb) Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	B3           9.578           4.000           4.573           0.0333	WATER CONTENT Wt Tare & Soil Wet Wt Tare & Soil Dry Wt Tare Wt Moisture Wt Dry Soil Water Content (dec) Water Content (%)	(W1)           (W2)           (W3)           (W4=W1-W2)           (W5=W2-W3)           (wc=W4/W5)           (W4/W5)*100	227.60 175.10 6.60 52.50 168.50 0.3116 31.16%
LOOSE DENSITY Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	11.965 2.387 71.78 54.73			
SETTLE DENSITY A verage Settlement (in.) Settled Height (in.) Settlement Volume (cu.ft.) Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	1.782 2.792 0.0203 11.965 2.387 117.58 89.65	Settleme	nt (in.): 1.938 1.750 1.688 1.750 1.938 1.625	
Loose Wet Density (pcf) Loose Dry Density (pcf) Settled Wet Density (pcf) Settled Dry Density (pcf) Vertical Percent Settlement Percent Compaction	71.777 54.726 117.584 89.651 38.96% 87.89%	Sample Description: Sandy CLAY (CL), som (TRANSITION TAILIN	e silt, medium plasticity, gr (GS).	<b>TECH</b> LAQ <b>DATE</b> 3/23/06
* ASTM D-4254 Standard Me	ed, but follow recommendations from: ethod for Maximum Index Density and Unit ethod for Maximum Index Density and Unit ming optimum values from Proctor Testing.		ve Density	REVIEW RPD

LOOSE & SETTLED DENSITY				
PROJECT NAME PROJECT NUMBER SAMPLE IDENTITY SAMPLE TYPE	Moab Tailings 053-2269 GABT-10 N/A Composite			
Mold Number Mold Weight (lb) Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	B3         9.578           4.000         4.573           0.0333         0.0333	WATER CONTENT Wt Tare & Soil Wet Wt Tare & Soil Dry Wt Tare Wt Moisture Wt Dry Soil Water Content (dec) Water Content (%)	(W2) (W3) (W4=W1-W2) (W5=W2-W3) (wc=W4/W5)	205.30           166.20           6.60           39.10           159.60           0.2450           24.50%
LOOSE DENSITY Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	12.320 2.742 82.45 66.23			
SETTLE DENSITY A verage Settlement (in.) Settled Height (in.) Settlement Volume (cu.ft.) Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	1.604           2.969           0.0216           12.320           2.742           127.00           102.01	Settlemer	tt (in.): 1.750 1.750 1.500 1.500 1.500 1.625	
Loose Wet Density (pcf) Loose Dry Density (pcf) Settled Wet Density (pcf) Settled Dry Density (pcf) Vertical Percent Settlement Percent Compaction	82.452 66.227 127.003 102.012 35.08% 94.63%	Sample Description: Sandy CLAY (CL), some TAILINGS).	e silt, medium plasticity, wet, c	lark gray (TRANSITION TECH LAQ DATE 3/23/06 REVIEW RPD
* ASTM D-4254 Standard Me	d, but follow recommendations from: thod for Maximum Index Density and Unit thod for Maximum Index Density and Unit ning optimum values from Proctor Testing.	Weight of Soil and Calculation of Relativ	e Density	KEVIEW RPD

	LOOSE & SETTLED DENSITY				
PROJECT NAME PROJECT NUMBER SAMPLE IDENTITY SAMPLE TYPE	Moab Tailings           053-2269           GABT-11         N/A           Composite				
Mold Number Mold Weight (lb) Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	A3 9.572 4.000 4.573 0.0333	WATER CONTENT Wt Tare & Soil Wet Wt Tare & Soil Dry Wt Tare Wt Moisture Wt Dry Soil Water Content (dec) Water Content (%)	(W1) (W2) (W3) (W4=W1-W2) (W5=W2-W3) (wc=W4/W5) (W4/W5)*100	397.20         278.30         6.60         118.90         271.70         0.4376         43.76%	
LOOSE DENSITY Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	11.512 1.940 58.34 40.58				
SETTLE DENSITY A verage Settlement (in.) Settled Height (in.) Settlement Volume (cu.ft.) Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	1.750 2.823 0.0205 11.512 1.940 94.50 65.73	Settleme	nt (in.): 1.750 1.875 1.875 1.625 1.750 1.625		
Loose Wet Density (pcf) Loose Dry Density (pcf) Settled Wet Density (pcf) Settled Dry Density (pcf) Vertical Percent Settlement Percent Compaction	58.336           40.578           94.498           65.733           38.27%           68.47%	Sample Description: SILT (ML), low plasticit	ry, moist, dark brown (SLI	<b>TECH</b> RPD <b>DATE</b> 3/23/06	
* ASTM D-4254 Standard Me	ed, but follow recommendations from: ethod for Maximum Index Density and Unit V ethod for Maximum Index Density and Unit V ming optimum values from Proctor Testing.		ve Density	REVIEW LAQ	

LOOSE & SETTLED DENSITY				
PROJECT NAME PROJECT NUMBER SAMPLE IDENTITY SAMPLE TYPE	Moab Tailings 053-2269 GABT-12 N/A Composite			
Mold Number Mold Weight (lb) Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	A3           9.572           4.000           4.573           0.0333	WATER CONTENT Wt Tare & Soil Wet Wt Tare & Soil Dry Wt Tare Wt Moisture Wt Dry Soil Water Content (dec) Water Content (%)	(W1) (W2) (W3) (W4=W1-W2) (W5=W2-W3) (wc=W4/W5) (W4/W5)*100	475.00 324.20 6.60 150.80 317.60 0.4748 47.48%
LOOSE DENSITY Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	11.920 2.348 70.60 47.87			
SETTLE DENSITY Average Settlement (in.) Settled Height (in.) Settlement Volume (cu.ft.) Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	0.771 3.802 0.0277 11.920 2.348 84.92 57.58	Settlemen	nt (in.): 0.750 0.875 0.750 0.750 0.750 0.750 0.750	
Loose Wet Density (pcf) Loose Dry Density (pcf) Settled Wet Density (pcf) Settled Dry Density (pcf) Vertical Percent Settlement Percent Compaction	70.604           47.873           84.918           57.579           16.86%           56.67%	Sample Description: Silty Clay (CL), medium	plasticity, moist, grayish	brown (SLIMES TAILINGS). TECH RPD DATE 3/23/06
* ASTM D-4254 Standard Me	d, but follow recommendations from: ethod for Maximum Index Density and Unit ethod for Maximum Index Density and Unit ming optimum values from Proctor Testing.	Weight of Soil and Calculation of Relativ	re Density	REVIEW LAQ

LOOSE & SETTLED DENSITY				
PROJECT NAME PROJECT NUMBER SAMPLE IDENTITY SAMPLE TYPE	Moab Tailings           053-2269           GABT-13         N/A           Composite			
Mold Number Mold Weight (lb) Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	B3 9.578 4.000 4.573 0.0333	WATER CONTENT Wt Tare & Soil Wet Wt Tare & Soil Dry Wt Tare Wt Moisture Wt Dry Soil Water Content (dec) Water Content (%)	(W1) (W2) (W3) (W4=W1-W2) (W5=W2-W3) (wc=W4/W5) (W4/W5)*100	327.30         233.30         6.70         94.00         226.60         0.4148         41.48%
LOOSE DENSITY Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	11.870 2.292 68.92 48.71			
SETTLE DENSITY A verage Settlement (in.) Settled Height (in.) Settlement Volume (cu.ft.) Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	1.813         2.761         0.0201         11.870         2.292         114.17         80.70	Settlemer	nt (in.): 2.000 1.875 1.875 1.750 1.750 1.625	
Loose Wet Density (pcf) Loose Dry Density (pcf) Settled Wet Density (pcf) Settled Dry Density (pcf) Vertical Percent Settlement Percent Compaction	68.920 48.713 114.172 80.697 39.63% 84.94%	Sample Description: CLAY (CL), low to med TAILINGS).	ium plasticity, moist, gray	TECH LAQ DATE 3/23/06
* ASTM D-4254 Standard Me	ed, but follow recommendations from: ethod for Maximum Index Density and Unit V ethod for Maximum Index Density and Unit V ming optimum values from Proctor Testing.		re Density	REVIEW RPD

LOOSE & SETTLED DENSITY				
PROJECT NAME PROJECT NUMBER SAMPLE IDENTITY SAMPLE TYPE	Moab Tailings       053-2269       GABT-14       N/A       Composite			
Mold Number Mold Weight (lb) Mold Diameter (in.) Mold Height (in.) Mold Volume (cu.ft.)	A3 9.572 4.000 4.573 0.0333	WATER CONTENT Wt Tare & Soil Wet Wt Tare & Soil Dry Wt Tare Wt Moisture Wt Dry Soil Water Content (dec) Water Content (%)	(W1) (W2) (W3) (W4=W1-W2) (W5=W2-W3) (wc=W4/W5) (W4/W5)*100	470.00 324.20 6.60 145.80 317.60 0.4591 45.91%
LOOSE DENSITY Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	12.691 3.119 93.79 64.28			
SETTLE DENSITY Average Settlement (in.) Settled Height (in.) Settlement Volume (cu.ft.) Mass of wet soil & mold (lb) Mass of wet soil (lb) Wet Density of Soil (pcf) Dry Density of Soil (pcf)	0.792 3.781 0.0275 12.691 3.119 113.42 77.74	Settlemer	nt (in.): 0.750 0.875 0.875 0.750 0.750 0.750	
Loose Wet Density (pcf) Loose Dry Density (pcf) Settled Wet Density (pcf) Settled Dry Density (pcf) Vertical Percent Settlement Percent Compaction	93.788 64.280 113.424 77.737 17.31% 76.59%	Sample Description: CLAY (CH), High plasti	city, moist, dark gray (SI	<b>TECH</b> RPD <b>DATE</b> 3/23/06
* ASTM D-4254 Standard Me	ed, but follow recommendations from: ethod for Maximum Index Density and Unit V ethod for Maximum Index Density and Unit V ming optimum values from Proctor Testing.		re Density	REVIEW LAQ

Appendix B

Certificate of Analysis

November 7, 2006



Geotechnical Laboratory PO Box 4339 1570 Bear Creek Road Oak Ridge TN 37830 (865) 482-6497

### **CERTIFICATE OF ANALYSIS**

Erin Stanley General Engineering Laboratories 2040 Savage Road Charleston SC 29407 November 7, 2006

This is the Certificate of Analysis for the following samples:

Shaw Project ID: Shaw Project Number: Date Received by Lab: Number of Samples: Sample Type: **GEL – Moab** 109855.01410000 May 4, 2006 Fourteen (14) Soil

#### I. Introduction/Case Narrative

Fourteen soil samples were received by the Shaw Geotechnical Laboratory on May 4, 2006. Samples were submitted for determination of particle-size distribution, Atterberg limits, hydraulic conductivity, consolidated-undrained triaxial shear strength, one-dimensional consolidation properties, and capillary moisture relationships.

Please see Appendix A, Sample Number Cross Reference List; Appendix B, Analysis Results; and Appendix C, Chain-of-Custody/Sample Receipt Records.

Reviewed and Approved:

Juluhl

Ralph Cole Laboratory Manager, Geotechnical Services

#### II. <u>Analytical Results/Methodology</u>

REFERENCES: United Nations, *Recommendations on the Transport of Dangerous Goods, Manual of Tests and Criteria*, third ed. New York, 1999. United States Army Corps of Engineers (USACE), Engineer Manual 1110-2-1906, *Laboratory Soils Testing*, appendix II, 1970; United States Environmental Protection Agency, SW846, *Test Methods for Examining Solid Waste*, *Physical/Chemical Methods*, 3rd ed., Nov 1986 (EPA SW-846). Annual Book of ASTM Standards, Section 4, Construction, Volume 04.08, *Soil and Rock (I)*, and Volume 04.09, *Soil and Rock (II)*, 2006. Shaw Environmental and infrastructure, Standard Operating Procedures.

Particle-Size Distribution of Soils	ASTM D 422
Standard Proctor Compaction Characteristics	ASTM D 698
One-Dimensional Consolidation Properties of Soils	ASTM D 2435
Atterberg Limits	AASTM D 4318
Consolidated-Undrained Triaxial Shear Strength	ASTM D 4767
Constant Head Permeability	ASTM D 5084
Capillary Moisture Relationships	Mfr. procedure

### III. Quality Control

Quality control checks such as duplicates and spikes (QC samples), are not normally applicable to geotechnical testing. This is due largely to the inability of obtaining samples with known characteristics, the heterogenous nature of the samples, and quality control procedures built-in to the analytical method.

QC measures to ensure accuracy and precision of test results include the following:

- 100% verification of all numerical results raw data entries, transcriptions and calculations entered by lab technicians are checked, recalculated and verified. Most data calculations are performed by computer programs.
- Data validation through test reasonableness summaries of all test results for individual reports are reviewed to determine the overall reasonableness of data and to determine the presence of any data that may be considered outliers.
- Quality control procedures are built into most standardized geotechnical procedures. For example, liquid limit and plastic limit analyses call for re-analyses and specify acceptance criteria.

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- Routine instrument calibration instruments, gauges and equipment used in testing are calibrated on a routine basis. All instrument calibration follows ASTM or manufacturer guidelines.
- Maintenance of all past calibration records calibration records and certification documents of all instruments, gauges and equipment are updated routinely and maintained in the Quality Control Coordinators Quality/Operations files.
- Certified and trained personnel all technicians are trained in the application of standard laboratory procedures for geotechnical analyses as well as the quality assurance measures implemented by Shaw.
- Quantitative analyses frequently used in geotechnical/physical testing programs do not use QC tools common to wet chemistry or radiochemistry laboratories. Measures not employed in the analysis of samples reported in this report include: laboratory control samples (LCS), blanks, matrix spikes (MS), duplicate analyses, dilutions, digestions, correction factors, surrogate sample analyses, detection limit determinations, control charts, and/or tentatively identified compounds (TICs).

#### IV. Data Qualification

Moisture content results are reported as supporting data for several test results.

A specific gravity value of 2.65 was assumed and used for certain calculations of ancillary supporting data, such as phase relations (degree of saturation, void ratio) reported in some test results. This value is an average of all natural earth materials and could vary markedly from the sample being tested.

Hydraulic conductivity (permeability) tests were performed by recompacting bulk material into test specimens. Density and moisture parameters supplied by the client. One test specimen was prepared for each sample. The specimens were tested at 20 psi confining pressure, followed by 40 psi and 60 psi test series. In some cases, permeability tests at higher confining pressures produced slower flow rates. This is usually ascribed to specimen consolidation and smaller void channels. In other cases, permeability tests at higher confining pressures produced faster flow rates. This is usually ascribed to more thorough specimen saturation, or washing of fines.

Consolidated-undrained triaxial shear tests were performed by recompacting bulk material into test specimens. Density and moisture parameters supplied by the client. Three test specimens were prepared for each sample. The specimens were tested at 20 psi, 40 psi and 60 psi confining pressures.

Appendix A Sample Cross-Reference List

### SAMPLE NUMBER CROSS-REFERENCE LIST (Cont'd)

LAB SAMPLE NO.	CLIENT SAMPLE NO.	MATRIX
BC0911	GABT-01	Soil
BC0912	GABT-02	Soil
BC0913	GABT-03	Soil
BC0914	GABT-04	Soil
BC0915	GABT-05	Soil
BC0916	GABT-06	Soil
BC0917	GABT-07	Soil
BC0918	GABT-08	Soil
BC0919	GABT-09	Soil
BC0920	GABT-10	Soil
BC0921	GABT-11	Soil
BC0922	GABT-12	Soil
BC0923	GABT-13	Soil
BC0924	GABT-14	Soil

Appendix B Data Results Page 5 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

### **MOISTURE CONTENT**

PROJECT NAME GEL - Moab PROJECT NUMBER 109855.01410000

LAB SAMPLE NO.	CLIENT SAMPLE NO.	MOISTURE, % ASTM D 2216	MOISTURE , % SW846	SOLIDS, % SW846
BC0911	GABT-01	7.0	6.6	93.4
BC0912	GATB-02	6.6	6.1	93.9
BC0913	GATB-03	12.6	11.2	88.8
BC0914	GATB-04	12.8	11.3	88.7
BC0915	GATB-05	20.7	17.1	82.9
BC0916	GATB-06	8.1	7.5	92.5
BC0917	GATB-07	34.1	25.4	74.6
BC0918	GATB-08	11.4	10.2	89.8
BC0919	GATB-09	20.5	17.0	83.0
BC0920	GATB-10	16.9	14.5	85.5
BC0921	GATB-11	24.8	19.9	80.1
BC0922	GATB-12	20.9	17.3	82.7
BC0923	GATB-13	26.9	21.2	78.8
BC0924	GATB-14	17.8	15.1	84.9

ASTM D 2216 results are based on dry sample weight. SW846 results are based on wet sample weight. Solids content is determined by subtracting the SW846 moisture (%) from 100. Page 6 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

# ATTERBERG LIMITS ASTM D 4318

PROJECT NAME:

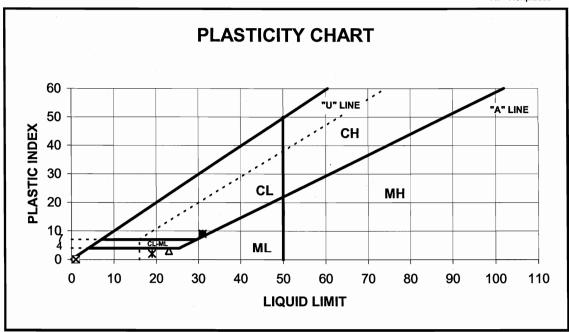
GEL-Moab

PROJECT NO. 109855.01410000

### ATTERBERG LIMITS RESULTS

LAB SAMPL	E NO.	FIELD SAMPLE NO.	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	USCS SYMBOL
BC0911	•	GABT-01	NP	NP	NP	NP
BC0912	•	GABT-02	NP	NP	NP	NP
BC0913		GABT-03	NP	NP	NP	NP
BC0914	<b>\$</b>	GABT-04	NP	NP	NP	NP
BC0915	+	GABT-05	NP	NP	NP	NP
BC0916	0	GABT-06	NP	NP	NP	NP
BC0917		GABT-07	31	22	9	CL
BC0918	X	GABT-08	NP	NP	NP	NP
BC0919	Δ	GABT-09	23	20	3	ML
BC0920	*	GABT-10	19	17	2	ML

*NP=Nonplastic



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Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

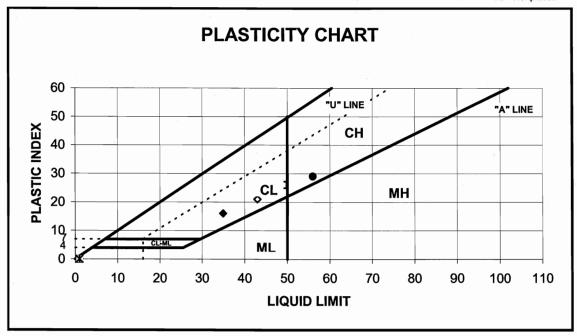
# ATTERBERG LIMITS ASTM D 4318

PROJECT NAME: GEL-Moab PROJECT NO. 109855.01410000

### ATTERBERG LIMITS RESULTS

LAB SAMPLE NO.		FIELD SAMPLE NO.	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	USCS SYMBOL
BC0921	•	GABT-11	56	27	29	СН
BC0922	•	GABT-22	35	19	16	CL
BC0923		GABT-13	49	23	26	CL
BC0924	<b>\$</b>	GABT-14	43	22	21	CL
	+					
	0					
	х					
	Δ				· ·	
	*					· · · · · · · · · · · · · · · · · · ·

*NP=Nonplastic



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# Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

# PARTICLE-SIZE ANALYSIS ASTM D 422

SIEVE ANALYSIS

Project Name GEL - Moab Project No. 109855.01310000

Specific Gravity = 2.65 assumed Client Sample No. GABT-01 Lab Sample No. BC0911

Moisture Content = 7.0% based on dry sample weight

	Sieve	Diameter	Percent
с	No.	mm	Finer
0	3"	75.000	100.0%
Α	1.5"	37.500	100.0%
R	0.75"	19.000	100.0%
S E	0.375"	9.500	97.8%
Ŀ	#4	4.750	95.6%
	#10	2.000	92.6%

	Sieve	Diameter	Percent
	No.	mm	Finer
F	#20	0.850	89.9%
Т	#40	0.425	82.8%
N	#60	0.250	69.0%
E	#100	0.149	46.6%
	#140	0.106	32.8%
	#200	0.075	22.9%

#### HYDROMETER ANALYSIS

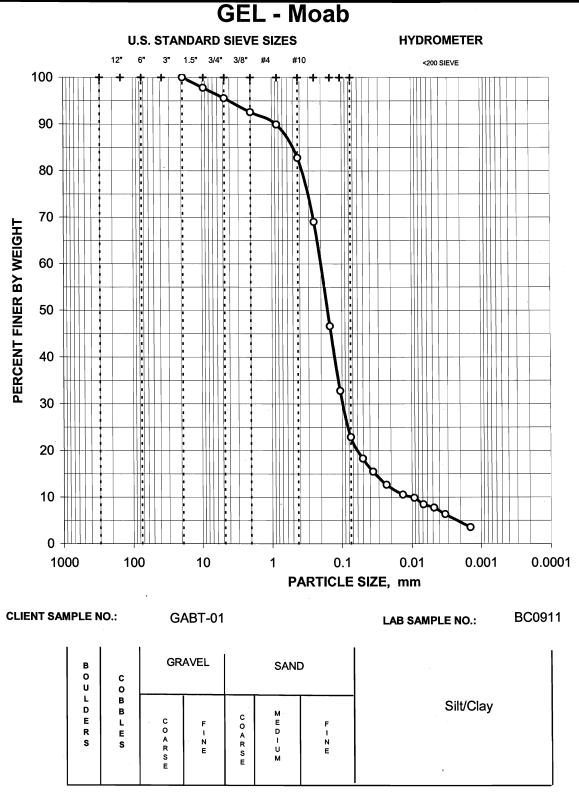
	Diameter	Percent
	mm	Finer
H		
Y D	0.05044	18.3%
R	0.03608	15.5%
0	0.02316	12.7%
M	0.01347	10.6%
E T	0.00925	9.9%
E	0.00681	8.4%
R	0.00480	7.7%
	0.00332	6.3%
	0.00143	3.5%

4.4% Gravel

```
72.7% Sand
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22.9% Silt/Clay

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## Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

# PARTICLE-SIZE ANALYSIS ASTM D 422

Project Name GEL - Moab Project No. 109855.01310000

Specific Gravity = 2.65 assumed Client Sample No. GABT-02 Lab Sample No. BC0912

Moisture Content = 6.6% based on dry sample weight

	Sieve	Diameter	Percent
с	No.	mm	Finer
0	3"	75.000	100.0%
Α	1.5"	37.500	100.0%
R	0.75"	19.000	100.0%
S E	0.375"	9.500	98.9%
L	#4	4.750	97.0%
	#10	2.000	95.5%

	Sieve	Diameter	Percent
	No.	mm	Finer
F	#20	0.850	94.1%
Т	#40	0.425	88.1%
N	#60	0.250	68.5%
E	#100	0.149	39.4%
	#140	0.106	25.0%
	#200	0.075	16.6%

#### HYDROMETER ANALYSIS

	Diameter	Percent
	mm	Finer
Н		
Y D	0.05102	14.1%
R	0.03635	12.8%
0	0.02324	10.2%
М	0.01361	7.7%
ET	0.00966	6.4%
E	0.00685	5.8%
R	0.00395	4.5%
	0.00335	3.8%
	0.00144	2.6%

3.0% Gravel

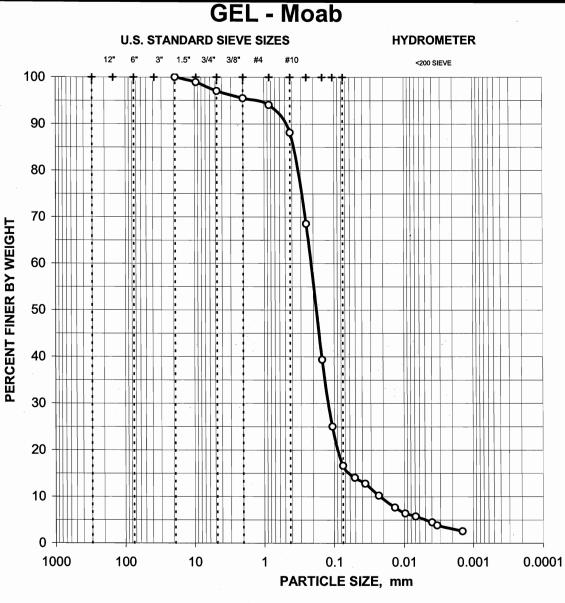
```
80.4% Sand
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16.6% Silt/Clay

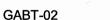
SIEVE ANALYSIS

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Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497



CLIENT SAMPLE NO.:



LAB SAMPLE NO.:

.: BC0912

B O U	c o	GR	AVEL		SANI	D	
L D E R S	B B L E S	C O A R S E	FINE	C O A R S E	M E D I U M	F I N E	Silt/Clay

Page 12 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

# Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

# PARTICLE-SIZE ANALYSIS ASTM D 422

SIEVE ANALYSIS

Project Name GEL - Moab Project No. 109855.01310000

Specific Gravity = 2.65 assumed Client Sample No. GABT-03 Lab Sample No. BC0913

Moisture Content = 12.6% based on dry sample weight

	Sieve	Diameter	Percent
С	No.	mm	Finer
0	3"	75.000	100.0%
Α	1.5"	37.500	100.0%
R	0.75"	19.000	100.0%
SE	0.375"	9.500	100.0%
	#4	4.750	99.5%
	#10	2.000	99.4%

	Sieve	Diameter	Percent
	No.	mm	Finer
F ·	#20	0.850	99.0%
Т	#40	0.425	93.9%
N	#60	0.250	77.7%
Е	#100	0.149	48.4%
	#140	0.106	29.0%
	#200	0.075	16.8%

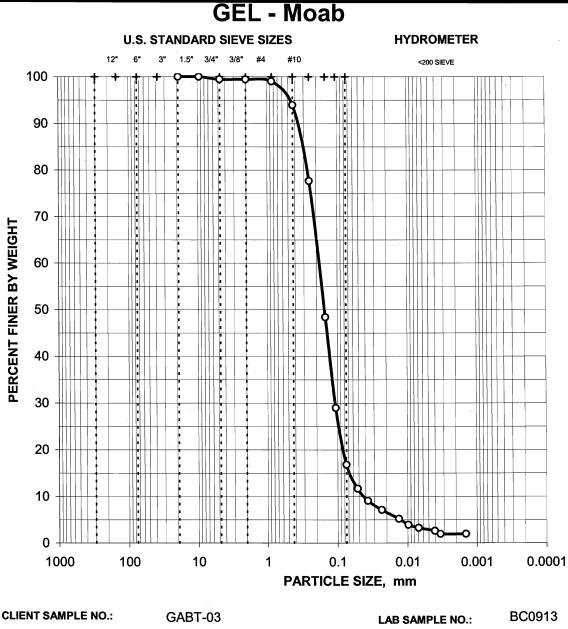
#### **HYDROMETER ANALYSIS**

0.5% Gravel

```
82.6% Sand
```

16.8% Silt/Clay

Page 13 of 108 November 7, 2006 **Erin Stanley General Engineering Laboratories** Shaw Project Name: GEL - Moab Shaw Project No. 109855.01410000



B O U	c o	GR	AVEL		SANE	0	
L D R S	B B L S S	C O A R S E	F I N E	C O A R S E	M u - C M	FINE	Silt/Clay

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# Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### PARTICLE-SIZE ANALYSIS ASTM D 422

SIEVE ANALYSIS

Project Name GEL - Moab Project No. 109855.01310000

Specific Gravity = 2.65 assumed Client Sample No. GABT-04 Lab Sample No. BC0914

Moisture Content = 12.8% based on dry sample weight

	Sieve	Diameter	Percent
с	No.	mm	Finer
0	3"	75.000	100.0%
Α	1.5"	37.500	100.0%
R S	0.75"	19.000	100.0%
S E	0.375"	9.500	100.0%
L	#4	4.750	99.7%
	#10	2.000	99.6%

_			
	Sieve	Diameter	Percent
	No.	mm	Finer
F	#20	0.850	99.4%
1	#40	0.425	96.9%
N	#60	0.250	82.0%
E	#100	0.149	52.9%
	#140	0.106	35.0%

0.075

23.6%

#200

#### HYDROMETER ANALYSIS

	Diameter	Percent
	mm	Finer
H		
Y D	0.05140	16.3%
R	0.03668	13.9%
0	0.02324	13.1%
Μ	0.01371	7.3%
E T	0.00976	5.7%
Ē	0.00692	4.9%
R	0.00490	4.1%
	0.00336	3.3%
1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 19	0.00144	3.3%

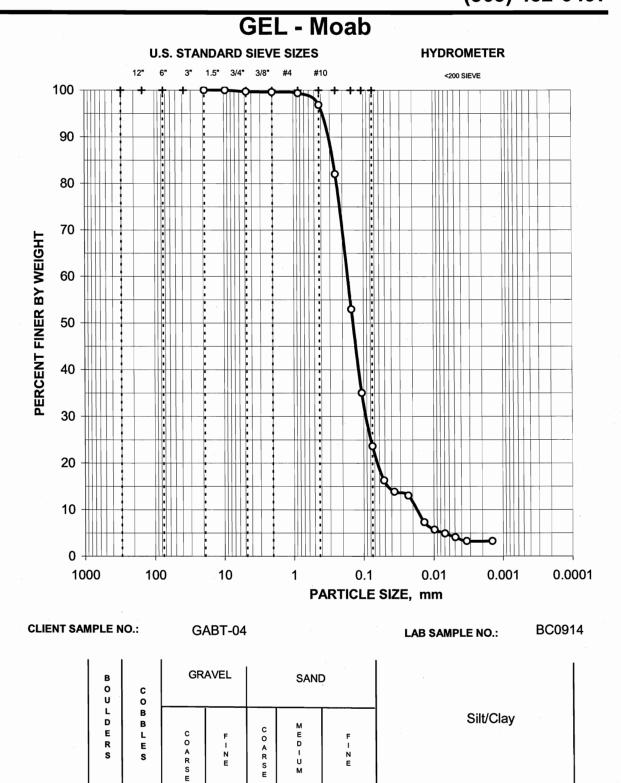
0.3% Gravel

```
76.1% Sand
```

23.6% Silt/Clay

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**Shaw Geotechnical** Laboratory Oak Ridge TN (865) 482-6497



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## Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### PARTICLE-SIZE ANALYSIS ASTM D 422

Project Name GEL - Moab Project No. 109855.01310000

Specific Gravity = 2.65 assumed Client Sample No. GABT-05 Lab Sample No. BC0915

Moisture Content = 20.7% based on dry sample weight

	Sieve	Diameter	Percent	
С	No.	mm	Finer	
0	3"	75.000	100.0%	
Α	1.5"	37.500	100.0%	
R	0.75"	19.000	100.0%	
S E	0.375"	9.500	98.7%	
-	#4	4.750	97.2%	
	#10	2.000	95.8%	

	Sieve	Diameter	Percent
	No.	mm	Finer
F	#20	0.850	94.4%
I	#40	0.425	90.0%
N	#60	0.250	73.4%
Е	#100	0.149	46.7%
	#140	0.106	31.5%
	#200	0.075	21.6%

#### HYDROMETER ANALYSIS

	Diameter	Percent
	mm	Finer
H		
Y D	0.05089	17.8%
R	0.03640	15.3%
0	0.01854	11.5%
М	0.01321	9.6%
E T	0.00971	8.3%
Ē	0.00687	7.0%
R	0.00491	5.7%
	0.00337	5.1%
	0.00128	4.5%

2.8% Gravel

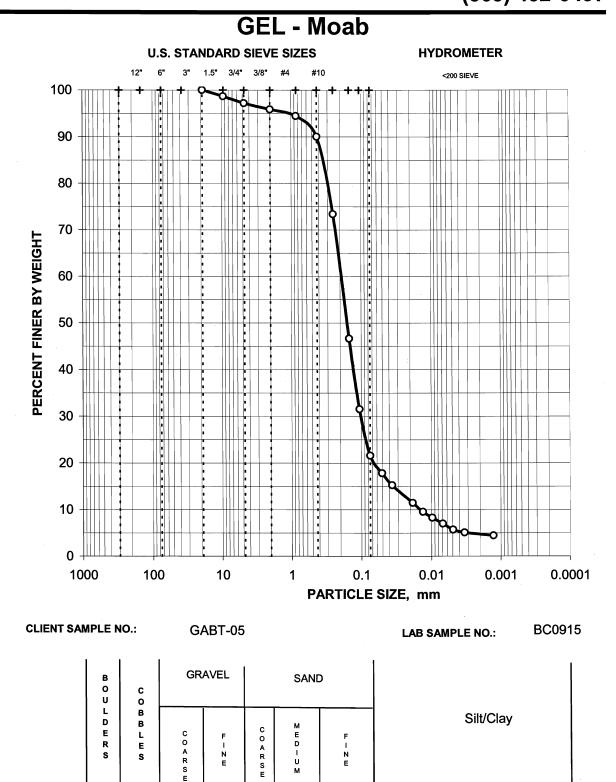
```
75.6% Sand
```

21.6% Silt/Clay

SIEVE ANALYSIS

Page 17 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL - Moab Shaw Project No. 109855.01410000

s



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## Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### PARTICLE-SIZE ANALYSIS ASTM D 422

SIEVE ANALYSIS

Project Name GEL - Moab Project No. 109855.01310000

Specific Gravity = 2.65 assumed Client Sample No. GABT-06 Lab Sample No. BC0916

Moisture Content = 8.1% based on dry sample weight

#### Sieve Diameter Percent No. mm Finer С 3" 75.000 100.0% 0 Α 1.5" 37.500 100.0% R 0.75" 19.000 100.0% s 0.375" 9.500 99.7% Е #4 4.750 99.5% #10 2.000 99.3%

	Sieve	Diameter	Percent
	No.	mm	Finer
F	#20	0.850	98.9%
Т	#40	0.425	92.8%
N	#60	0.250	69.9%
Е	#100	0.149	40.3%
	#140	0.106	25.6%
	#200	0.075	16.7%

#### HYDROMETER ANALYSIS

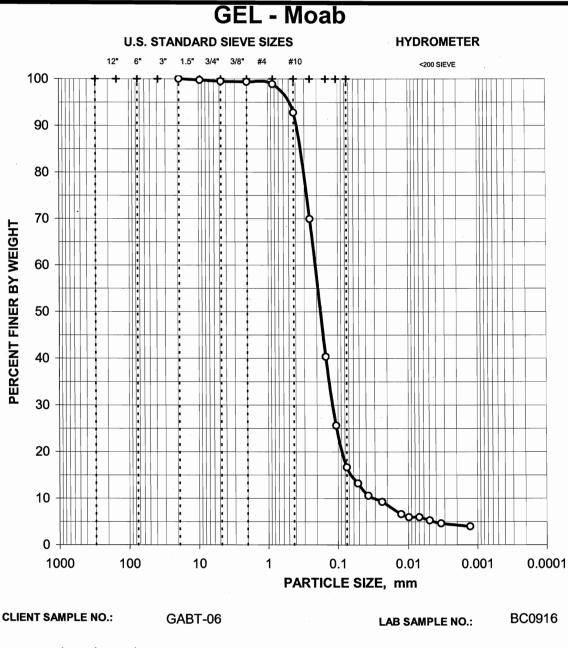
	Diameter	Percent
	mm	Finer
H		
Y D	0.05206	13.2%
R	0.03722	10.6%
0	0.02371	9.3%
М	0.01263	6.6%
E T	0.00982	5.9%
E	0.00694	5.9%
R	0.00493	5.3%
	0.00338	4.6%
	0.00129	4.0%

0.5% Gravel

```
82.8% Sand
```

16.7% Silt/Clay

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B O U	c o	GR	AVEL		SANE	D	
L D E R S	B L E S	с о < r » е	FINE	C O A R S E	MEDIUM	FINE	Silt/Clay

Page 20 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

## Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

# PARTICLE-SIZE ANALYSIS ASTM D 422

SIEVE ANALYSIS

Project Name GEL - Moab Project No. 109855.01310000

Specific Gravity = 2.65 assumed Client Sample No. GABT-07 Lab Sample No. BC0917

Moisture Content = 34.1% based on dry sample weight

	Sieve	Diameter	Percent
с	No.	mm	Finer
0	3"	75.000	100.0%
А	1.5"	37.500	100.0%
R	0.75"	19.000	100.0%
S E	0.375"	9.500	100.0%
-	#4	4.750	99.1%
	#10	2.000	98.1%

	Sieve	Diameter	Percent
	No.	mm	Finer
F	#20	0.850	95.4%
Ι	#40	0.425	92.1%
N	#60	0.250	85.1%
Е	#100	0.149	69.4%
	#140	0.106	58.4%
	#200	0.075	49.7%

#### HYDROMETER ANALYSIS

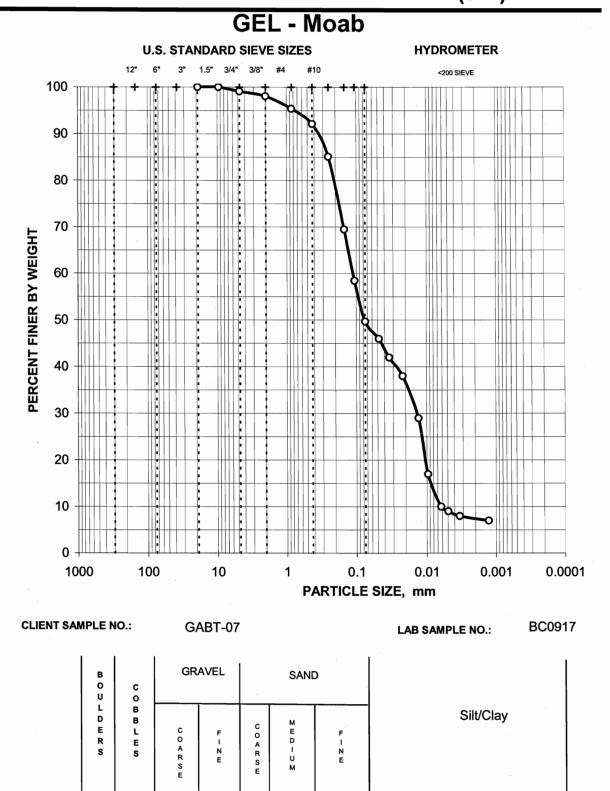
Diameter	Percent
mm	Finer
0.04784	46.0%
0.03427	42.0%
0.02195	38.0%
0.01309	29.0%
0.00959	17.0%
0.00623	10.0%
0.00491	9.0%
0.00337	8.0%
0.00128	7.0%
	mm 0.04784 0.03427 0.02195 0.01309 0.00959 0.00623 0.00491 0.00337

0.9% Gravel

```
49.4% Sand
```

49.7% Silt/Clay

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# Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### PARTICLE-SIZE ANALYSIS ASTM D 422

SIEVE ANALYSIS

Project Name GEL - Moab Project No. 109855.01310000

Specific Gravity = 2.65 assumed Client Sample No. GABT-08 Lab Sample No. BC0918

Moisture Content = 11.4% based on dry sample weight

_			
	Sieve	Diameter	Percent
С	No.	mm	Finer
0	3"	75.000	100.0%
Α	1.5"	37.500	100.0%
R	0.75"	19.000	100.0%
S E	0.375"	9.500	100.0%
-	#4	4.750	99.3%
	#10	2.000	98.4%

	Sieve	Diameter	Percent
	No.	mm	Finer
F	#20	0.850	96.6%
1 -	#40	0.425	90.7%
N	#60	0.250	78.7%
Е	#100	0.149	54.1%
	#140	0.106	39.1%
	#200	0.075	27.8%

#### HYDROMETER ANALYSIS

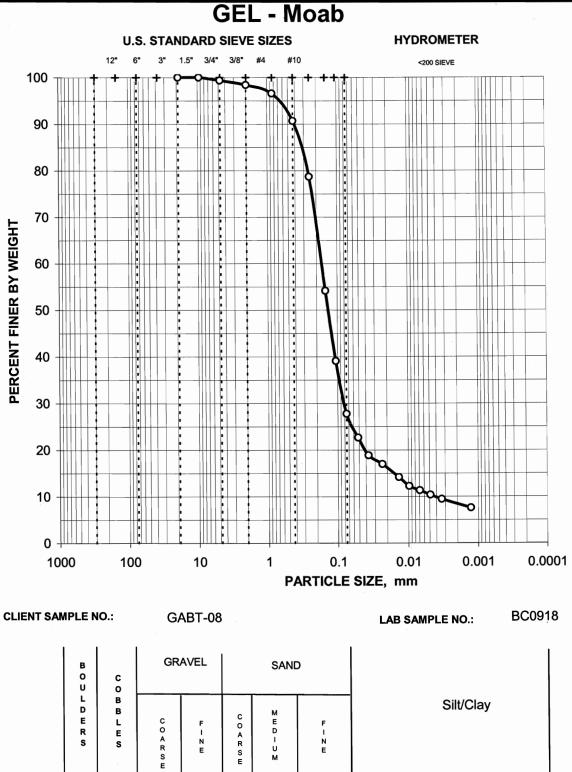
Diameter	Percent
mm	Finer
0.05148	22.7%
0.03681	18.9%
0.02346	17.0%
0.01364	14.2%
0.00971	12.3%
0.00689	11.3%
0.00486	10.4%
0.00335	9.4%
0.00128	7.6%
	mm 0.05148 0.03681 0.02346 0.01364 0.00971 0.00689 0.00486 0.00335

0.7% Gravel

```
71.6% Sand
```

27.8% Silt/Clay

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## Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### PARTICLE-SIZE ANALYSIS ASTM D 422

SIEVE ANALYSIS

Project Name GEL - Moab Project No. 109855.01310000

Specific Gravity = 2.65 assumed Client Sample No. GABT-09 Lab Sample No. BC0919

Moisture Content = 20.5% based on dry sample weight

#### Sieve Diameter Percent Finer No. mm С 3" 75.000 100.0% 0 А 1.5" 37.500 100.0% R 0.75" 19.000 100.0% s 0.375" 100.0% 9.500 Е #4 4.750 100.0% #10 2.000 100.0%

	Sieve	Diameter	Percent
	No.	mm	Finer
F	#20	0.850	98.4%
· 1	#40	0.425	96.5%
N	#60	0.250	94.1%
E	#100	0.149	85.0%
	#140	0.106	72.4%
	#200	0.075	58.3%

#### HYDROMETER ANALYSIS

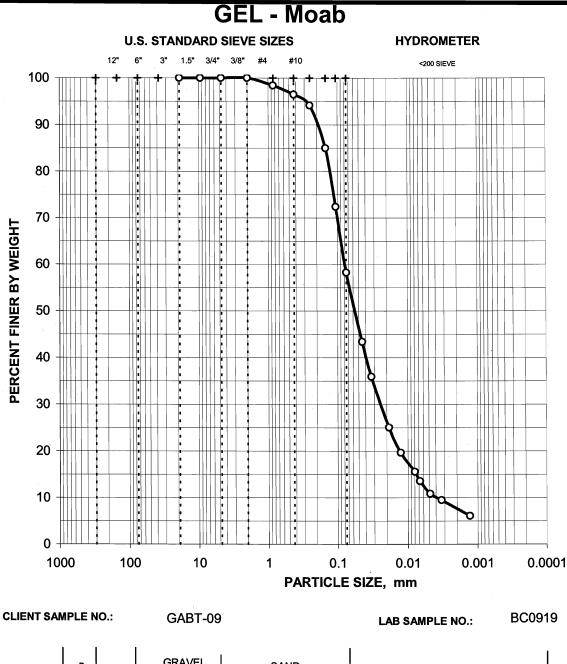
	Diameter	Percent
	mm	Finer
H		
Y D	0.04458	43.4%
R	0.03293	35.9%
0	0.01863	25.1%
М	0.01267	19.7%
E	0.00796	15.6%
T E	0.00672	13.6%
R	0.00481	10.8%
	0.00331	9.5%
	0.00130	6.1%

0.0% Gravel

```
41.7% Sand
```

58.3% Silt/Clay

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B O U	C O	GR	AVEL		SAN	)	
L D R S	B L E S	C O A R S E	F I N E	COARSE	2 w 2 M	<b>н — Z</b> Ш	Silt/Clay

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# Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### PARTICLE-SIZE ANALYSIS ASTM D 422

SIEVE ANALYSIS

Project Name GEL - Moab Project No. 109855.01310000

Specific Gravity = 2.65 assumed Client Sample No. GABT-10 Lab Sample No. BC0920

Moisture Content = 16.9% based on dry sample weight

С	Sieve	Diameter	Percent
	No.	mm	Finer
0	3"	75.000	100.0%
А	1.5"	37.500	100.0%
R	0.75"	19.000	100.0%
S E	0.375"	9.500	100.0%
-	#4	4.750	100.0%
	#10	2.000	98.2%

	Sieve	Diameter	Percent
	No.	mm	Finer
F	#20	0.850	95.7%
· 1	#40	0.425	90.3%
N	#60	0.250	78.5%
Е	#100	0.149	55.7%
	#140	0.106	40.8%
	#200	0.075	30.4%

#### **HYDROMETER ANALYSIS**

	Diameter	Percent
	mm	Finer
H	0.06621	29.9%
Y D	0.04806	25.5%
R	0.03455	22.4%
0	0.02230	18.7%
М	0.01356	8.1%
E	0.00963	7.5%
E	0.00678	6.8%
R	0.00480	6.8%
	0.00331	6.2%
	0.00138	6.2%

0.0% Gravel

```
69.6% Sand
```

30.4% Silt/Clay

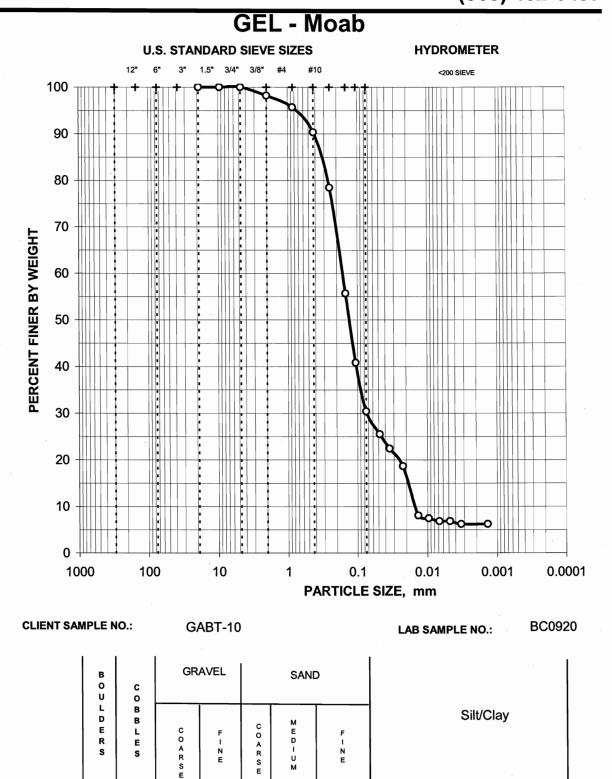
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N E

**Shaw Geotechnical** Laboratory Oak Ridge TN (865) 482-6497



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## Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

# PARTICLE-SIZE ANALYSIS ASTM D 422

Project Name GEL - Moab Project No. 109855.01310000

Specific Gravity = 2.65 assumed Client Sample No. GABT-11 Lab Sample No. BC0921

Moisture Content = 24.8% based on dry sample weight

С	Sieve	Diameter	Percent
	No.	mm	Finer
0	3"	75.000	100.0%
Α	1.5"	37.500	100.0%
R	0.75"	19.000	100.0%
S E	0.375"	9.500	100.0%
<b>-</b>	#4	4.750	100.0%
	#10	2.000	99.5%

#### SIEVE ANALYSIS

	Sieve	Diameter	Percent
	No.	mm	Finer
F	#20	0.850	98.0%
1.	#40	0.425	95.2%
N	#60	0.250	91.5%
E	#100	0.149	86.2%
	#140	0.106	82.2%
	#200	0.075	78.4%

#### **HYDROMETER ANALYSIS**

	Diameter	Percent
	mm	Finer
H		
P P		
R	0.02636	75.2%
0	0.01595	69.1%
M	0.01059	61.4%
E	0.00775	55.3%
E	0.00568	49.1%
R	0.00418	41.5%
	0.00296	33.8%
	0.00133	16.9%

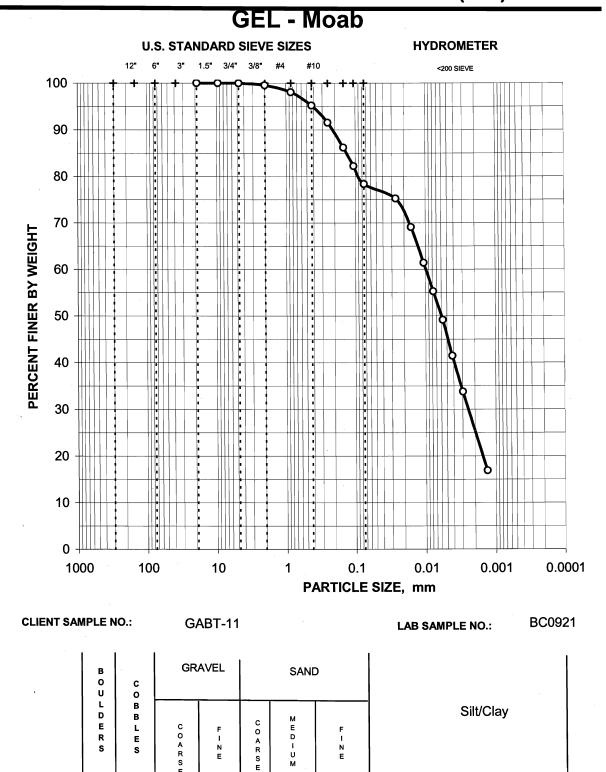
0.0% Gravel

```
21.6% Sand
```

78.4% Silt/Clay

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**Shaw Geotechnical** Laboratory Oak Ridge TN (865) 482-6497



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## Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

## PARTICLE-SIZE ANALYSIS ASTM D 422

SIEVE ANALYSIS

Project Name GEL - Moab Project No. 109855.01310000

Specific Gravity = 2.65 assumed Client Sample No. GABT-12 Lab Sample No. BC0922

Moisture Content = 20.9% based on dry sample weight

#### Sieve Diameter Percent No. mm Finer С 3" 75.000 100.0% 0 А 1.5" 37.500 100.0% R 0.75" 19.000 100.0% S 0.375" 9.500 100.0% Е #4 4.750 100.0% #10 2.000 99.6%

	Sieve	Diameter	Percent
	No.	mm	Finer
F	#20	0.850	95.5%
1.	#40	0.425	91.2%
N	#60	0.250	86.8%
E	#100	0.149	75.8%
	#140	0.106	63.9%
	#200	0.075	58.8%

#### HYDROMETER ANALYSIS

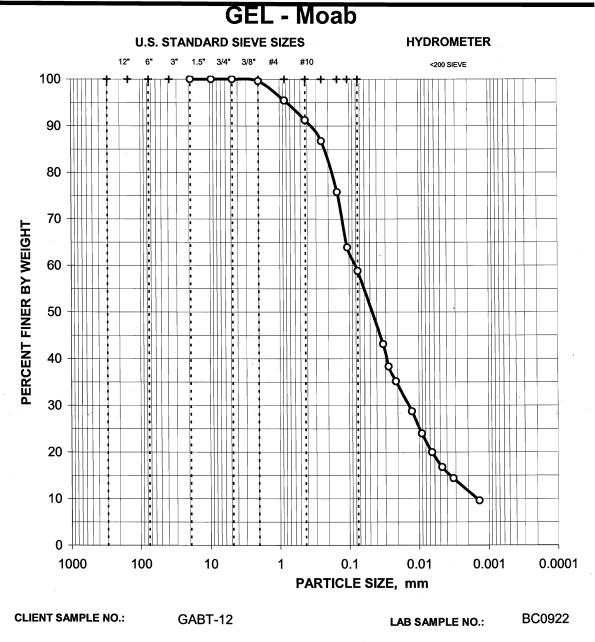
	Diameter	Percent
	mm	Finer
H		
Y D	0.03236	43.1%
R	0.02703	38.3%
0	0.02122	35.1%
м	0.01262	28.7%
E T	0.00910	24.0%
Ē	0.00654	20.0%
R	0.00467	16.8%
	0.00324	14.4%
	0.00137	9.6%

0.0% Gravel

```
41.2% Sand
```

58.8% Silt/Clay

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B O C U O		GR	AVEL		SAN	D	
L D E R S	B L E S	C O A R S E	FINE	COARSE	M E D I U M	F I N E	Silt/Clay

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#### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### PARTICLE-SIZE ANALYSIS ASTM D 422

SIEVE ANALYSIS

Project Name GEL - Moab Project No. 109855.01310000

Specific Gravity = 2.65 assumed Client Sample No. GABT-13 Lab Sample No. BC0923

Moisture Content = 26.9% based on dry sample weight

#### Sieve Diameter Percent No. Finer mm С 3" 75.000 100.0% 0 Α. 1.5" 37.500 100.0% R 0.75" 19.000 100.0% S 0.375" 9.500 100.0% E #4 4.750 100.0% #10 2.000 99.9%

	Sieve	Diameter	Percent
	No.	mm	Finer
F	#20	0.850	99.8%
1	#40	0.425	99.2%
NE	#60	0.250	98.3%
	#100	0.149	96.0%
	#140	0.106	92.8%
	#200	0.075	88.3%

#### HYDROMETER ANALYSIS

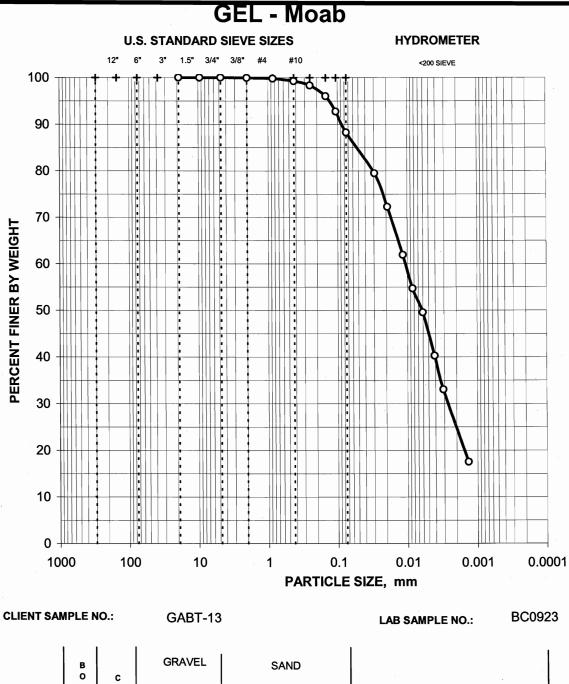
	Diameter	Percent
	mm	Finer
H		
Y D	0.04030	86.7%
R	0.02943	79.5%
0	0.01918	72.3%
м	0.01154	61.9%
E T	0.00839	54.7%
E	0.00604	49.5%
R	0.00411	40.3%
	0.00309	33.0%
	0.00135	17.5%

0.0% Gravel

```
11.7% Sand
```

88.3% Silt/Clay

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B O U	C O	GR	AVEL		SAN	)	
L D E R S	B L E S	C O A R S E	F I N E	C O A R S E	M E U M	F I N E	Silt/Clay

Page 34 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

## Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### PARTICLE-SIZE ANALYSIS ASTM D 422

SIEVE ANALYSIS

Project Name GEL - Moab Project No. 109855.01310000

Specific Gravity = 2.65 assumed Client Sample No. GABT-14 Lab Sample No. BC0924

Moisture Content = 17.8% based on dry sample weight

	Sieve	Diameter	Percent
С	No.	mm	Finer
0	3"	75.000	100.0%
А	1.5"	37.500	100.0%
R	0.75"	19.000	100.0%
S E	0.375"	9.500	100.0%
	#4	4.750	100.0%
	#10	2.000	100.0%

	Sieve	Diameter	Percent
	No.	mm	Finer
F	#20	0.850	98.5%
1	#40	0.425	96.5%
N	#60	0.250	95.0%
E	#100	0.149	92.8%
	#140	0.106	89.4%
	#200	0.075	83.6%

#### HYDROMETER ANALYSIS

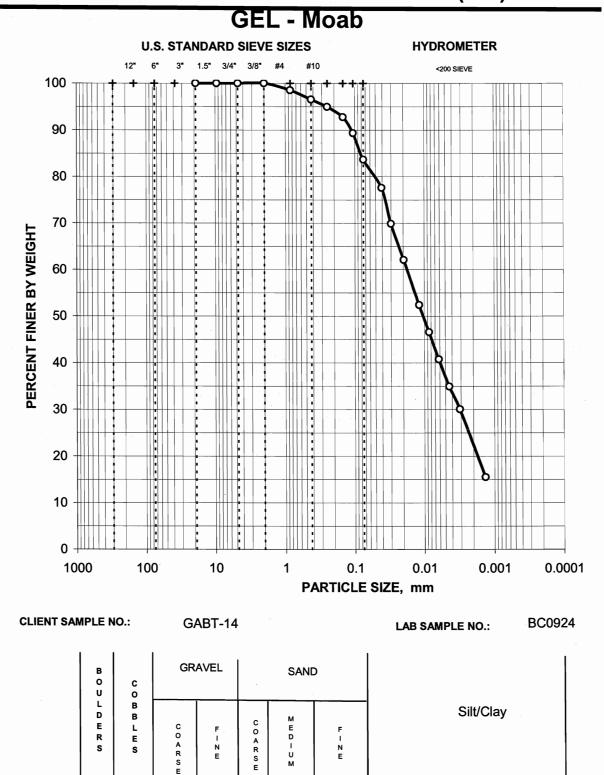
	Diameter	Percent
	mm	Finer
H		
Y D	0.04103	77.6%
R	0.03001	69.9%
0	0.01969	62.1%
M E T	0.01182	52.4%
	0.00855	46.6%
T E	0.00618	40.7%
R	0.00441	34.9%
	0.00310	30.1%
	0.00135	15.5%

0.0% Gravel

```
16.4% Sand
```

83.6% Silt/Clay

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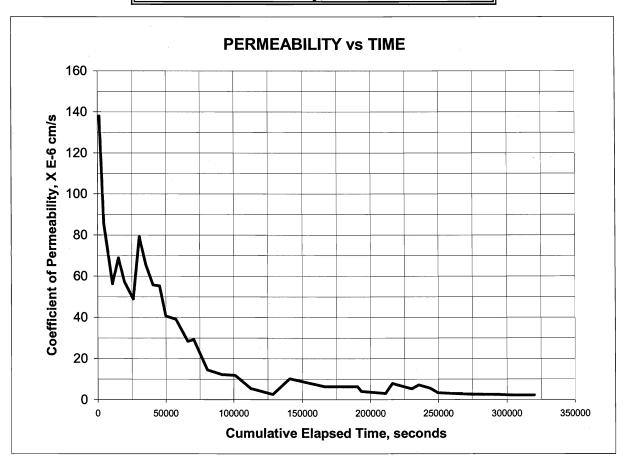
Page 36 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

## Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

# HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab PROJECT NO. 109855.014	CLIENT SAMPLE NO. GATB-01 LAB SAMPLE NO. BC0911				
		DOUDT	•		
	INITIAL				
Specimen diameter, cm	4.96		Cell confining pressure,	psi	20
Specimen length, cm	10.12		Hydraulic gradient		3.5
Wet weight of specimen, g.	356.39		Min. consolidation stress, psi		2.0
Specimen cross-sect. area, cm/	2 19.34		Max. consolidation stres	3.0	
Water content, %	7.0		Total backpressure, psi		17.0
Wet unit weight, pcf	113.7				
Dry unit weight, pcf	106.3		Permeant Fluid	Deaire	d DI Water
Degree of saturation, %	33.4	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 4.7E-06



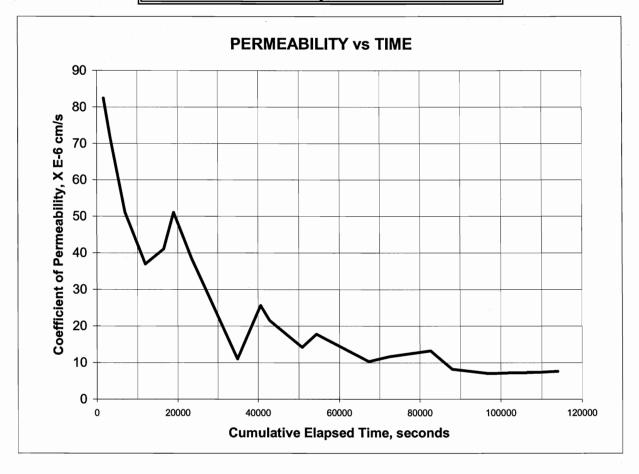
Page 37 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab		CLIENT SAMPLE NO. GATB-01		
PROJECT NO. 109855.014	1000	LAB SAMPLE NO. BC0911		
	INITIAL			
Specimen diameter, cm	4.96	Cell confining pressure,	psi	40
Specimen length, cm	10.12	Hydraulic gradient		7.0
Wet weight of specimen, g.	356.39	Min. consolidation stress	2.0	
Specimen cross-sect. area, cm ⁴ 2	2 19.34	Max. consolidation stress, psi		3.0
Water content, %	7.0	Total backpressure, psi		37.0
Wet unit weight, pcf	113.7			
Dry unit weight, pcf	106.3	Permeant Fluid	Deaired	DI Water
Degree of saturation, %	33.4 estimated			
Specific gravity of solids	2.65 assumed			

#### Coefficient of Permeability, cm/s 7.6E-06



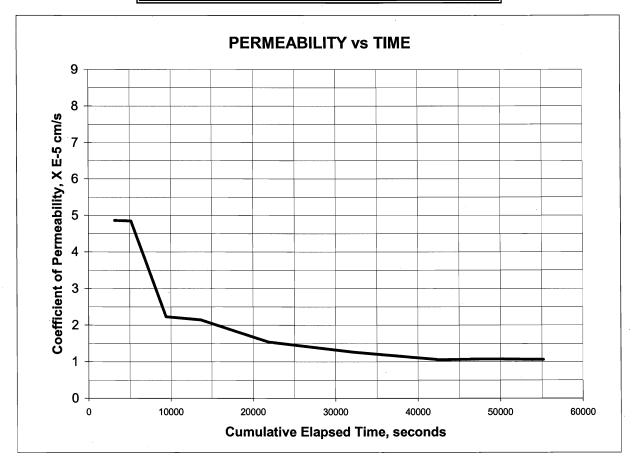
Page 38 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

# Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab	CLIENT SAMPLE NO. GATB-01		01		
PROJECT NO. 109855.0141000			LAB SAMPLE NO.	BC0911	l j
	INITIAL				
Specimen diameter, cm	4.96		Cell confining pressure,	psi	60
Specimen length, cm	10.12		Hydraulic gradient		7.0
Wet weight of specimen, g.	356.39		Min. consolidation stress	s, psi	2.0
Specimen cross-sect. area, cm ²	2 19.34		Max. consolidation stress, psi 3.0		
Water content, %	7.0		Total backpressure, psi		57.0
Wet unit weight, pcf	113.7				
Dry unit weight, pcf	106.3		Permeant Fluid	Deaired	DI Water
Degree of saturation, %	33.4	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 1.1E-05



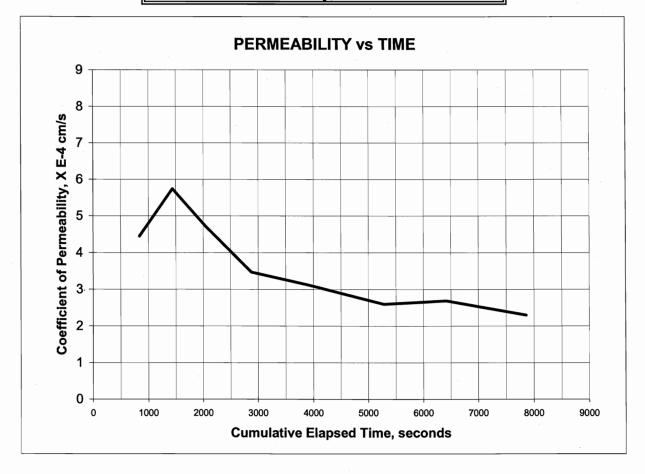
Page 39 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

# HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab PROJECT NO. 109855.01	CLIENT SAMPLE NO. GABT-03 LAB SAMPLE NO. BC0913				
	INITIAL				
Specimen diameter, cm	4.96		Cell confining pressure,	psi	20
Specimen length, cm	10.12		Hydraulic gradient		3.5
Wet weight of specimen, g.	324.39		Min. consolidation stress, psi 2		2.0
Specimen cross-sect. area, cm	^2 19.34		Max. consolidation stress, psi 2.5		
Water content, %	14.4		Total backpressure, psi		17.5
Wet unit weight, pcf	103.5				
Dry unit weight, pcf	90.5		Permeant Fluid	Deaire	d DI Water
Degree of saturation, %	46.0	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 2.7E-04



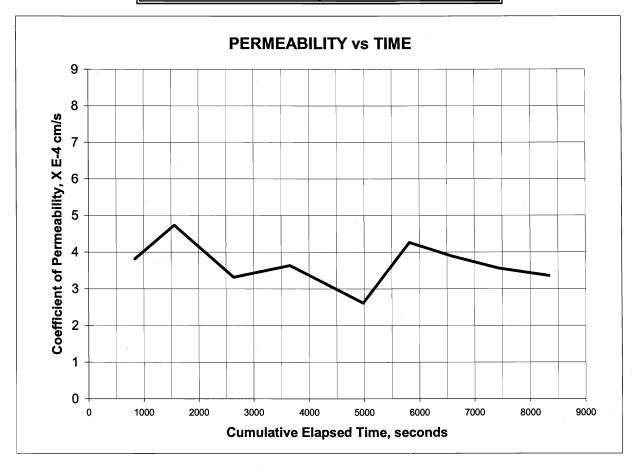
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# Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab PROJECT NO. 109855.0141000		CLIENT SAMPLE NO. GABT- LAB SAMPLE NO. BC091			
	INITIAL				
Specimen diameter, cm 4.96		Cell confining pressure, psi		40	
Specimen length, cm	10.12		Hydraulic gradient		3.5
Wet weight of specimen, g. 324.39		Min. consolidation stress, psi		2.0	
Specimen cross-sect. area, cm^2 19.34			Max. consolidation stress, psi 2.5		
Water content, %	14.4		Total backpressure, psi		37.5
Wet unit weight, pcf	103.5				
Dry unit weight, pcf	90.5		Permeant Fluid	Deaired	d DI Water
Degree of saturation, %	46.0	estimated			
Specific gravity of solids	2.650	assumed			

#### Coefficient of Permeability, cm/s 3.8E-04



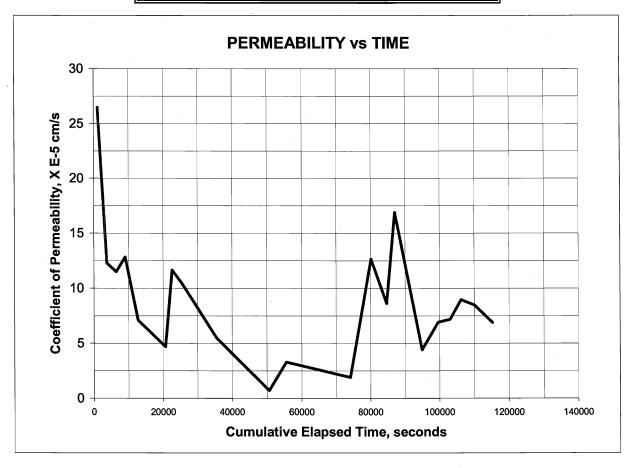
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# Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

# HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab PROJECT NO. 109855.0141000			CLIENT SAMPLE NO. LAB SAMPLE NO.		
	INITIAL				
Specimen diameter, cm 4.96		Cell confining pressure, psi		60	
Specimen length, cm 10.12			Hydraulic gradient		3.5
Wet weight of specimen, g. 324.39			Min. consolidation stress, psi		2.0
Specimen cross-sect. area, cm^2 19.34			Max. consolidation stress, psi 2.5		
Water content, %	14.4		Total backpressure, psi		57.5
Wet unit weight, pcf	103.5				
Dry unit weight, pcf	90.5		Permeant Fluid	Deaire	d DI Water
Degree of saturation, %	46.0	estimated			
Specific gravity of solids	2.650	assumed	Test readings did not meet the stability requirements of ASTM D 5084		

#### Coefficient of Permeability, cm/s 7.9E-05



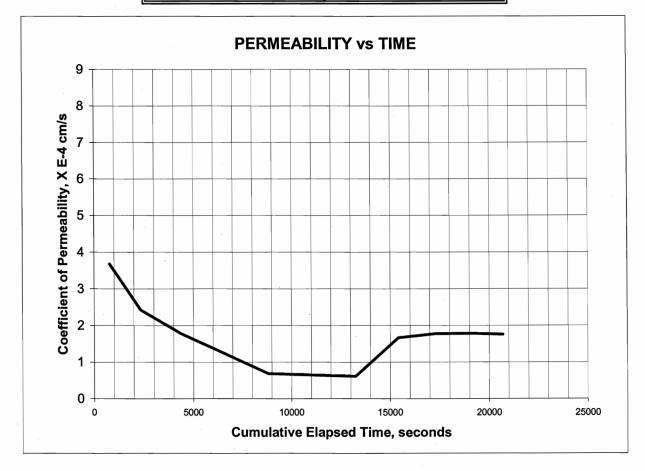
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## Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

## HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab PROJECT NO. 109855.0141000		CLIENT SAMPLE NO. GATB- LAB SAMPLE NO. BC091			
	INITIAL				
Specimen diameter, cm	4.96		Cell confining pressure,	psi	20
Specimen length, cm	10.12		Hydraulic gradient		3.5
Wet weight of specimen, g.	et weight of specimen, g. 324.78		Min. consolidation stress, psi		2.0
Specimen cross-sect. area, cm ² 19.34		Max. consolidation stress, psi 2.5		2.5	
Water content, %	17.5		Total backpressure, psi		17.5
Wet unit weight, pcf	103.6				
Dry unit weight, pcf	88.2		Permeant Fluid	Deaired	d DI Water
Degree of saturation, %	53.0	estimated			
Specific gravity of solids	2.650	assumed			

#### Coefficient of Permeability, cm/s 1.7E-04



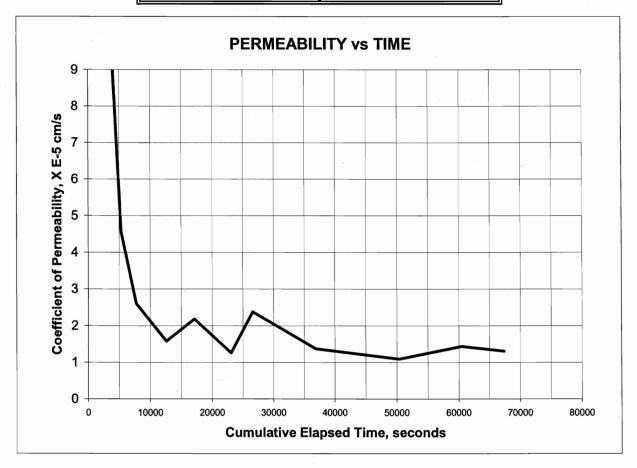
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## Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

# HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab PROJECT NO. 109855.0141000		CLIENT SAMPLE NO. LAB SAMPLE NO.	GATB-( BC0914		
	INITIAL				
Specimen diameter, cm	4.96		Cell confining pressure,	psi	40
Specimen length, cm 10.12			Hydraulic gradient		7.0
Wet weight of specimen, g. 324.78			Min. consolidation stress, psi		2.0
Specimen cross-sect. area, cm^2 19.34		Max. consolidation stress, psi		3.0	
Water content, %	17.5		Total backpressure, psi		37.0
Wet unit weight, pcf	103.6				
Dry unit weight, pcf	88.2		Permeant Fluid	Deaired	d DI Water
Degree of saturation, %	53.0	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 1.3E-05



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Specific gravity of solids

#### **Shaw Geotechnical** Laboratory **Oak Ridge TN** (865) 482-6497

GATB-04

BC0914

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY **ASTM D 5084**

PROJECT NAME: GEL Moab CLIENT SAMPLE NO. PROJECT NO. 109855.0141000 LAB SAMPLE NO. INITIAL Specimen diameter, cm 4.96 Specimen length, cm 10.12 Wet weight of specimen, g. 324.78 Specimen cross-sect. area, cm² 19.34 Water content, % 17.5 Wet unit weight, pcf 103.6 Dry unit weight, pcf 88.2 Degree of saturation, % 53.0 estimated

2.65

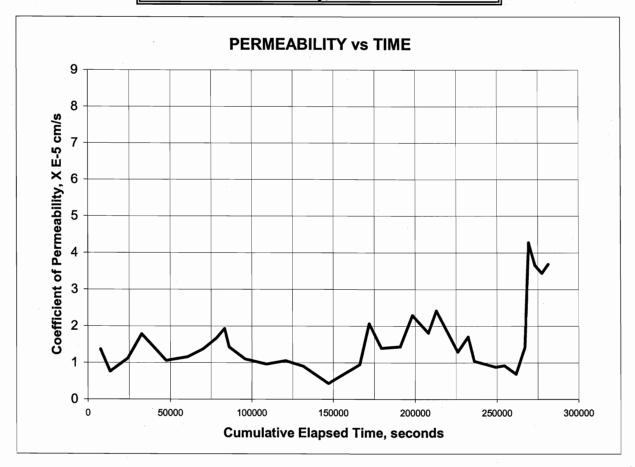
Cell confining pressure, psi	60
Hydraulic gradient	7.0
Min. consolidation stress, psi	2.0
Max. consolidation stress, psi	3.0
Total backpressure, psi	57.0

Permeant Fluid

**Deaired DI Water** 

#### Coefficient of Permeability, cm/s 1.8E-05

assumed



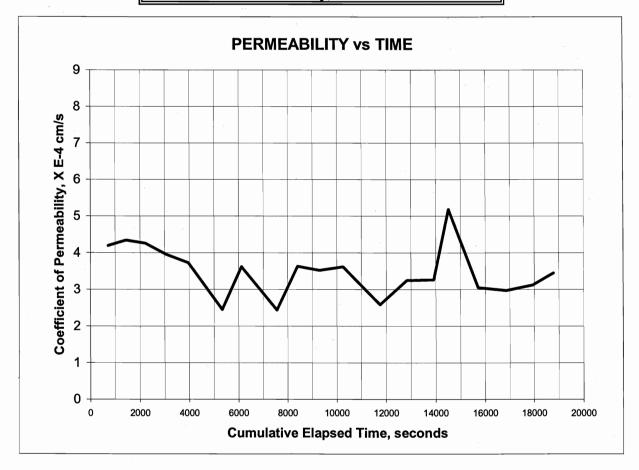
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### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: PROJECT NO.				CLIENT SAMPLE NO. GATB LAB SAMPLE NO. BC091		
		INITIAL				
Specimen diameter, cm		4.96		Cell confining pressure, psi		20
Specimen length, cm		10.12		Hydraulic gradient		3.5
Wet weight of specimen, g.		367.42		Min. consolidation stress, psi		2.0
Specimen cross-se	ct. area, cm^2	2 19.34		Max. consolidation stress, psi 2.5		
Water content, %		15.3		Total backpressure, psi		17.5
Wet unit weight, pc	f	117.3				
Dry unit weight, pcf		101.7		Permeant Fluid	Deaired	DI Water
Degree of saturatio	n, %	64.6	estimated			
Specific gravity of s	olids	2.65	assumed			

#### Coefficient of Permeability, cm/s 3.1E-04



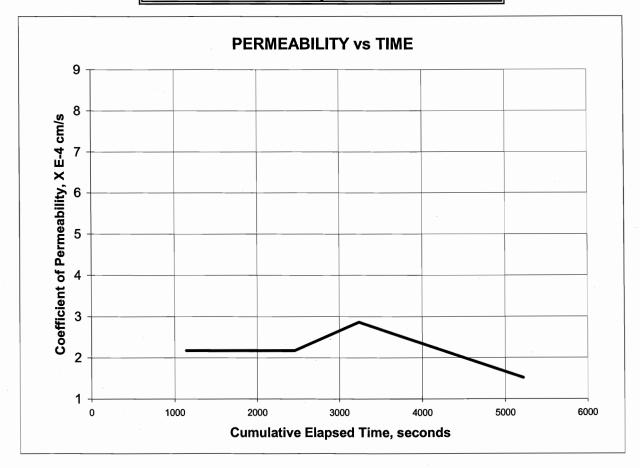
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### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab PROJECT NO. 109855.0141000			CLIENT SAMPLE NO. GATB-05 LAB SAMPLE NO. BC0915		
	INITIAL				
Specimen diameter, cm	4.96		Cell confining pressure,	psi	40
Specimen length, cm	10.12		Hydraulic gradient		3.5
Wet weight of specimen, g.	367.42		Min. consolidation stress, psi		2.0
Specimen cross-sect. area, cm ^A	2 19.34		Max. consolidation stress, psi 2.5		
Water content, %	15.3		Total backpressure, psi		37.5
Wet unit weight, pcf	117.3				
Dry unit weight, pcf	101.7		Permeant Fluid	Deaired	DI Water
Degree of saturation, %	64.6	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 2.2E-04



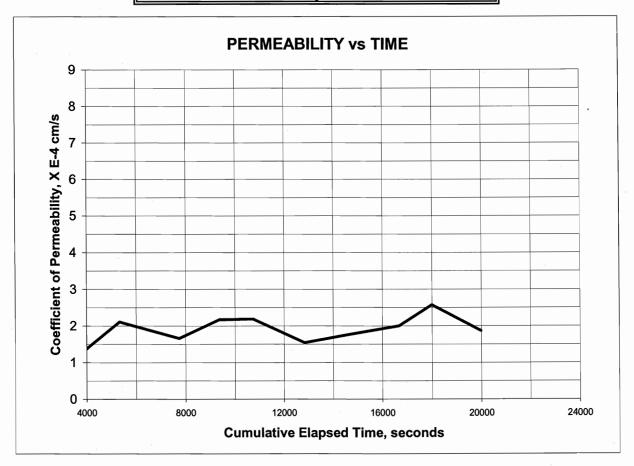
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### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab PROJECT NO. 109855.0141000			CLIENT SAMPLE NO. GATB- LAB SAMPLE NO. BC091		
	INITIAL				
Specimen diameter, cm	4.96		Cell confining pressure,	psi	60
Specimen length, cm	10.12		Hydraulic gradient		3.5
Wet weight of specimen, g.	367.42		Min. consolidation stress, psi		2.0
Specimen cross-sect. area, cm^	2 19.34		Max. consolidation stress, psi 2.5		2.5
Water content, %	15.3		Total backpressure, psi		57.5
Wet unit weight, pcf	117.3				
Dry unit weight, pcf	101.7		Permeant Fluid	Deaired	d DI Water
Degree of saturation, %	64.6	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 2.1E-04



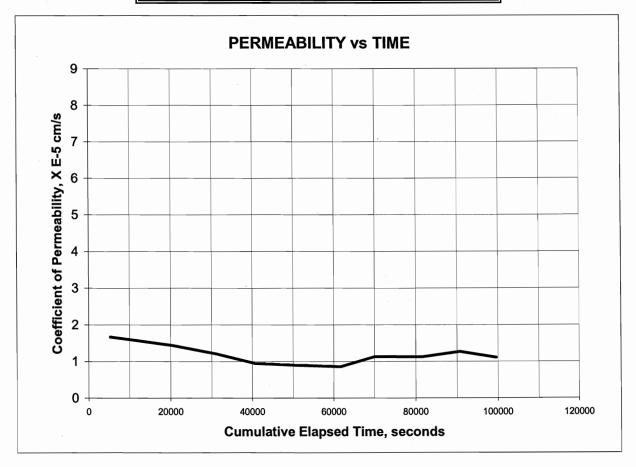
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### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

			CLIENT SAMPLE NO. GABT-07 LAB SAMPLE NO. BC0917		
	INITIAL				
Specimen diameter, cm	4.96		Cell confining pressure, psi		20
Specimen length, cm	10.12		Hydraulic gradient		7.0
Wet weight of specimen, g.	363.52		Min. consolidation stress, psi		2.0
Specimen cross-sect. area, cr	n^2 19.34		Max. consolidation stress, psi 3.0		
Water content, %	20.5		Total backpressure, psi		17.0
Wet unit weight, pcf	116.0				
Dry unit weight, pcf	96.3		Permeant Fluid	Deaire	d DI Water
Degree of saturation, %	75.6	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 1.2E-05



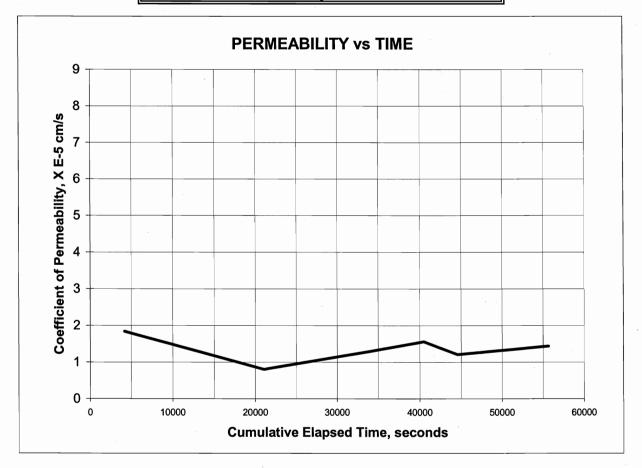
Page 49 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

#### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab CLIENT SAMPLE NO. GABT-07 PROJECT NO. 109855.0141000 LAB SAMPLE NO. BC0917 INITIAL Specimen diameter, cm 4.96 Cell confining pressure, psi 40 Specimen length, cm 7.0 10.12 Hydraulic gradient Wet weight of specimen, g. 363.52 Min. consolidation stress, psi 2.0 Max. consolidation stress, psi Specimen cross-sect. area, cm² 19.34 3.0 Water content, % 20.5 Total backpressure, psi 37.0 Wet unit weight, pcf 116.0 Dry unit weight, pcf 96.3 Permeant Fluid **Deaired DI Water** Degree of saturation, % 75.6 estimated Specific gravity of solids 2.65 assumed

#### Coefficient of Permeability, cm/s 1.4E-05



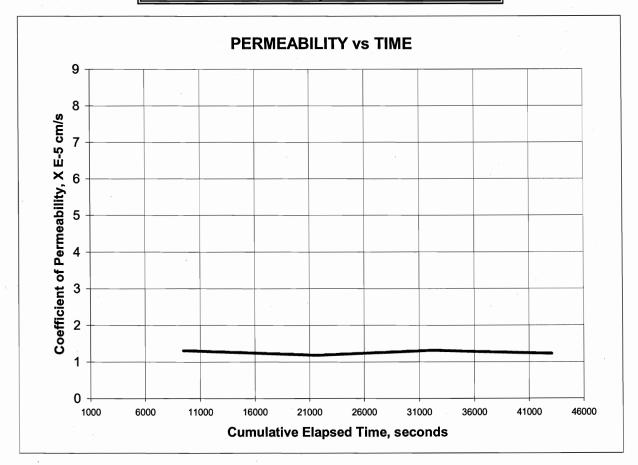
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### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab PROJECT NO. 109855.0141000			CLIENT SAMPLE NO. GABT-07 LAB SAMPLE NO. BC0917		
FROJECT NO. 109855.01	141000		LAD SAMFLE NO.	DC091	1
	INITIAL				
Specimen diameter, cm	4.96		Cell confining pressure,	psi	60
Specimen length, cm	en length, cm 10.12 Hydraulic gradient		7.0		
Wet weight of specimen, g.	363.52		Min. consolidation stress, psi		2.0
Specimen cross-sect. area, cm	1 <b>^2 19</b> .34		Max. consolidation stress, psi 3.0		
Water content, %	20.5		Total backpressure, psi		57.0
Wet unit weight, pcf	116.0				
Dry unit weight, pcf	96.3		Permeant Fluid	Deaire	d DI Water
Degree of saturation, %	75.6	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 1.3E-05



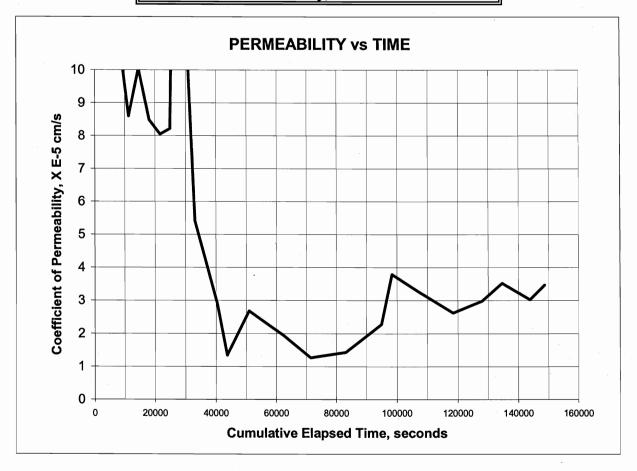
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### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab PROJECT NO. 109855.0141000			CLIENT SAMPLE NO. GABT-08 LAB SAMPLE NO. BC0918		
	INITIAL	]			
Specimen diameter, cm	4.96	-	Cell confining pressure	, psi	20
Specimen length, cm	10.12		Hydraulic gradient		3.5
Wet weight of specimen	i, g. 374.76		Min. consolidation stress, psi		2.0
Specimen cross-sect. a	rea, cm^2 19.34		Max. consolidation stress, psi 2.5		
Water content, %	17.9		Total backpressure, psi		17.5
Wet unit weight, pcf	119.6				
Dry unit weight, pcf	101.4		Permeant Fluid	Deaire	d DI Water
Degree of saturation, %	75.2	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 3.2E-05



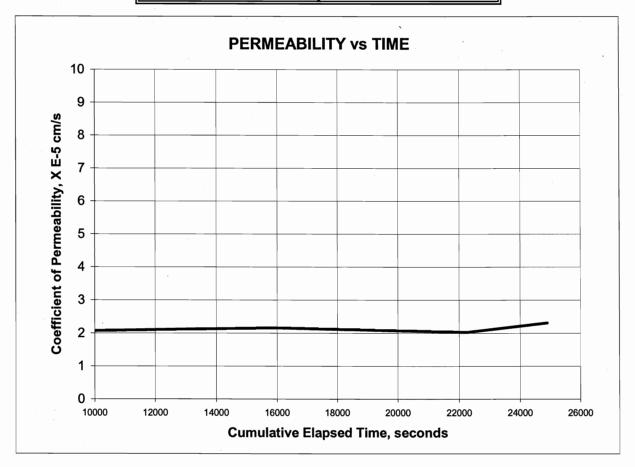
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### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab			CLIENT SAMPLE NO.	GABT-0	-
PROJECT NO. 109855.014	1000		LAB SAMPLE NO.	BC0918	<b>}</b>
	INITIAL				
Specimen diameter, cm	4.96		Cell confining pressure,	psi	40
Specimen length, cm	10.12		Hydraulic gradient		7.0
Wet weight of specimen, g. 374.76			Min. consolidation stress, psi		2.0
Specimen cross-sect. area, cm ²	2 19.34		Max. consolidation stress, psi 3.0		
Water content, %	17.9		Total backpressure, psi		37.0
Wet unit weight, pcf	119.6				
Dry unit weight, pcf	101.4		Permeant Fluid	Deaired	I DI Water
Degree of saturation, %	75.2	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 2.1E-05



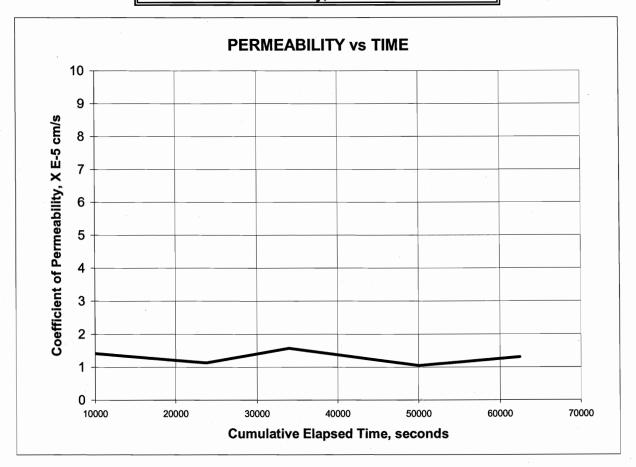
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#### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab CLIENT SAMPLE NO. GABT-08 PROJECT NO. 109855.0141000 LAB SAMPLE NO. BC0918 INITIAL Specimen diameter, cm Cell confining pressure, psi 60 4.96 7.0 Specimen length, cm 10.12 Hydraulic gradient 2.0 Wet weight of specimen, g. Min. consolidation stress, psi 374.76 Specimen cross-sect. area, cm² 19.34 Max. consolidation stress, psi 3.0 57.0 Water content, % Total backpressure, psi 17.9 Wet unit weight, pcf 119.6 **Deaired DI Water** Permeant Fluid Dry unit weight, pcf 101.4 Degree of saturation, % 75.2 estimated Specific gravity of solids 2.65 assumed

#### Coefficient of Permeability, cm/s 7.4E-05



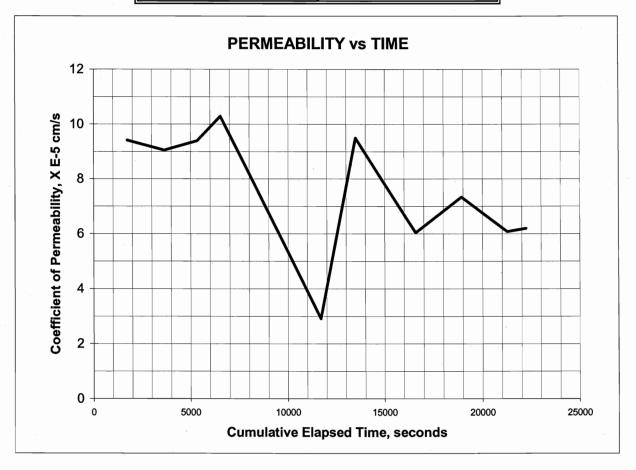
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### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab			CLIENT SAMPLE NO.	GATB	-09
PROJECT NO. 10985	5.0141000		LAB SAMPLE NO. BC0919		19
	INITIAL	]			
Specimen diameter, cm	4.96		Cell confining pressure	, psi	20
Specimen length, cm	10.12		Hydraulic gradient		7.0
Wet weight of specimen, g	. 353.83		Min. consolidation stress, psi		2.0
Specimen cross-sect. area	, cm^2 19.34		Max. consolidation stress, psi 3.0		
Water content, %	23.0		Total backpressure, psi		17.0
Wet unit weight, pcf	112.9				
Dry unit weight, pcf	91.8		Permeant Fluid	Deaire	ed DI Water
Degree of saturation, %	75.9	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 6.4E-05



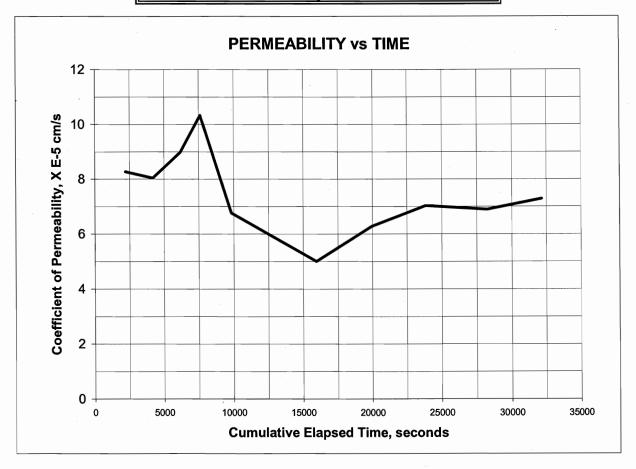
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### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: PROJECT NO.				CLIENT SAMPLE NO. GATB-09 LAB SAMPLE NO. BC0919		•
		INITIAL				
Specimen diameter	, cm	4.96		Cell confining pressure, psi		40
Specimen length, cm		10.12		Hydraulic gradient		7.0
Wet weight of specimen, g.		353.83		Min. consolidation stress, psi		2.0
Specimen cross-se	ct. area, cm^2	2 19.34		Max. consolidation stress, psi 2.5		
Water content, %		23.0		Total backpressure, psi		37.5
Wet unit weight, pc	f	112.9				
Dry unit weight, pcf		91.8		Permeant Fluid	Deaired	I DI Water
Degree of saturatio	n, %	75.9	estimated			
Specific gravity of s	olids	2.65	assumed			

#### Coefficient of Permeability, cm/s 6.9E-05



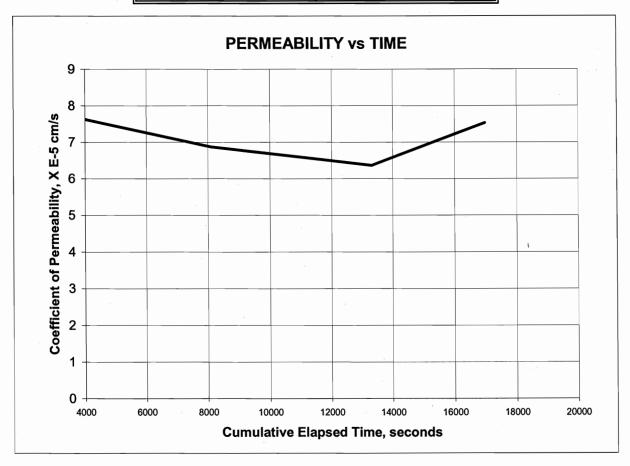
Page 56 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

#### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab	GEL Moab C		CLIENT SAMPLE NO.	GATB-0	)9	
PROJECT NO. 109855.014	1000		LAB SAMPLE NO. BC091		19	
		1				
	INITIAL					
Specimen diameter, cm	4.96		Cell confining pressure,	psi	60	
Specimen length, cm	10.12		Hydraulic gradient		3.5	
Wet weight of specimen, g.	353.83		Min. consolidation stress, psi		2.0	
Specimen cross-sect. area, cm	<b>`</b> 2 19.34		Max. consolidation stress, psi 2.5			
Water content, %	23.0		Total backpressure, psi		57.5	
Wet unit weight, pcf	112.9					
Dry unit weight, pcf	91.8		Permeant Fluid	Deaired	DI Water	
Degree of saturation, %	75.9	estimated				
Specific gravity of solids	2.65	assumed				

#### Coefficient of Permeability, cm/s 7.1E-05



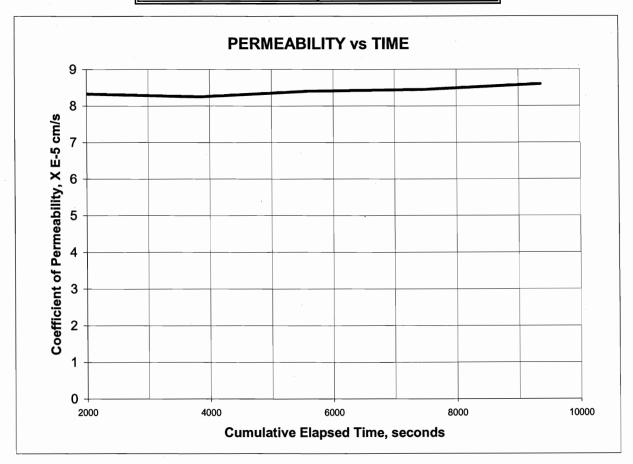
Page 57 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab PROJECT NO. 109855.0141000			CLIENT SAMPLE NO. LAB SAMPLE NO.	O. GATB-12 BC0922	
	INITIAL				
Specimen diameter, cm	4.96		Cell confining pressure,	psi	20
Specimen length, cm	10.12		Hydraulic gradient		7.0
Wet weight of specimen, g.	316.81		Min. consolidation stress, psi		2.0
Specimen cross-sect. area, cm^	2 19.34		Max. consolidation stress, psi 3.0		
Water content, %	20.9		Total backpressure, psi		17.0
Wet unit weight, pcf	101.1				
Dry unit weight, pcf	83.6		Permeant Fluid	Deaire	d DI Water
Degree of saturation, %	56.6	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 8.4E-05



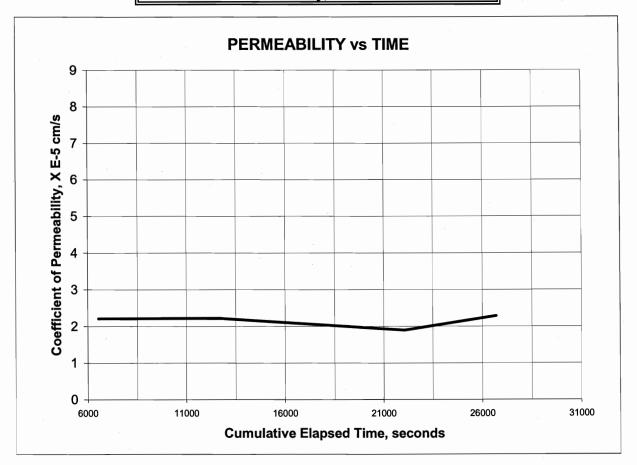
Page 58 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab PROJECT NO. 109855.014	1000		CLIENT SAMPLE NO. LAB SAMPLE NO.	GATB-1 BC0922	-
	INITIAL				
Specimen diameter, cm	4.96		Cell confining pressure,	psi	40
Specimen length, cm	10.12		Hydraulic gradient	•	7.0
Wet weight of specimen, g.	316.81		Min. consolidation stress	s, psi	2.0
Specimen cross-sect. area, cm^	2 19.34		Max. consolidation stres	s, psi	3.0
Water content, %	20.9		Total backpressure, psi		37.0
Wet unit weight, pcf	101.1				
Dry unit weight, pcf	83.6		Permeant Fluid	Deaired	I DI Water
Degree of saturation, %	56.6	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 2.1E-05



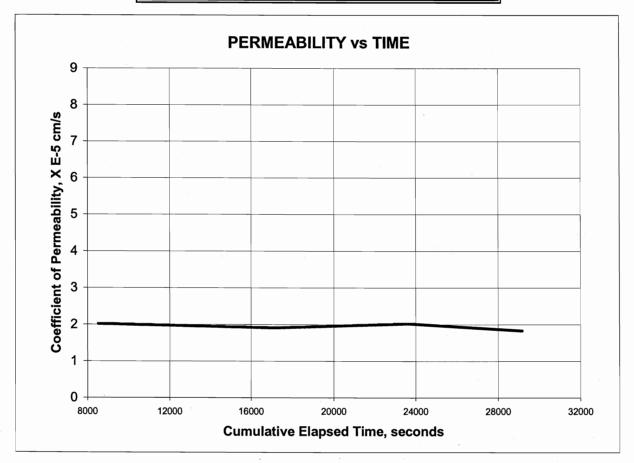
Page 59 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL M	bab		CLIENT SAMPLE NO	). GATB	-12
PROJECT NO. 109855	.0141000		LAB SAMPLE NO.	BC092	22
	INITIAL	]			
Specimen diameter, cm	4.96		Cell confining pressu	re, psi	60
Specimen length, cm	10.12		Hydraulic gradient		7.0
Wet weight of specimen, g.	316.81		Min. consolidation str	ess, psi	2.0
Specimen cross-sect. area,	cm^2 19.34		Max. consolidation st	ress, psi	3.0
Water content, %	20.9		Total backpressure,	osi	57.0
Wet unit weight, pcf	101.1				
Dry unit weight, pcf	83.6		Permeant Fluid	Deaire	ed DI Water
Degree of saturation, %	56.6	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 1.9E-05



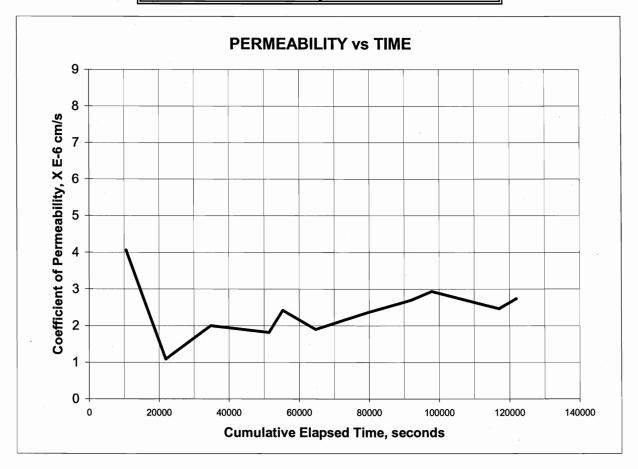
Page 60 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

#### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moa	~		CLIENT SAMPLE NO		
PROJECT NO. 109855.0	141000		LAB SAMPLE NO.	BC09	924
	INITIAL	]			
Specimen diameter, cm	4.96		Cell confining pressu	e, psi	20
Specimen length, cm	10.12		Hydraulic gradient		7.0
Wet weight of specimen, g.	312.37		Min. consolidation str	ess, psi	2.0
Specimen cross-sect. area, cr	m <b>^2 19</b> .34		Max. consolidation st	ress, psi	4.0
Water content, %	22.8		Total backpressure, p	osi	16.0
Wet unit weight, pcf	99.7				
Dry unit weight, pcf	81.2		Permeant Fluid	Deai	red DI Water
Degree of saturation, %	58.2	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 2.7E-06



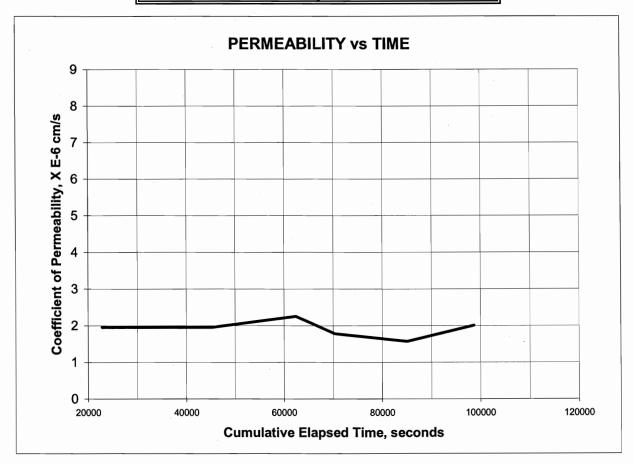
Page 61 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

#### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab PROJECT NO. 109855.014	1000		CLIENT SAMPLE NO. LAB SAMPLE NO.	GATB-1 BC0924	
	INITIAL				
Specimen diameter, cm	4.96		Cell confining pressure,	psi	40
Specimen length, cm	10.12		Hydraulic gradient		13.9
Wet weight of specimen, g.	312.37		Min. consolidation stress	s, psi	2.0
Specimen cross-sect. area, cm ²	2 19.34		Max. consolidation stres	s, psi	4.0
Water content, %	22.8		Total backpressure, psi		36.0
Wet unit weight, pcf	99.7				
Dry unit weight, pcf	81.2		Permeant Fluid	Deaired	DI Water
Degree of saturation, %	58.18	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 1.8E-06



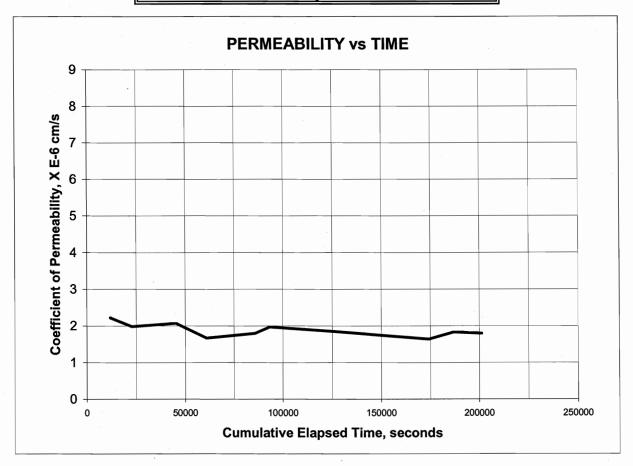
Page 62 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

### Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### HYDRAULIC CONDUCTIVITY / PERMEABILITY ASTM D 5084

PROJECT NAME: GEL Moab PROJECT NO. 109855.014	1000		CLIENT SAMPLE NO. LAB SAMPLE NO.	GATB- BC0924	
	INITIAL				
Specimen diameter, cm	4.96		Cell confining pressure,	psi	60
Specimen length, cm	10.12		Hydraulic gradient		13.9
Wet weight of specimen, g.	312.37		Min. consolidation stress	s, psi	2.0
Specimen cross-sect. area, cm^	2 19.34		Max. consolidation stres	s, psi	4.0
Water content, %	22.8		Total backpressure, psi		56.0
Wet unit weight, pcf	99.7				
Dry unit weight, pcf	81.2		Permeant Fluid	Deaired	d DI Water
Degree of saturation, %	58.18	estimated			
Specific gravity of solids	2.65	assumed			

#### Coefficient of Permeability, cm/s 1.8E-06



Page 63 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

### CAPILLARY-MOISTURE RELATIONS by POTENTIOMETER

PROJECT NAME GEL Moab

#### LAB SAMPLE NUMBER BC0914

PROJECT NUMBER 109855.01410000 CLIENT SAMPLE NUMBER GABT-04

TEST	MOISTURE, %	MOISTURE, %	SOLIDS, %	VAPOR
NUMBER	ASTM D 2216	SW846	SW846	TENSION, Mpa
1	3.1	3.0	97.0	-4.1
2	3.8	3.7	96.3	-3.3
3	4.8	4.6	95.4	-1.4
4	5.5	5.2	94.8	-1.4
5	6.6	6.2	93.8	-1.0
6	7.2	6.7	93.3	-0.6
7	8.7	8.0	92.0	-0.4
8	11.4	10.2	89.8	-0.3
9	12.0	10.7	89.3	-0.3
10	13.8	12.2	87.8	-0.3
11	16.7	14.3	85.7	-0.2
12	17.3	14.8	85.2	-0.2
13	21.8	17.9	82.1	-0.1
14	27.8	21.8	78.2	-0.1
15	29.8	22.9	77.1	-0.1
16	32.1	24.3	75.7	0.0
17				
18				
19				
20				
21				•
22				
23				
24				
25				

ASTM D 2216 results are based on dry sample weight.

SW846 results are based on wet sample weight.

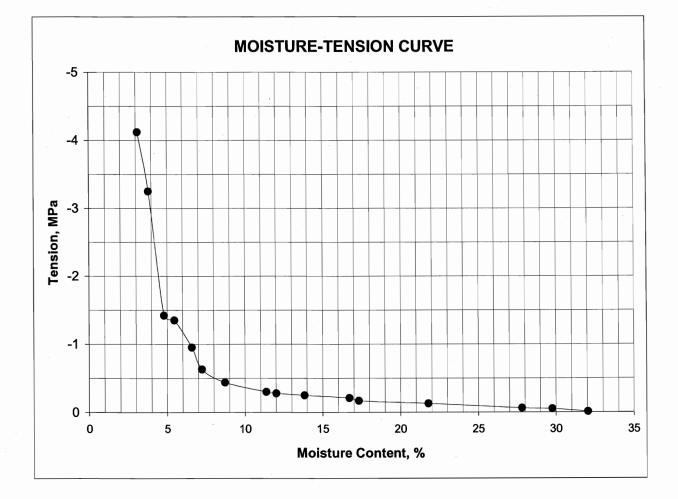
Solids content is determined by subtracting the SW846 moisture (%) from 100.

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Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

### CAPILLARY-MOISTURE RELATIONS by POTENTIOMETER

PROJECT NAME: PROJECT NO.: GEL Moab 109855.01410000 CLIENT SAMPLE NO. GABT-04 SHAW LAB SAMPLE BC0914



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Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

### CAPILLARY-MOISTURE RELATIONS by POTENTIOMETER

PROJECT NAME GEL Moab

# LAB SAMPLE NUMBER BC0916

CLIENT SAMPLE NUMBER

GABT-06

PROJECT NUMBER

109855.01410000

TEST NUMBER	MOISTURE, % ASTM D 2216	MOISTURE,% SW846	SOLIDS, % SW846	VAPOR TENSION, Mpa
1	2.9	2.9	97.1	-8.8
2	4.8	4.5	95.5	-6.4
3	4.9	4.6	95.4	-5.9
4	8.1	7.5	92.5	-3.2
5	10.7	9.7	90.3	-2.2
6	13.2	11.6	88.4	-1.7
7	16.6	14.3	85.7	-1.3
8	19.7	16.5	83.5	-1.1
9	22.6	18.4	81.6	-0.8
10	25.4	20.3	79.7	-0.6
11	28.4	22.1	77.9	-0.6
12				
13				
14				
15				
16				
17				
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19				
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22				
23				
24				
25				

ASTM D 2216 results are based on dry sample weight.

SW846 results are based on wet sample weight.

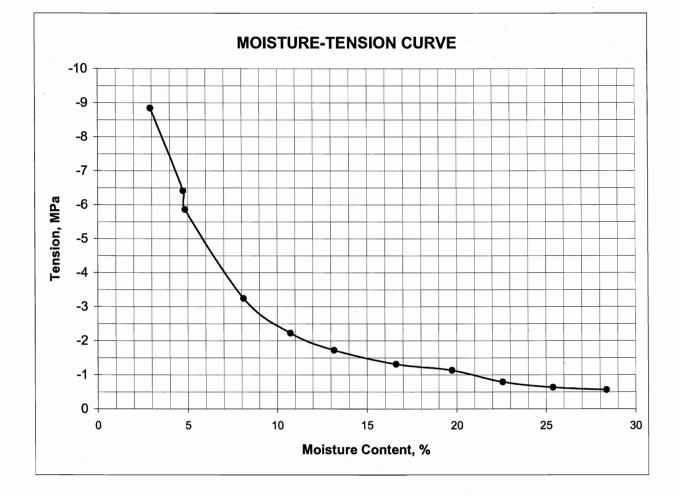
Solids content is determined by subtracting the SW846 moisture (%) from 100.

Page 66 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

### CAPILLARY-MOISTURE RELATIONS by POTENTIOMETER

PROJECT NAME: PROJECT NO.: GEL Moab 109855.01410000 CLIENT SAMPLE NO. GABT-06 SHAW LAB SAMPLE BC0916



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Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

### CAPILLARY-MOISTURE RELATIONS by POTENTIOMETER

PROJECT NAME

GEL Moab

# LAB SAMPLE NUMBER BC0919

CLIENT SAMPLE NUMBER

GABT-09

PROJECT NUMBER

109855.01410000

TEST	MOISTURE, %	MOISTURE, %	SOLIDS, %	VAPOR
NUMBER	ASTM D 2216	SW846	SULIDS, % SW846	TENSION, Mpa
1	4.3	4.1	95.9	-12.1
2	6.7	6.3	93.7	-6.4
3	8.5	7.8	92.2	-4.1
4	11.0	9.9	90.1	-3.1
5	12.5	11.1	88.9	-2.3
6	14.6	12.7	87.3	-1.7
7	18.5	15.6	84.4	-1.4
8	22.4	18.3	81.7	-1.3
9	26.9	21.2	78.8	-1.2
10	29.7	22.9	77.1	-0.9
11	31.6	24.0	76.0	-0.8
12	34.7	25.8	74.2	-0.7
13	37.6	27.3	72.7	-0.7
14	38.1	27.6	72.4	-0.7
15				
16				
17				
18				
19				
20				
21				
22				
23				
24				
25				
ASTM D 2216 resu	lto are beend on dr	a ammila suaight	•	

ASTM D 2216 results are based on dry sample weight.

SW846 results are based on wet sample weight.

Solids content is determined by subtracting the SW846 moisture (%) from 100.

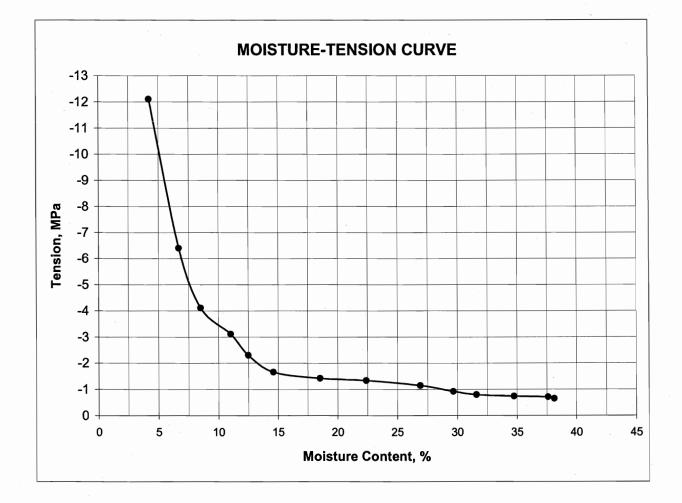
Page 68 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

## CAPILLARY-MOISTURE RELATIONS by POTENTIOMETER

PROJECT NAME: PROJECT NO.:

GEL Moab 109855.01410000 CLIENT SAMPLE NO. GABT-09 SHAW LAB SAMPLE BC0919



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Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

### CAPILLARY-MOISTURE RELATIONS by POTENTIOMETER

PROJECT NAME

GEL Moab

LAB SAMPLE NUMBER BC0920

PROJECT NUMBER 109855.01410000 CLIENT SAMPLE NUMBER GABT-10

TEST NUMBER	MOISTURE, % ASTM D 2216	MOISTURE,% SW846	SOLIDS, % SW846	VAPOR TENSION, Mpa
1	5.8	5.4	94.6	-25.6
2	7.9	7.3	92.7	-24.6
3	10.3	9.3	90.7	-18.3
4	12.4	11.0	89.0	-16.0
5	13.8	12.2	87.8	-13.6
6	16.3	14.0	86.0	-12.0
7	20.7	17.1	82.9	-9.4
8	22.4	18.3	81.7	-8.1
9	23.7	19.2	80.8	-7.5
10	25.4	20.2	79.8	-6.7
11	28.5	22.2	77.8	-5.8
12	30.8	23.6	76.4	-5.5
13				
14				
15				
16				
17				
18				
19				
20				
21				
22				
23				
24				
25	ults are based on dry			

ASTM D 2216 results are based on dry sample weight.

SW846 results are based on wet sample weight.

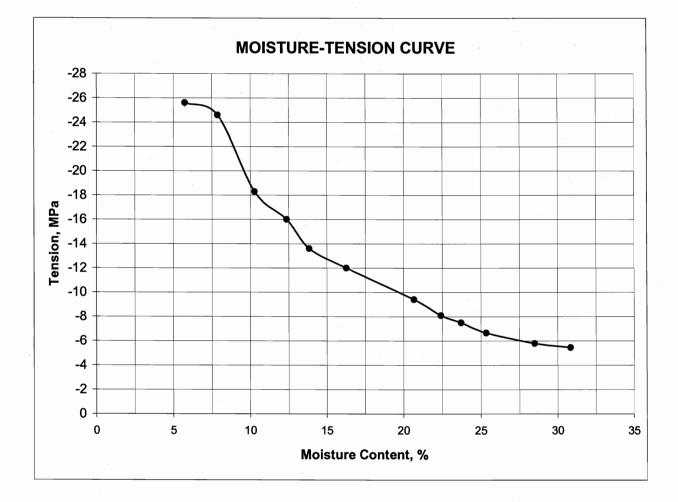
Solids content is determined by subtracting the SW846 moisture (%) from 100.

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Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

### CAPILLARY-MOISTURE RELATIONS by POTENTIOMETER

PROJECT NAME: PROJECT NO.: GEL Moab 109855.01410000 CLIENT SAMPLE NO. GABT-10 SHAW LAB SAMPLE BC0920



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Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

### CAPILLARY-MOISTURE RELATIONS by POTENTIOMETER

PROJECT NAME

GEL Moab

## LAB SAMPLE NUMBER BC0921

CLIENT SAMPLE NUMBER

GABT-11

PROJECT NUMBER

109855.01410000

TEST NUMBER	MOISTURE, % ASTM D 2216	MOISTURE , % SW846	SOLIDS, % SW846	VAPOR TENSION, Mpa
1	10.9	9.9	90.1	-10.9
2	13.1	11.6	88.4	-5.5
3	17.5	14.9	85.1	-2.7
4	21.1	17.4	82.6	-1.8
5	22.4	18.3	81.7	-1.5
6	26.4	20.9	79.1	-1.1
7	33.6	25.2	74.8	-0.9
8	37.5	27.3	72.7	-0.7
9	40.1	28.6	71.4	-0.6
10	45.2	31.1	68.9	-0.5
11	56.9	36.3	63.7	-0.5
12	60.4	37.6	62.4	-0.4
13				
14				
15				
16				
17				
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19				
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21				
22				
23				
24				
25				

ASTM D 2216 results are based on dry sample weight.

SW846 results are based on wet sample weight.

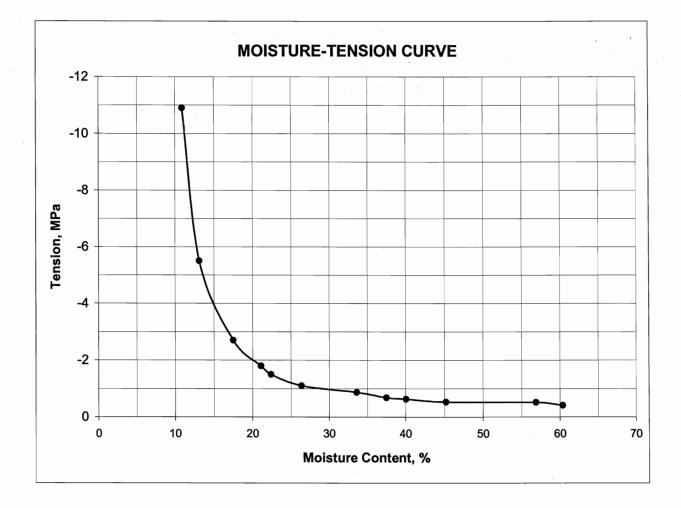
Solids content is determined by subtracting the SW846 moisture (%) from 100.

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Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

### CAPILLARY-MOISTURE RELATIONS by POTENTIOMETER

PROJECT NAME: PROJECT NO.: GEL Moab 109855.01410000 CLIENT SAMPLE NO. GABT-11 SHAW LAB SAMPLE BC0921



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Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

#### CAPILLARY-MOISTURE RELATIONS by POTENTIOMETER

PROJECT NAME GEL Moab LAB SAMPLE NUMBER BC0923

CLIENT SAMPLE NUMBER

**GABT-13** 

PROJECT NUMBER

109855.01410000

TEST	MOISTURE, %	MOISTURE,%	SOLIDS, %	VAPOR
NUMBER	ASTM D 2216	SW846	SW846	TENSION, Mpa
1	11.7	10.4	89.6	-6.0
2	15.4	13.4	86.6	-2.8
3	16.1	13.8	86.2	-2.5
4	16.8	14.4	85.6	-2.1
5	17.9	15.2	84.8	-1.9
6	20.8	17.2	82.8	-1.3
7	23.1	18.8	81.2	-1.0
8	28.1	21.9	78.1	-0.7
9	33.2	24.9	75.1	-0.5
10	39.4	28.3	71.7	-0.5
. 11	43.9	30.5	69.5	-0.4
12	51.2	33.9	66.1	-0.4
13	56.1	35.9	64.1	-0.3
14	60.5	37.7	62.3	-0.3
15				
16				
17				
18				
19				
20				
21			· · · · ·	-
22				
23				
24				
25				

ASTM D 2216 results are based on dry sample weight.

SW846 results are based on wet sample weight.

Solids content is determined by subtracting the SW846 moisture (%) from 100.

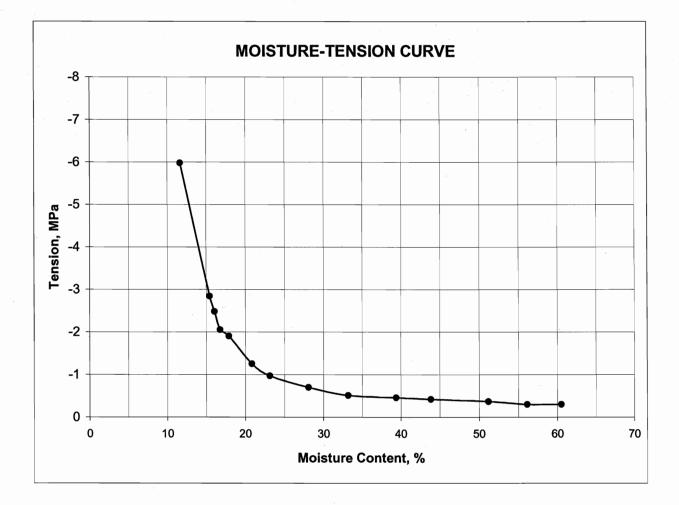
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Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

### CAPILLARY-MOISTURE RELATIONS by POTENTIOMETER

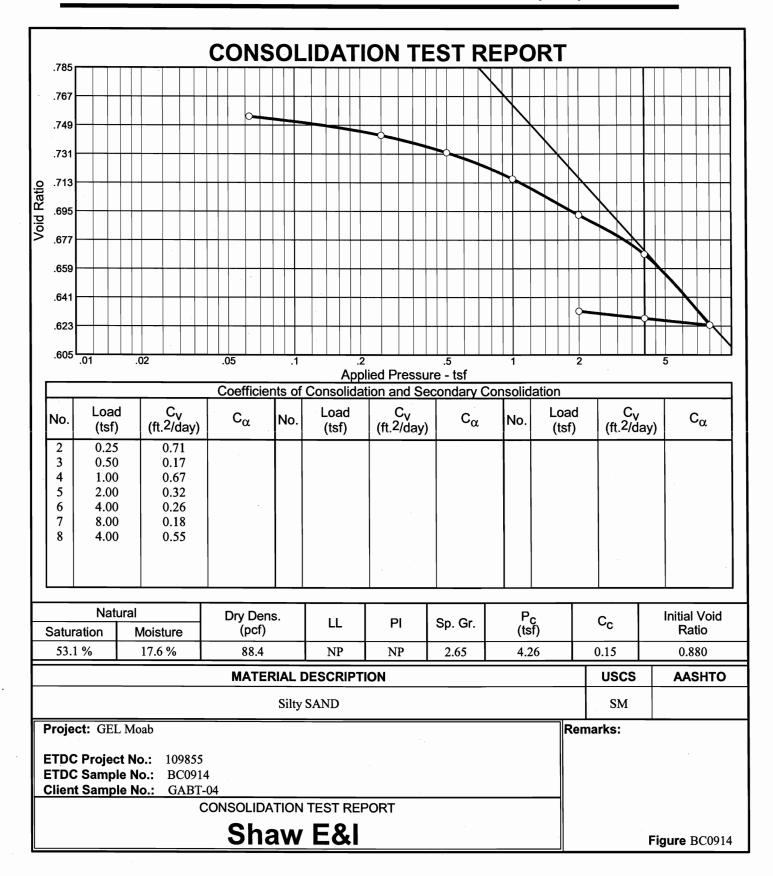
PROJECT NAME: PROJECT NO.:

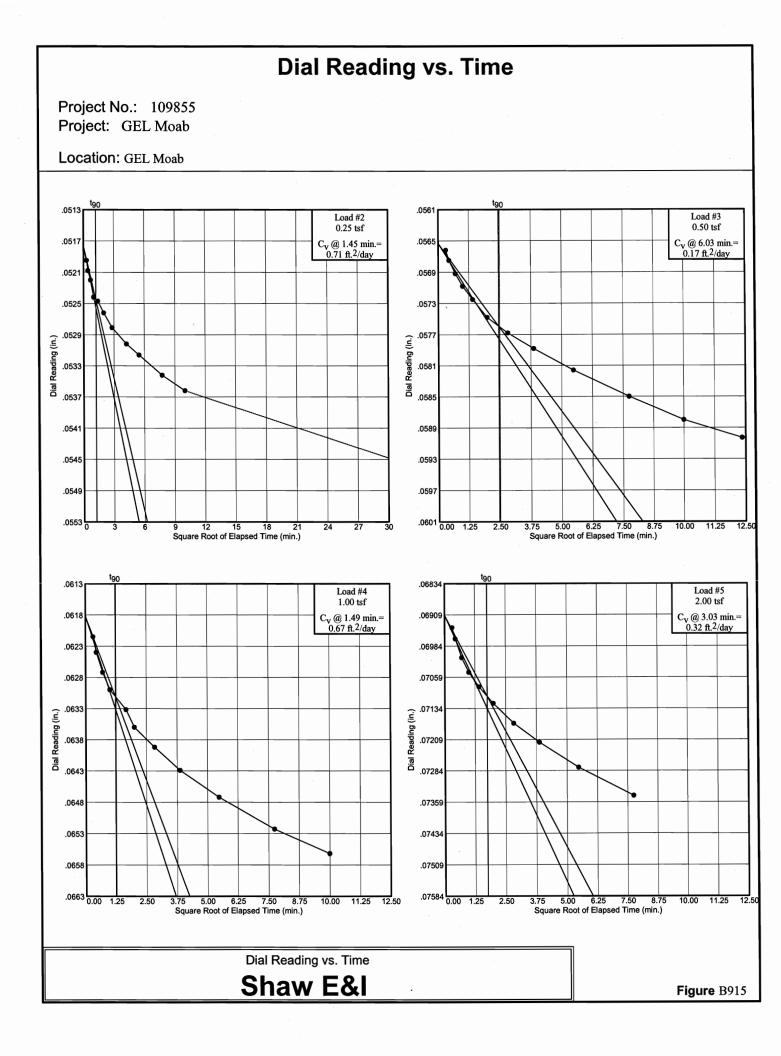
GEL Moab 109855.01410000 CLIENT SAMPLE NO. GABT-13 SHAW LAB SAMPLE BC0923

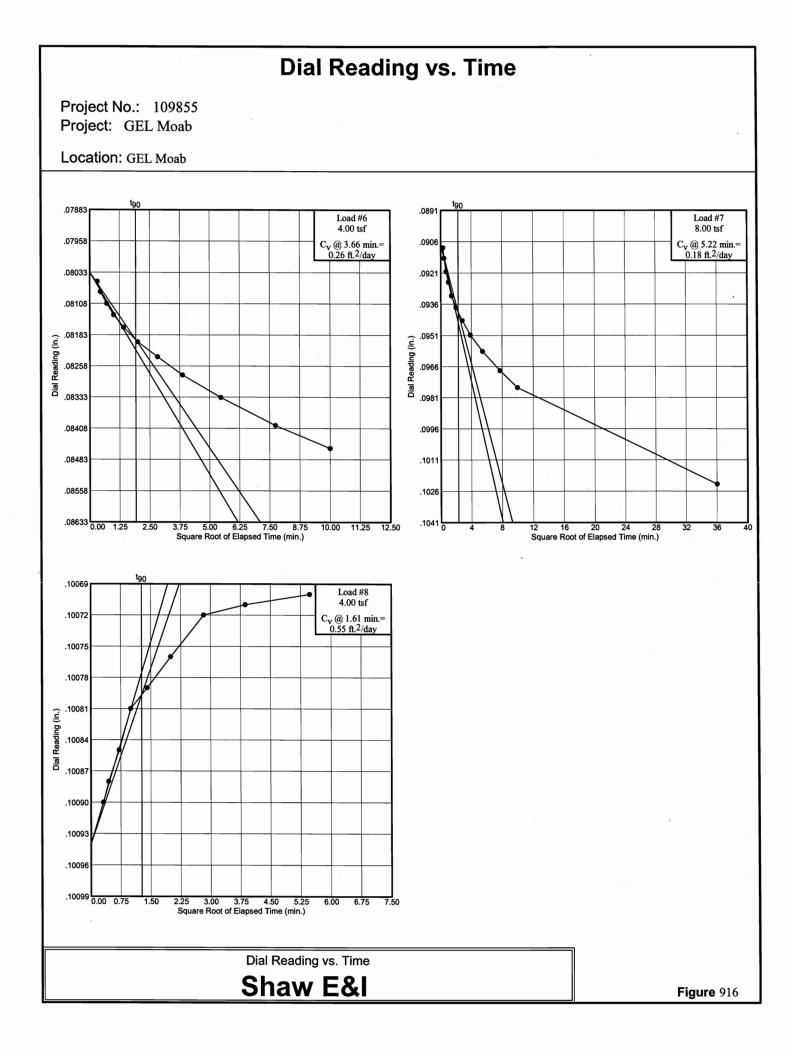


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Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

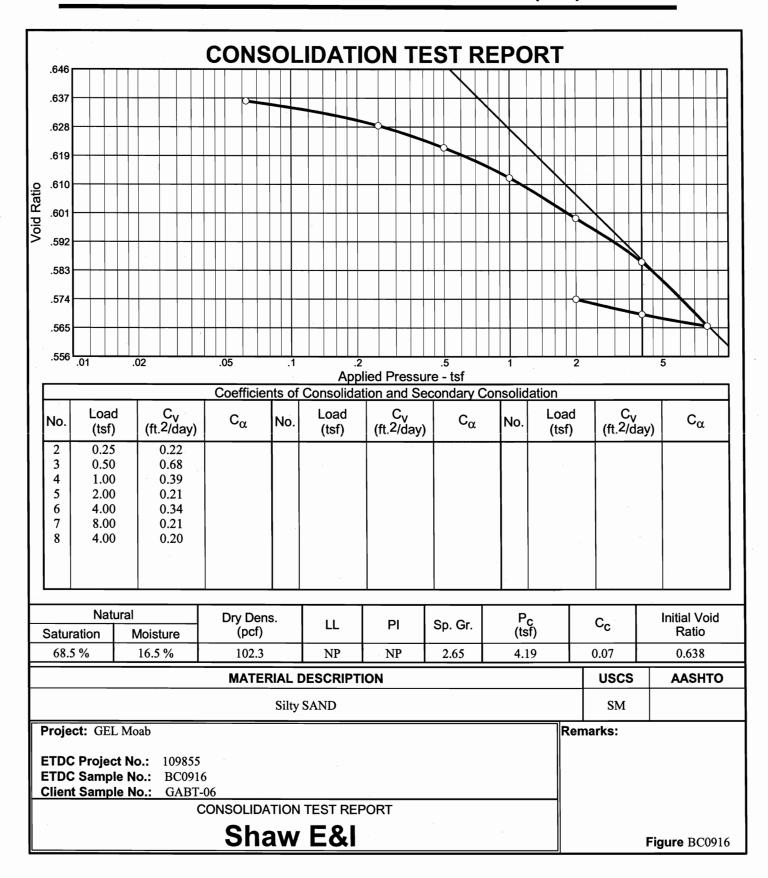


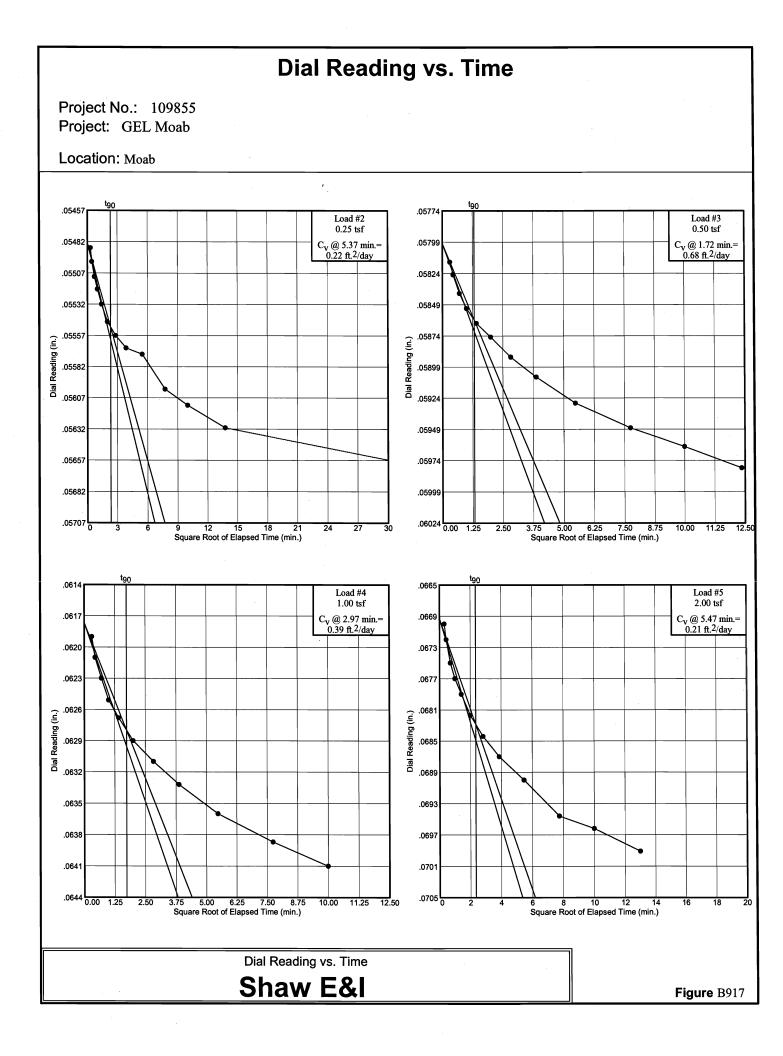




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Shaw Geotechnical Laboratory Oak Ridge TN (865) 482-6497

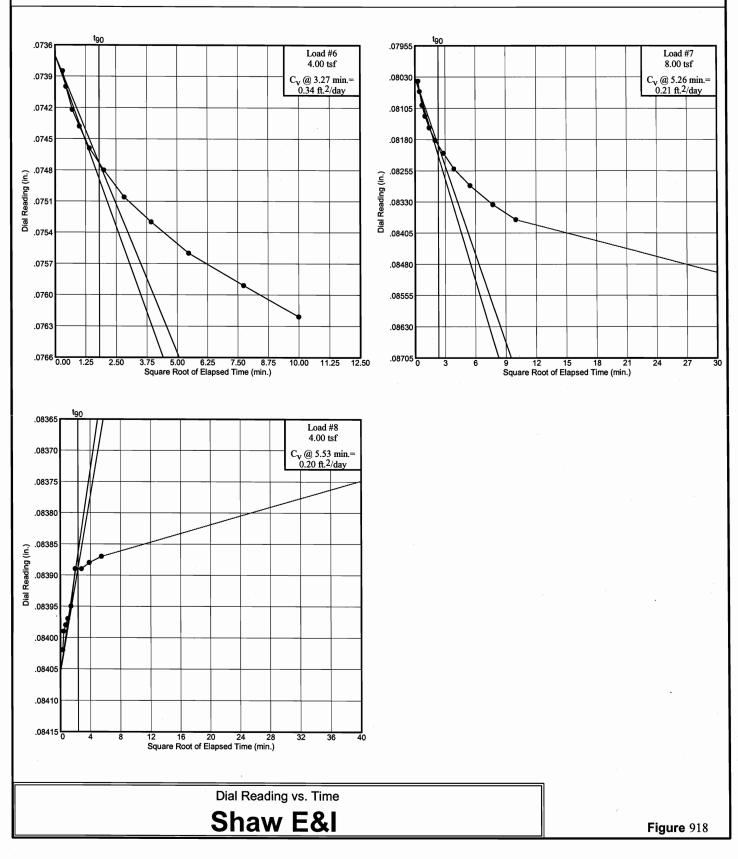




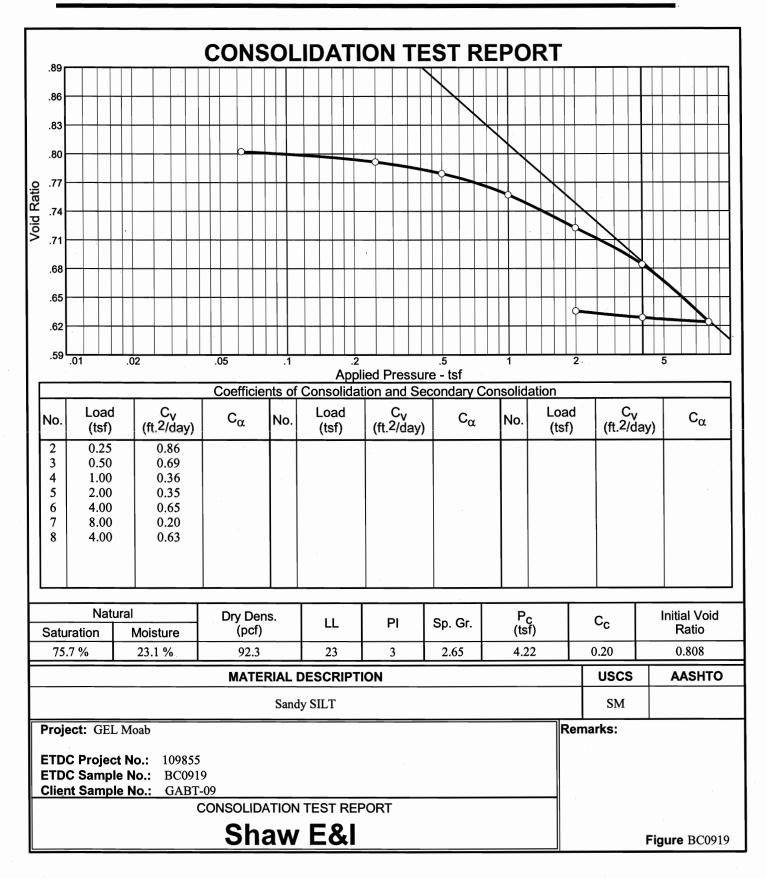
# **Dial Reading vs. Time**

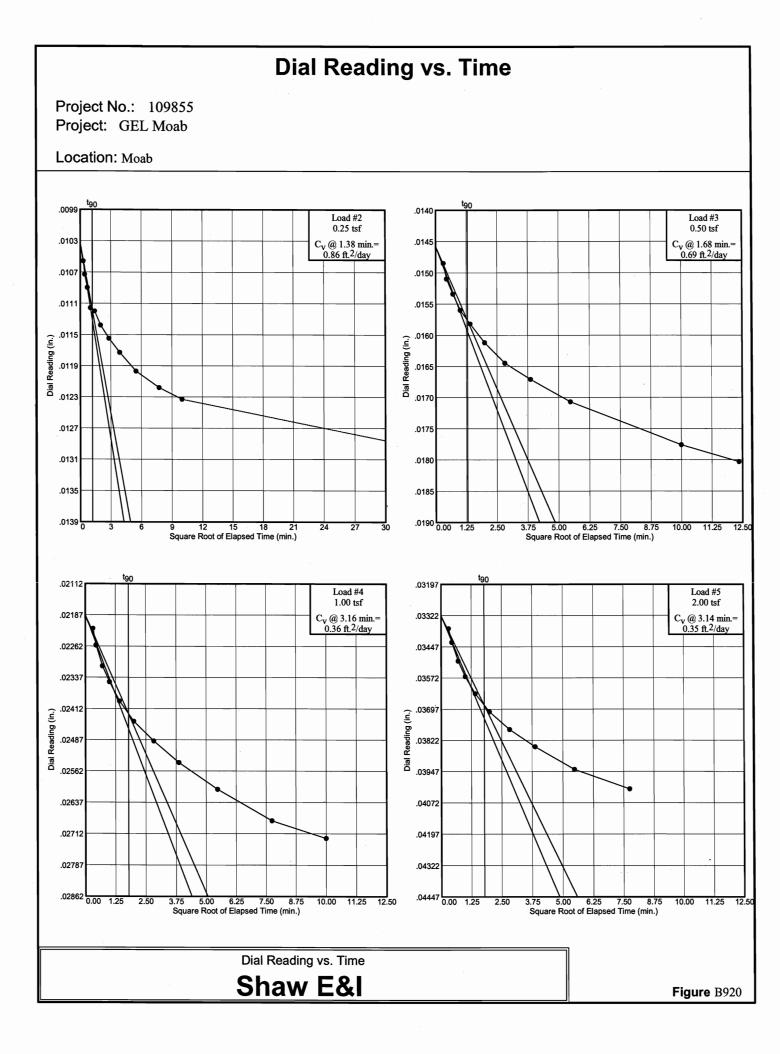
Project No.: 109855 Project: GEL Moab

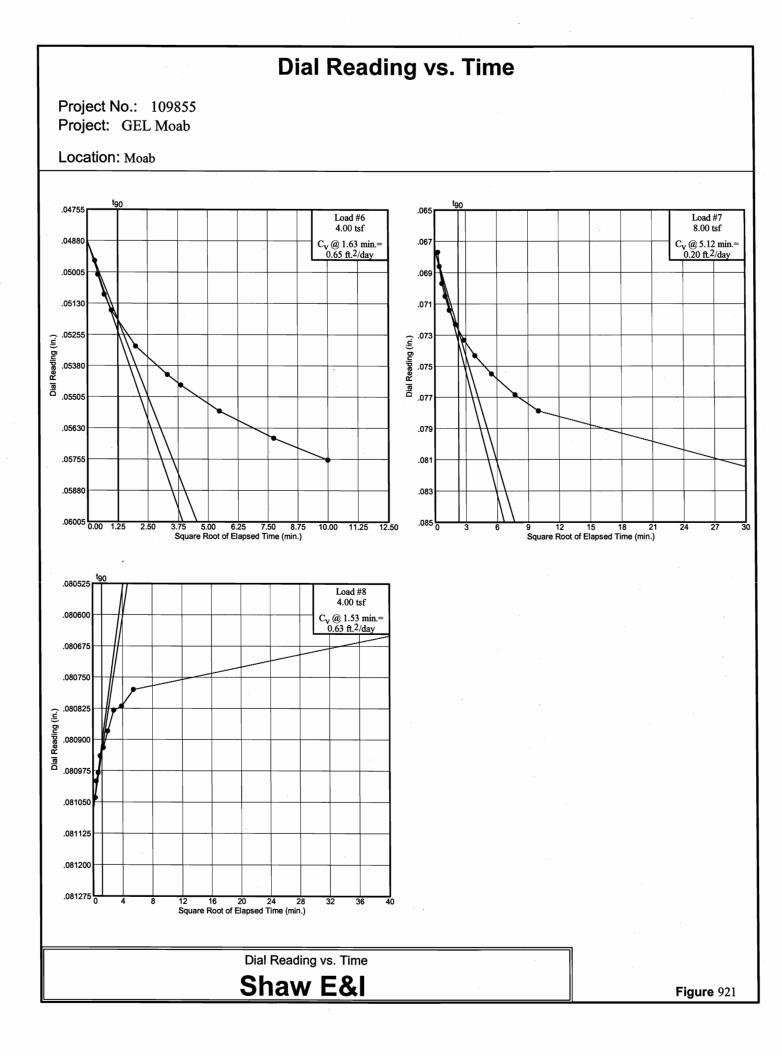
Location: Moab



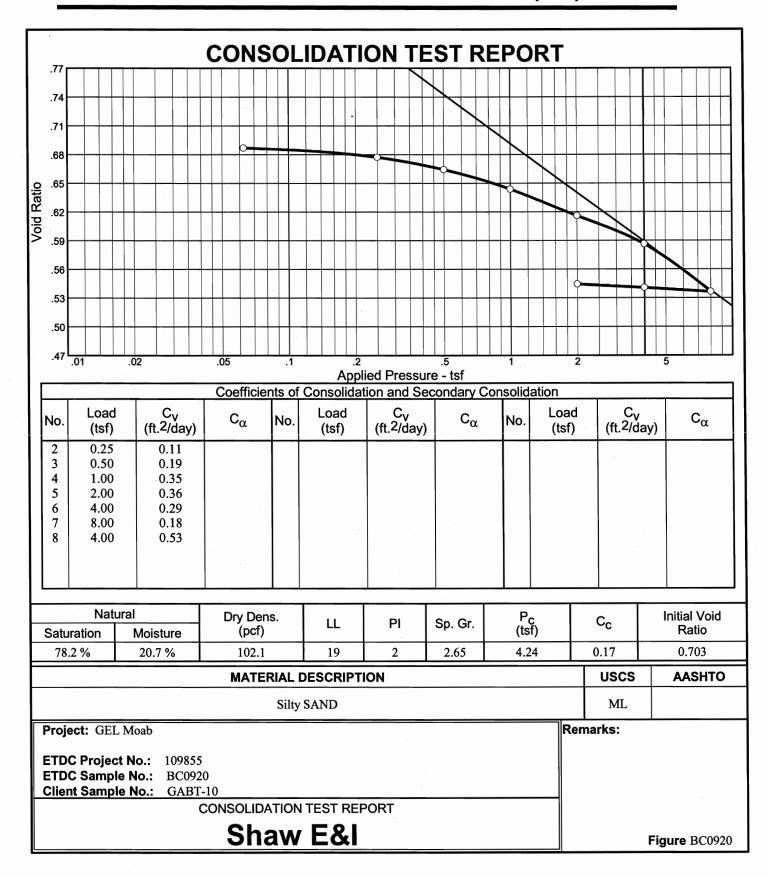
Page 81 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

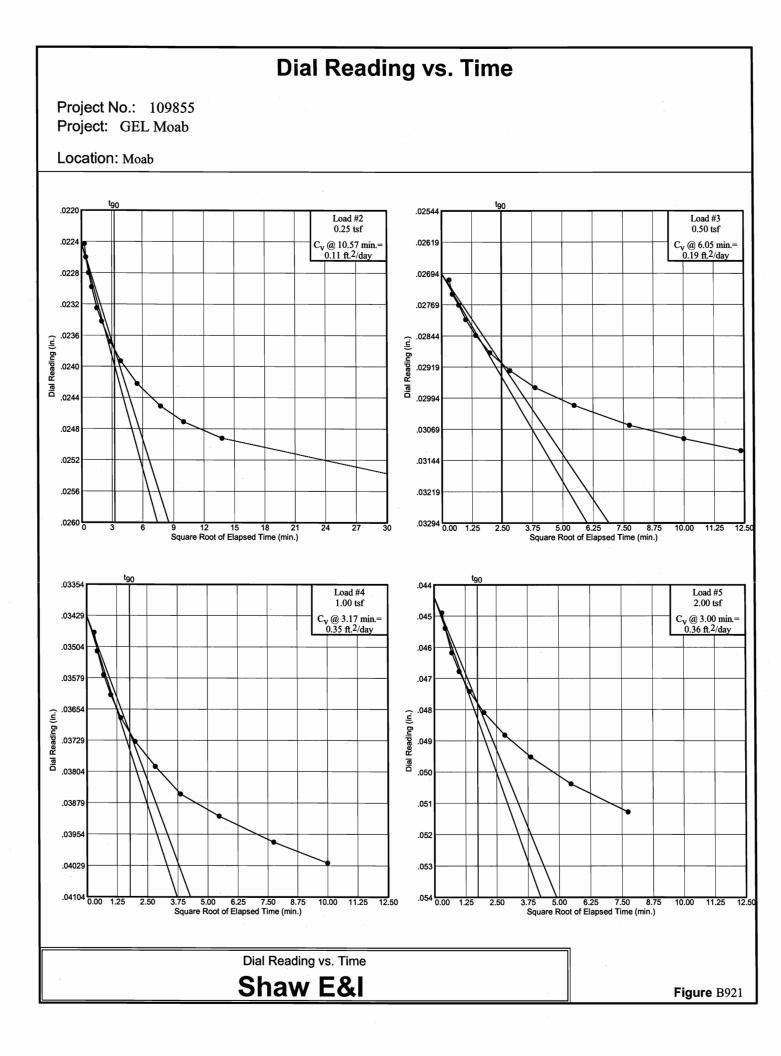


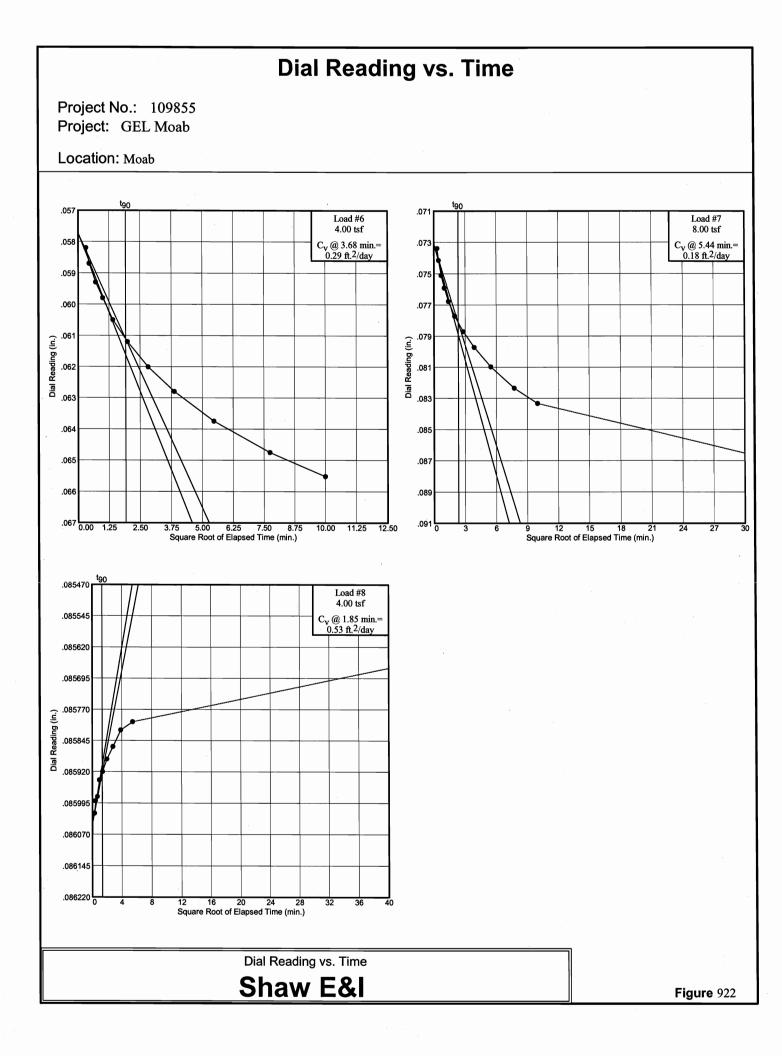




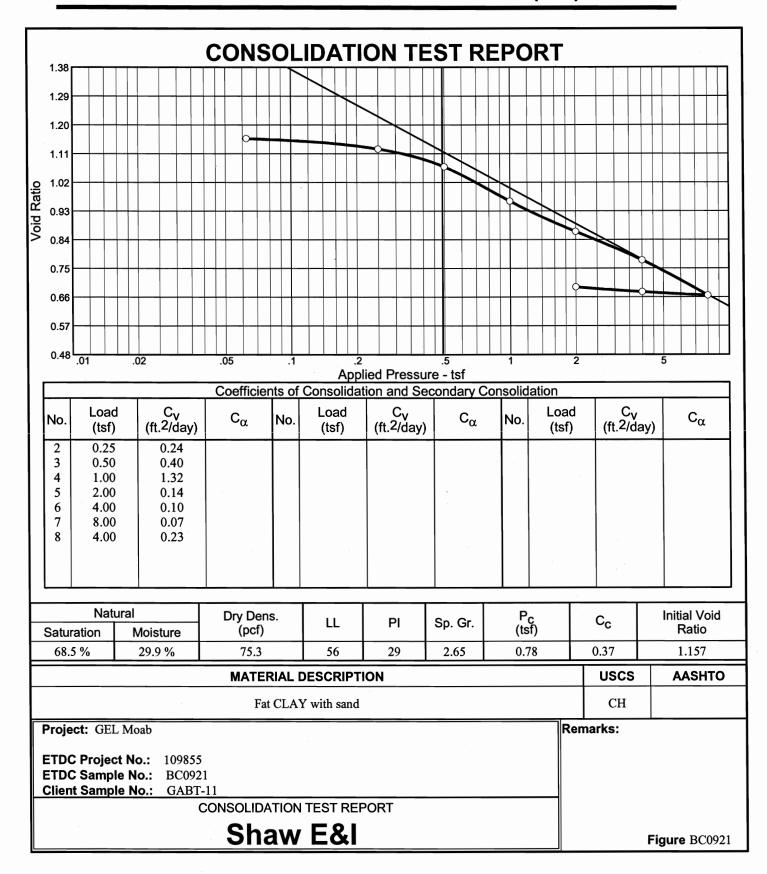
Page 84 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000







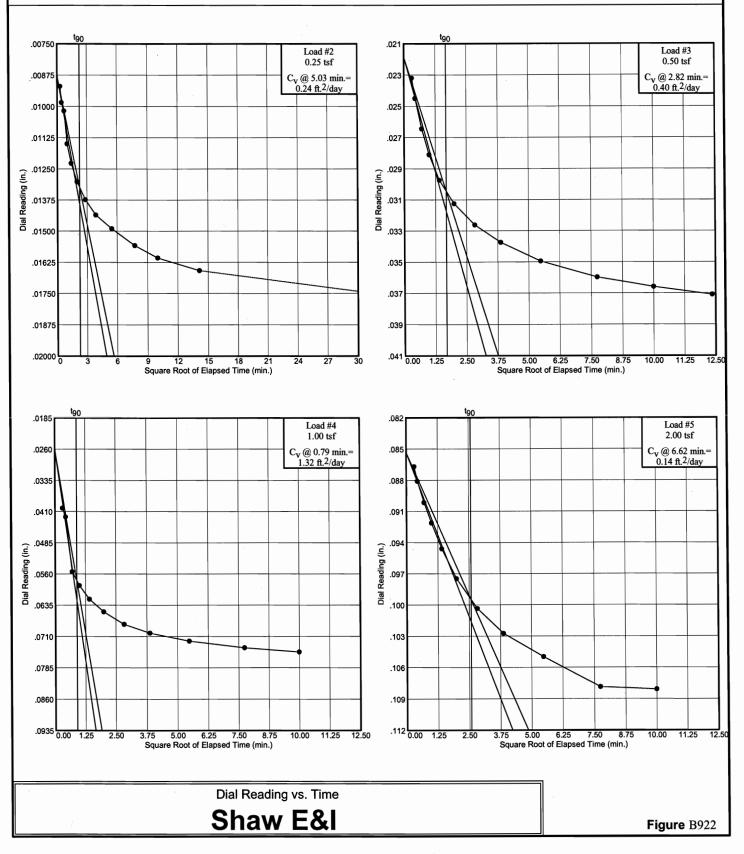
Page 87 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

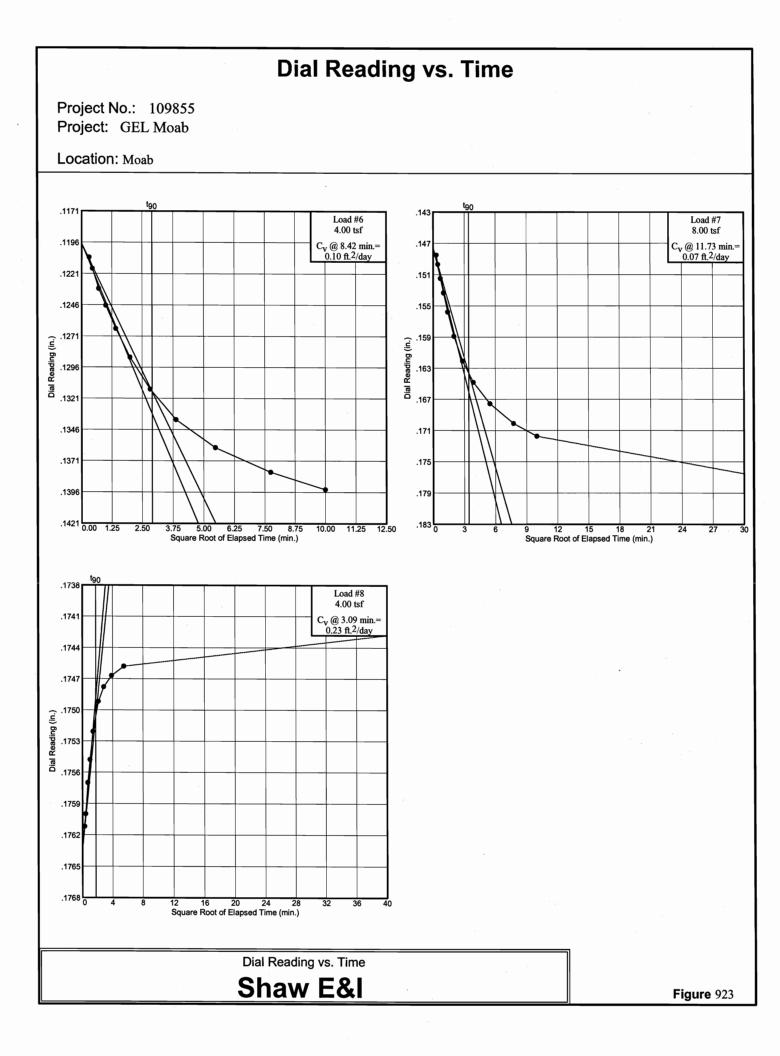


## Dial Reading vs. Time

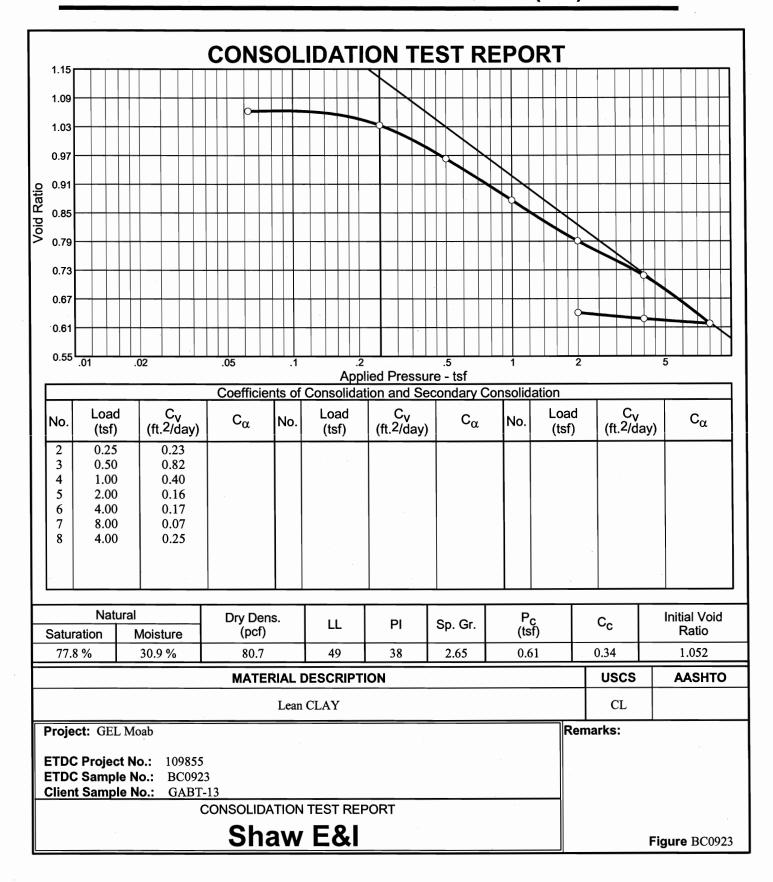
Project No.: 109855 Project: GEL Moab

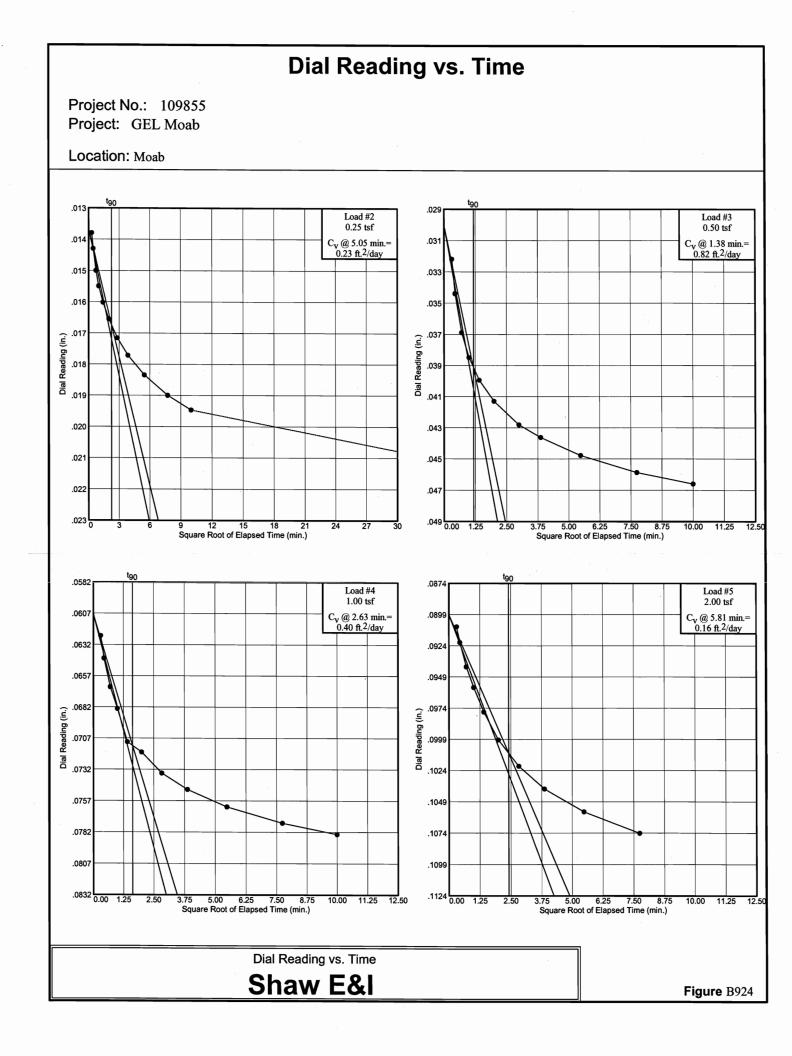
#### Location: Moab

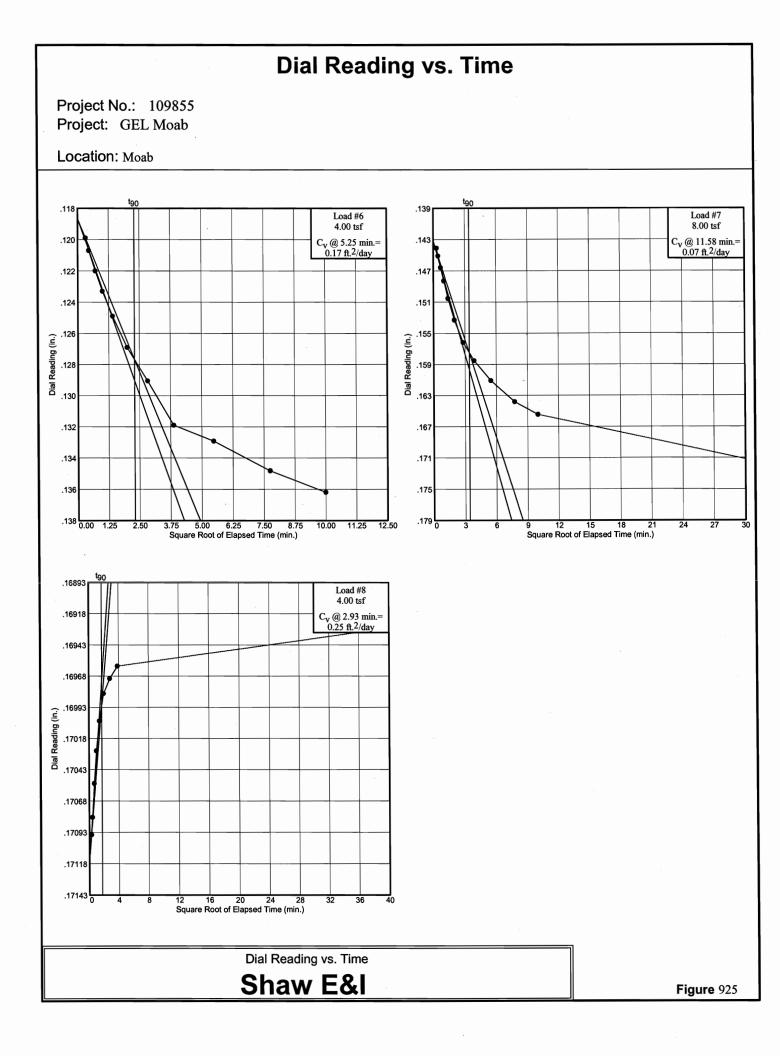


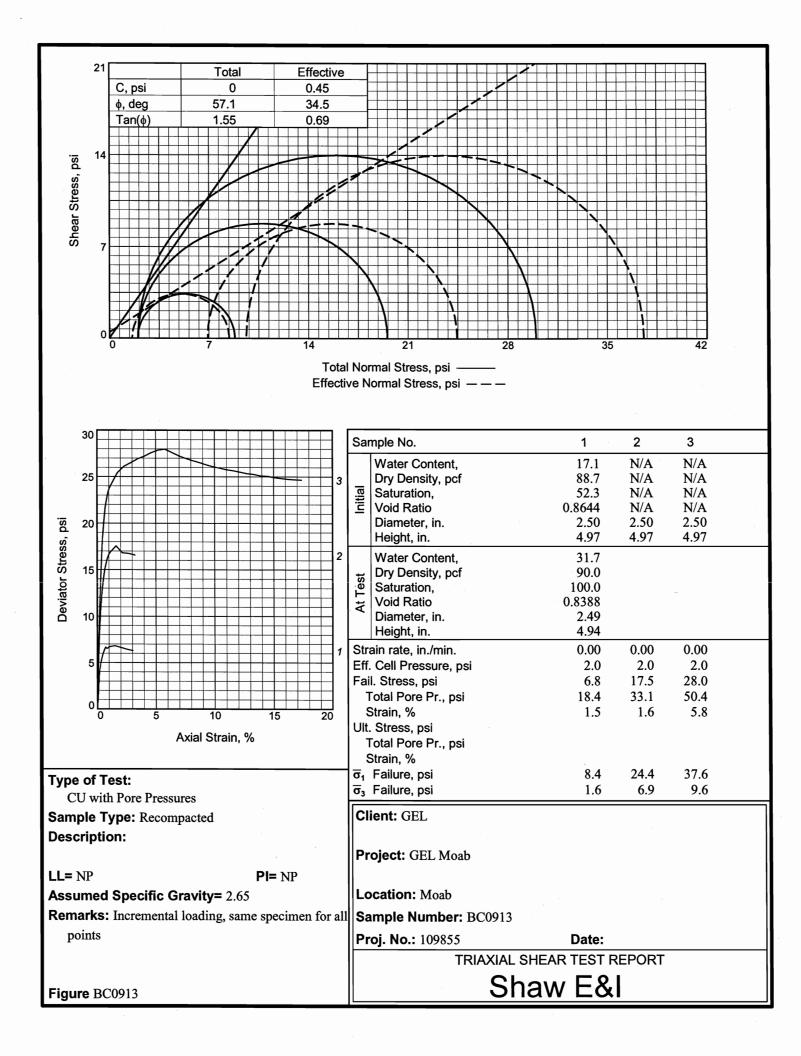


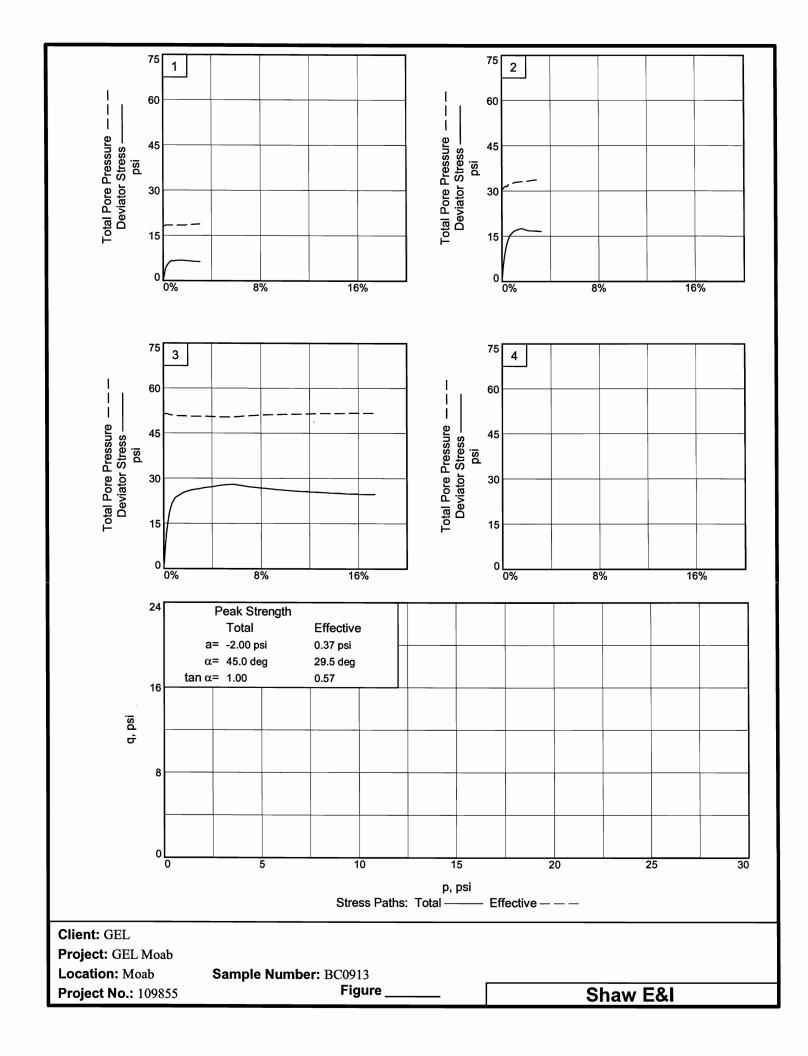
Page 90 of 108 November 7, 2006 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

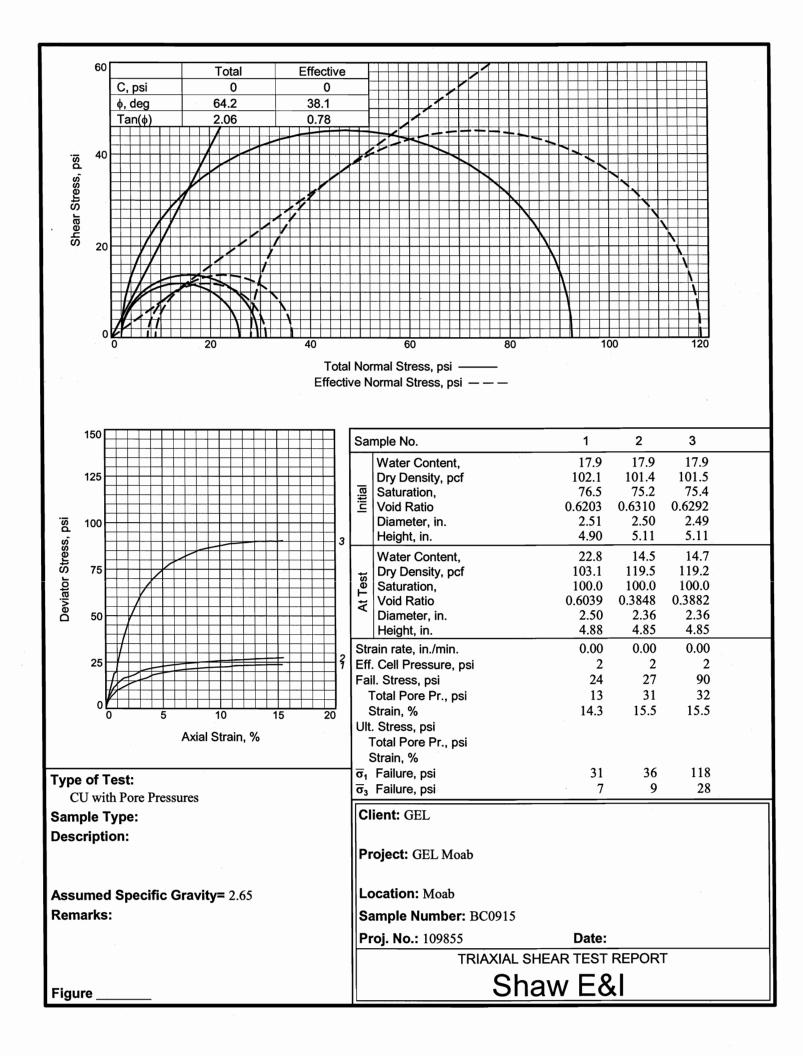


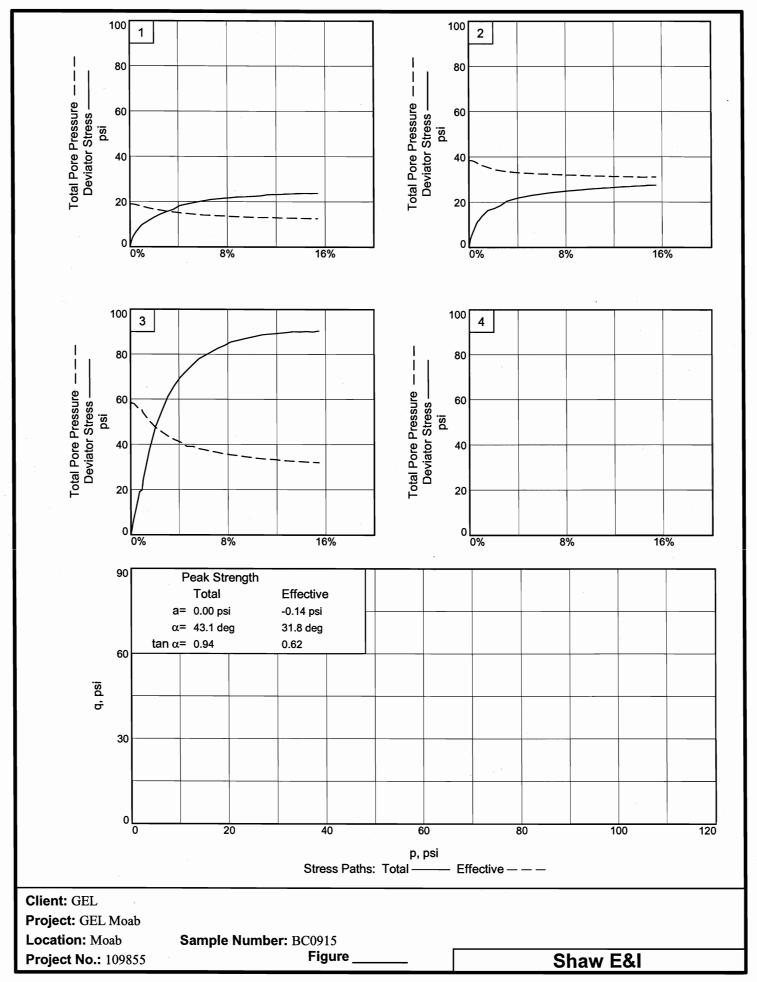




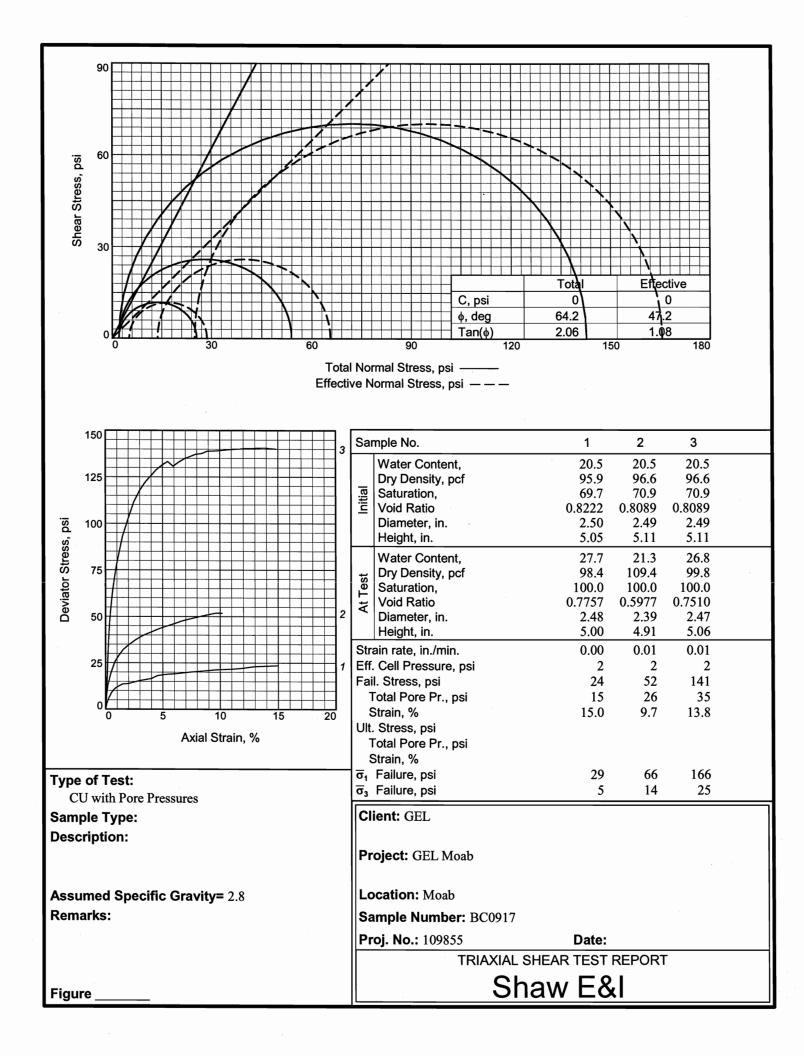


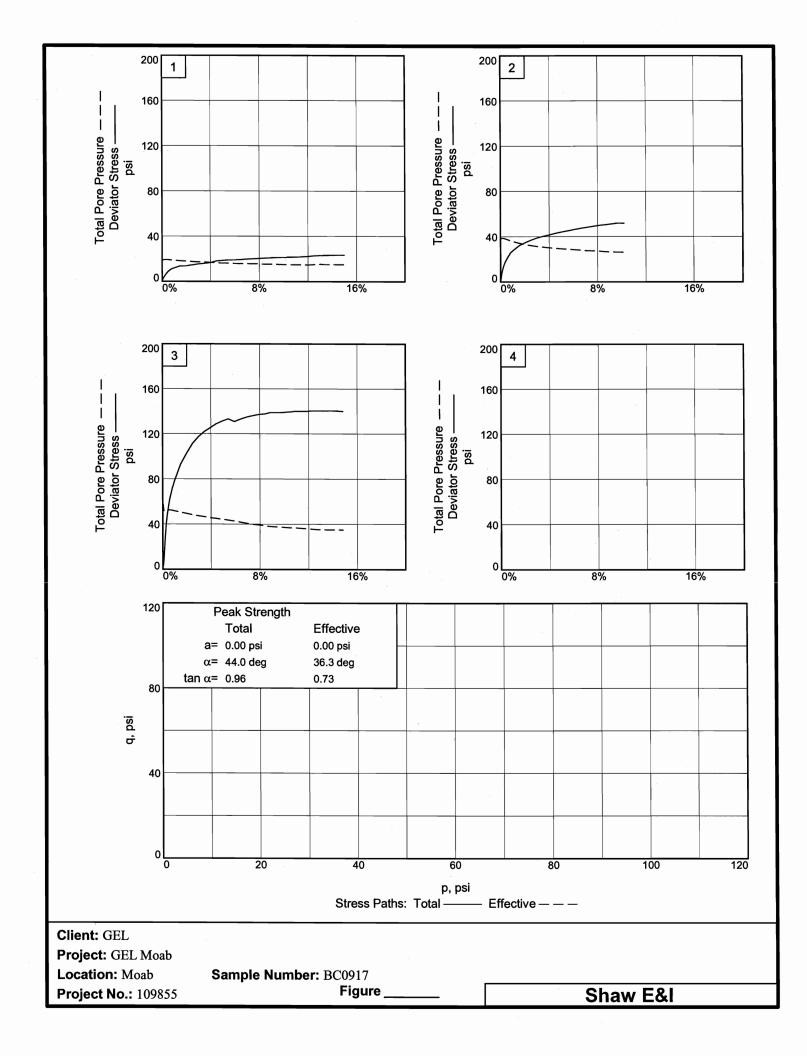


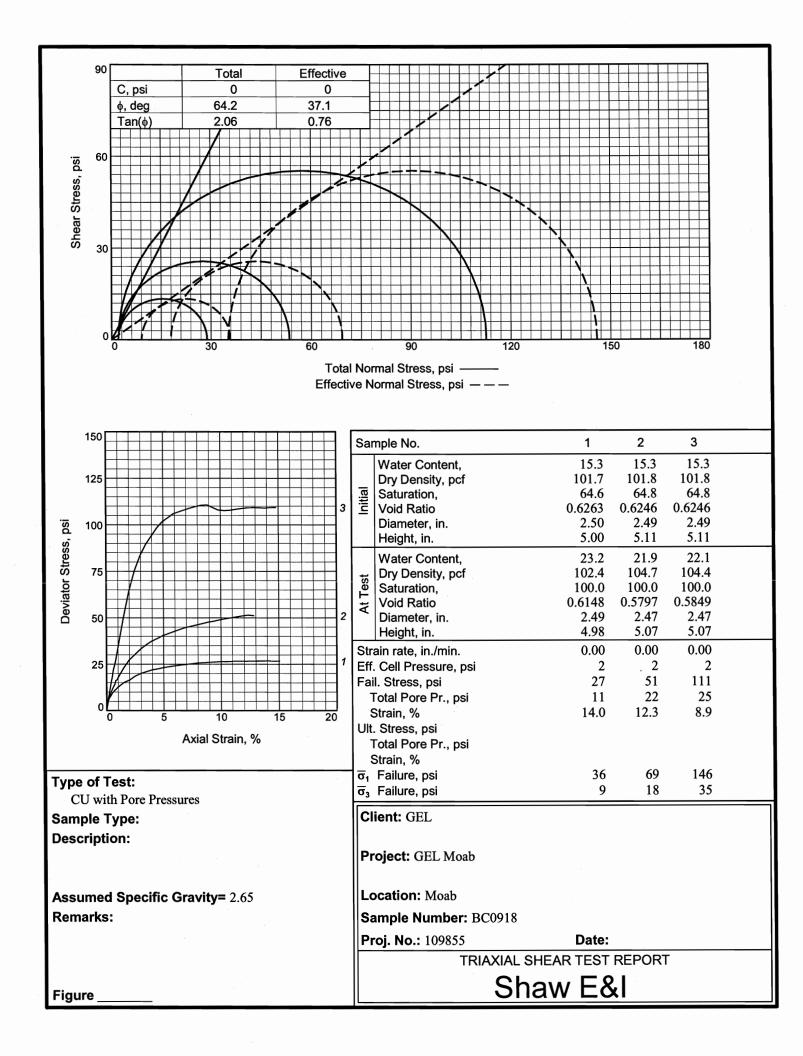


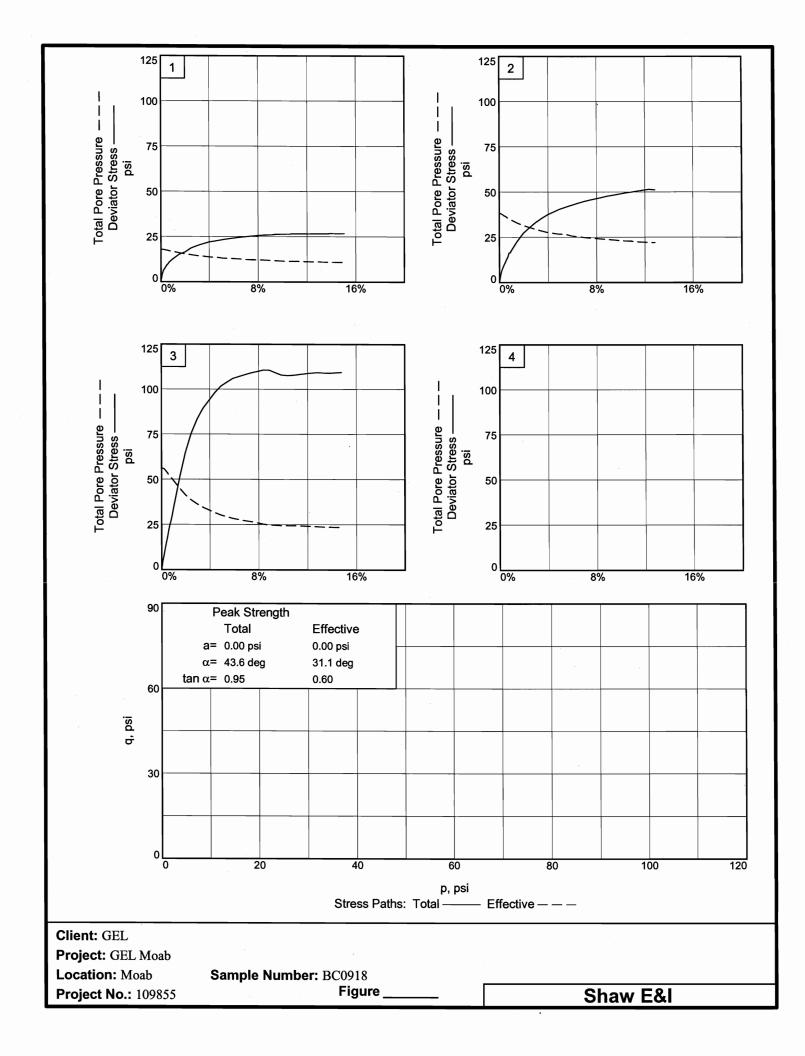


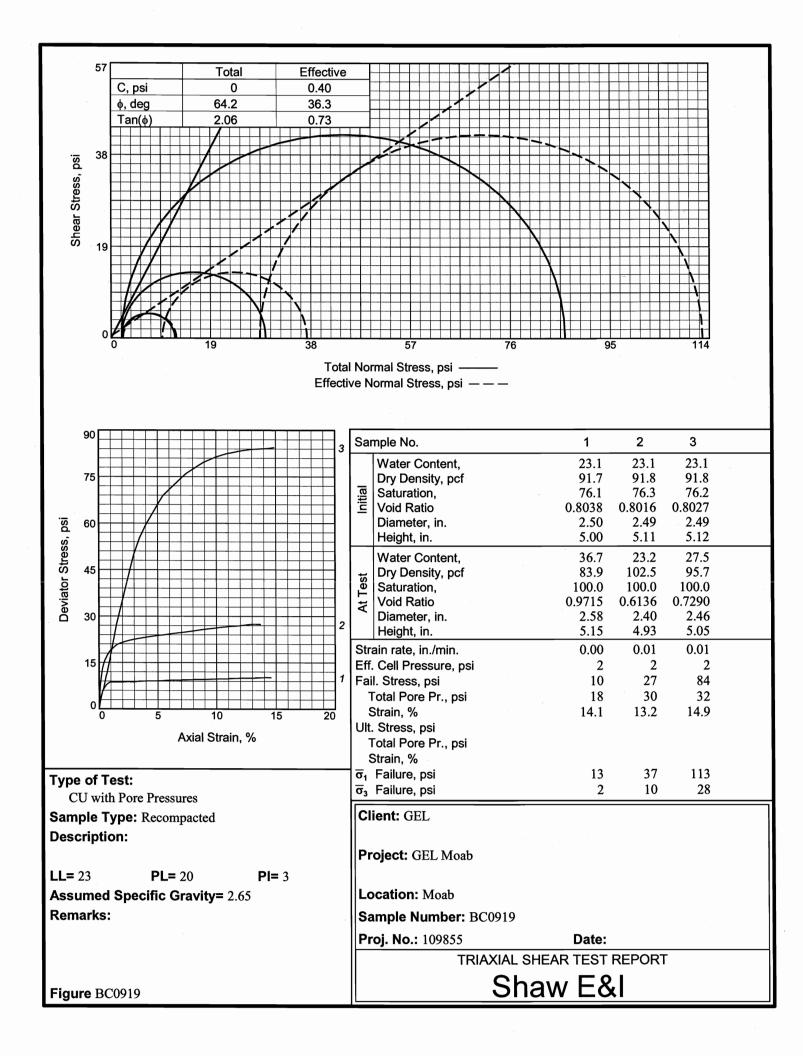
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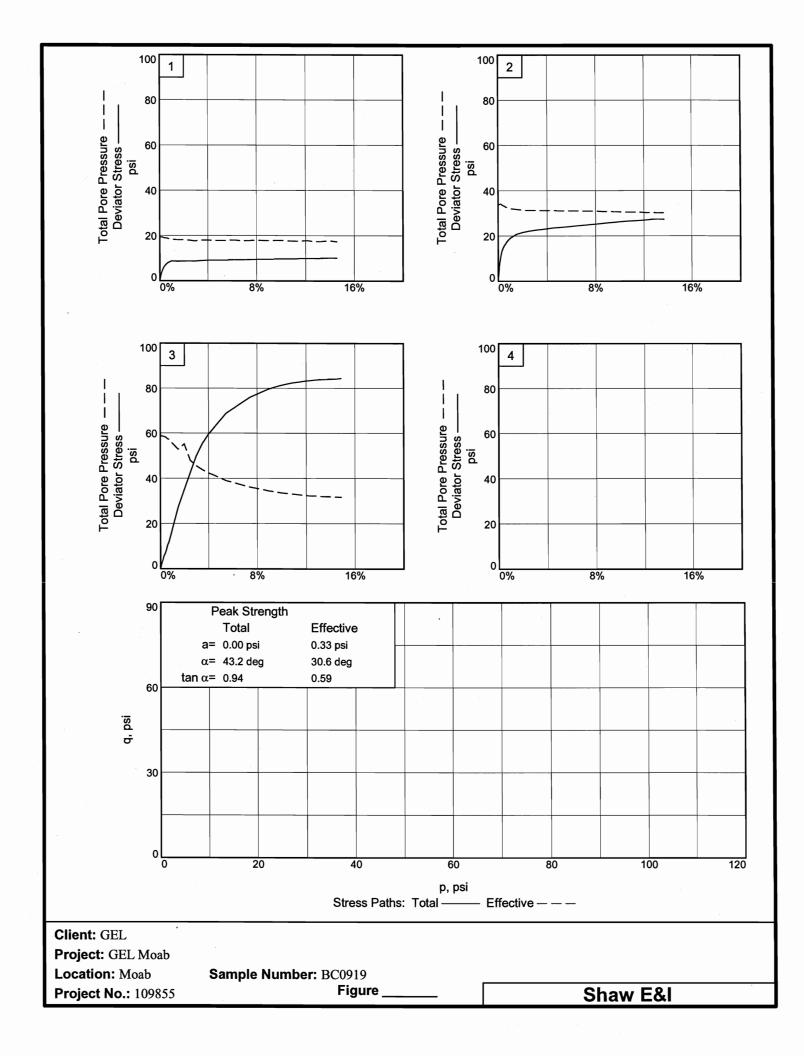


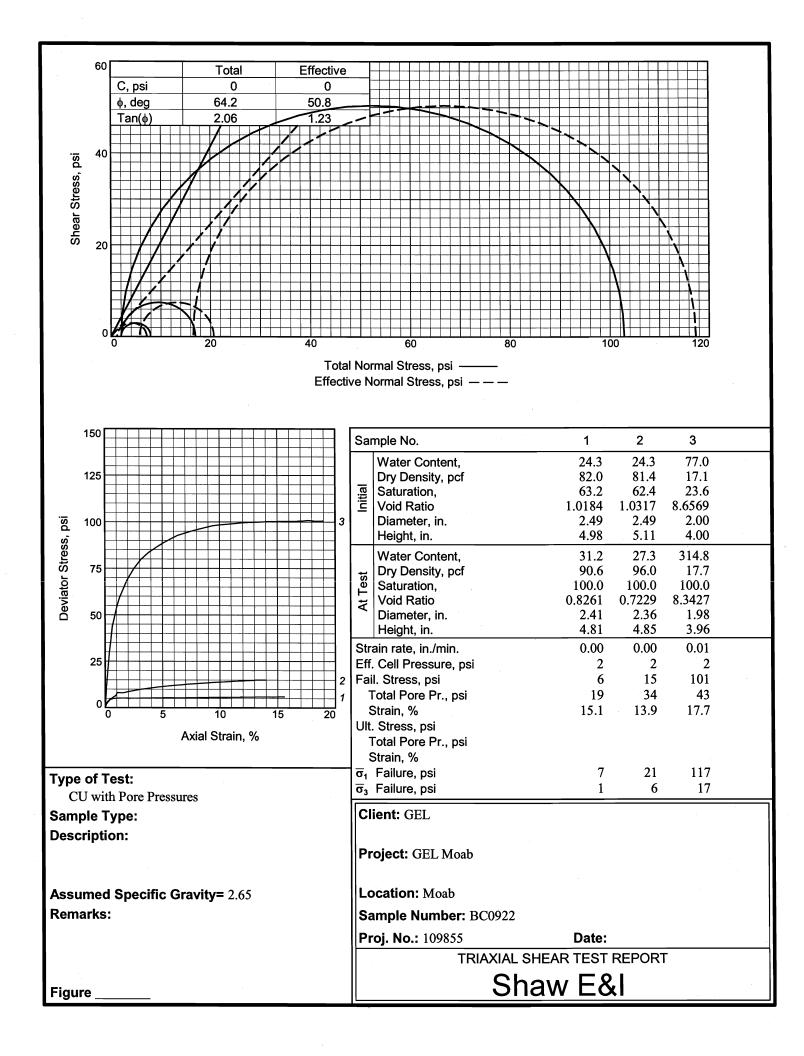


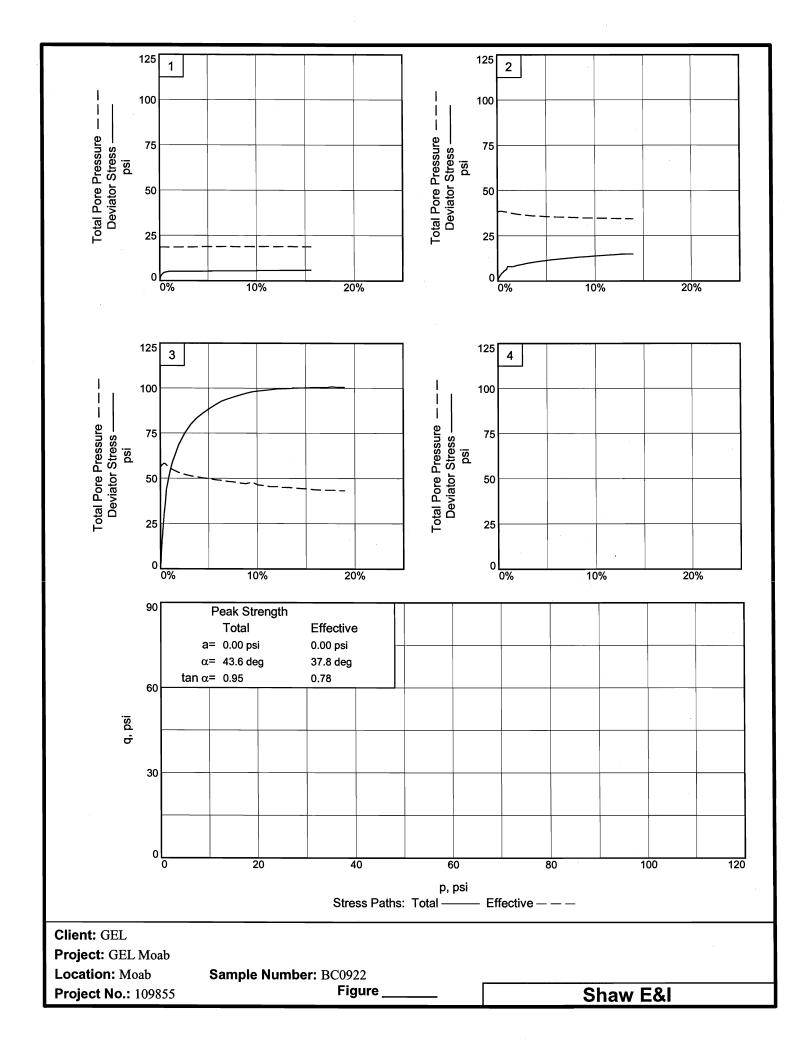


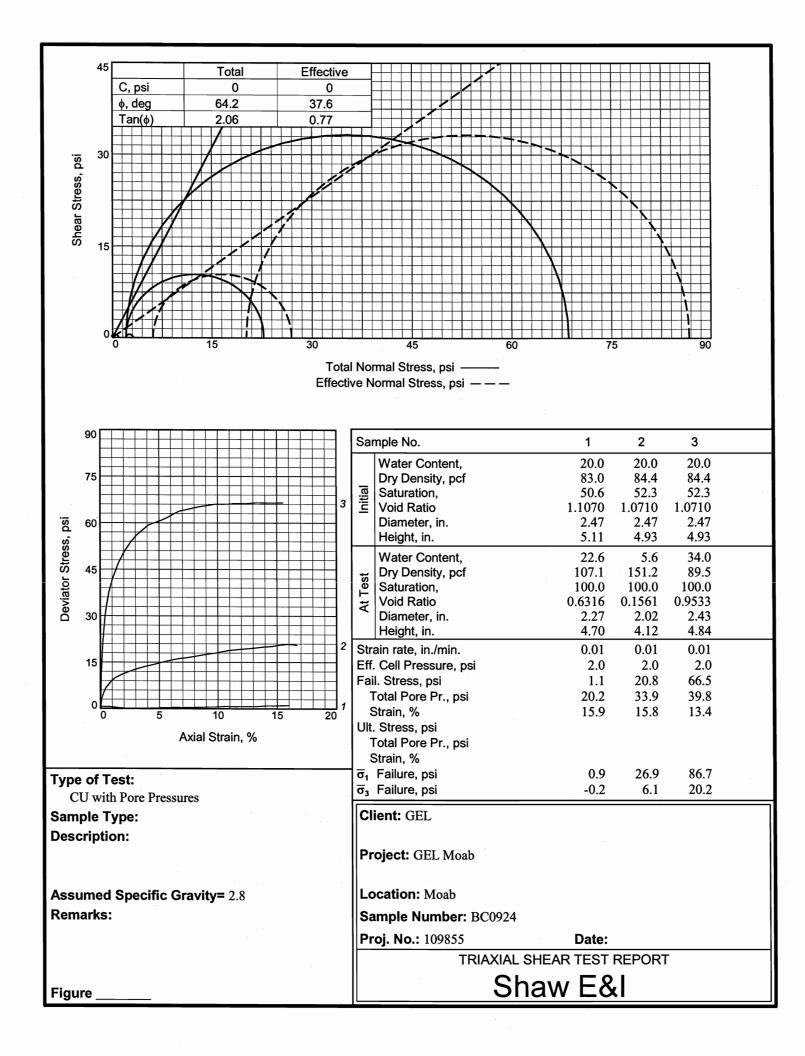


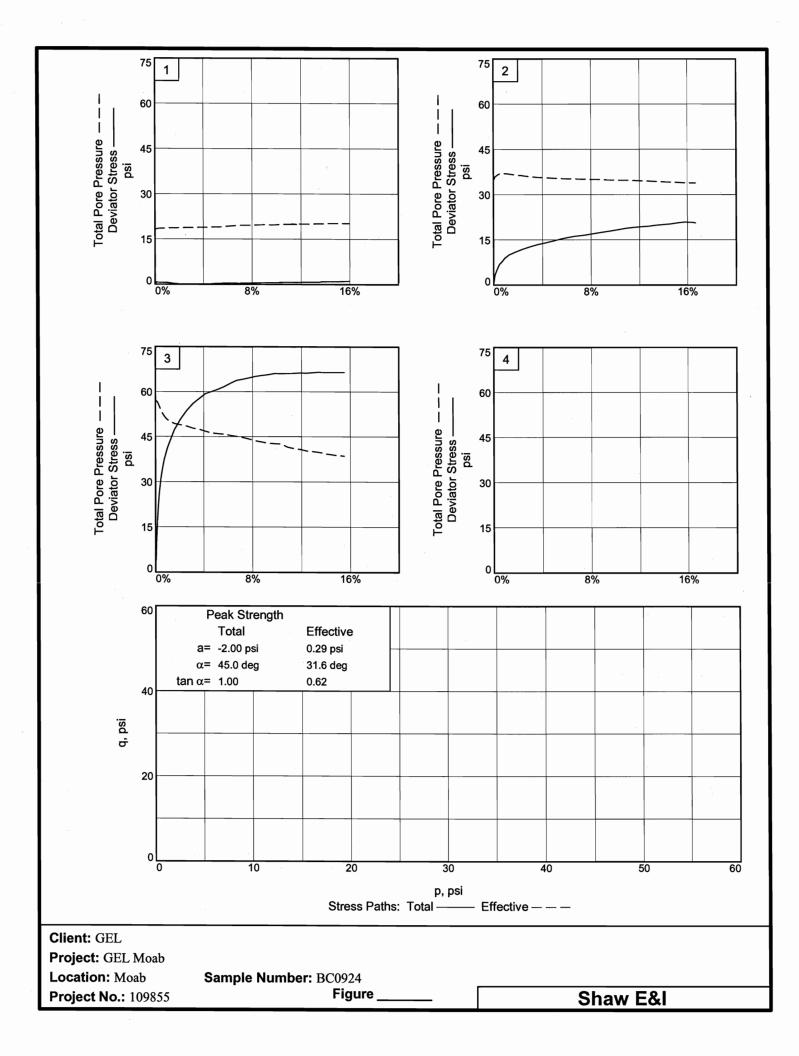












Appendix C Chain of Custody Records <u>Stoller</u> Legacy Management Team

Stoller • Battelle • Source One

## Chain of Custody / Sample Submittal Form

RIN: 06040352 COC: 06040352.1.1 Sampler(s):

Project: MOAB CHARACTERIZATION Cost Number: 06-203-4-01-5-2-01 Purchase Order: 24316 Laboratory: General Engineering Laboratories Address: 2040 Savage Road Charleston, SC 29407 Phone: 843.556.8171

Ticket	Sample Date	Time	Site	Location	Container	Preservation	Filtered	# Containsers	Analysis
	12/08/2005		MOA01	GABT-01	BUCKET	None		1	ODC,HC,TSS,CMR,Part Size,At Lim
	12/08/2005		MOA01	GABT-02	BUCKET	None		1	ODC,HC,TSS,CMR,Part Size,At Lim
	12/08/2005		MOA01	GABT-03	BUCKET	None		1	ODC,HC,TSS,CMR,Part Size,At Lim
	12/08/2005		MOA01	GABT-04	BUCKET	None		1	ODC,HC,TSS,CMR,Part Size,At Lim
	12/08/2005		MOA01	GABT-05	BUCKET	None		1	ODC,HC,TSS,CMR,Part Size,At Lim
	12/08/2005		MOA01	GABT-06	BUCKET	None		1	ODC,HC,TSS,CMR,Part Size,At Lim
	12/08/2005		MOA01	GABT-07	BUCKET	None		1	ODC,HC,TSS,CMR,Part Size,At Lim
	12/08/2005		MOA01	GABT-08	BUCKET	None		1	ODC,HC,TSS,CMR,Part Size,At Lim
	12/08/2005		MOA01	GABT-09	BAG	None		1	ODC,HC,TSS,CMR,ST,LRT,Part Size,At Lim
	12/08/2005		MOA01	GABT-10	BAG	None		1	ODC,HC,TSS,CMR,ST,LRT,Part Size,At Lim
	12/08/2005		MOA01	GABT-11	BAG	None		1	ODC,HC,TSS,CMR,ST,LRT,Part Size,At Lim
	12/08/2005		MOA01	GABT-12	BAG	None		1	ODC,HC,TSS,CMR,ST,LRT,Part Size,At Lim
	12/08/2005		MOA01	GABT-13	BAG	None		1	ODC,HC,TSS,CMR,ST,LRT,Part Size,At Lim
	12/08/2005		MOA01	GABT-14	BAG	None		1	ODC,HC,TSS,CMR,ST,LRT,Part Size,At Lim
			,						

ODC: Consolidation (ASTM D2435), HC: Hydraulic Conductivity (ASTM D5084), TSS: CU Triaxial Shear Strengh (ASTM D4767), CMR: Capillary Moisture Relationship (ASTM D2325) Part Size: Grain Size Distrubution (ASTM D422), At Lim: Atterburg Limits (ASTM D4318), ST: Shake Test (SOG TALGEO0002), LRT: Liquid Release Test (9095A)

R <u>eligu</u> ished by (signature)	Date	Time	Reliquished by (signature)		Date	Time
Et Can	5-2-06	0915				·
Received by (signature)	Date	Time	Received by (signature)	4	Date	Time
Oon Huley SHAPE+ I	5-4-06	0830				
						•

Page 1 of 1

Additional T	esting Reque	sts								
Bench Test Sample No.		Atterberg limits (LL/PL,PI) ASTM D 4318	Grain Size Distribution ASTM D422	Sample Prep	Conductivity using Flexible Wall Permeameter (ASTM	CU Triaxial Shear Strength with Pore Pressure Measurement (ASTM D 4767) confining pressure = 20,40,60 psi	Consolidation (ASTM D 2435) confining pressure = 15,30,60 psi	Capillary- Moisture Relationship (ASTM D 2325, D 3152, or D 5298)	Maximum Dry Density (pcf) (Standard Proctor)	Optimum Moisture Content (Standard Proctor)
oumpie no.		A0111 D 4010		Compact to 85%				0. 2 0200,	1100101/	1100001/
				max, 2% wet of					1. A.	
GABT-01	Cover Soil	x	X	opt	х				117.7	11.9%
GABT-02	Cover Soil	X	Х						109.2	13.8%
				Compact to 85%						
	Sand			of max, 2% wet of						
GABT-03	Tailings	х	х	opt	х	Х			106.3	12.7%
				Compact to 85%						
	Sand			of max, 2% wet of						
GABT-04	Tailings	х	х	opt	X	Х	Х	Х	103.9	15.6%
				Compact to 90%						
	Transition			of max, 2% wet of		~ /				
GABT-05	Tailings	Х	Х	opt	X	х			113.3	13.1%
	_			Compact to 95%						
	Sand			of max, 2% wet of						
GABT-06	Tailings	X	х	opt			X	х	107.3	14.6%
	L			Compact to 90%		/				
	Transition			of max, 2% wet of					107.0	10 101
GABT-07	Tailings	х	х	opt	X	x			107.3	18.4%
	-			Compact to 90%		·				
	Transition	~	v	of max, 2% wet of		v			110.0	10.00/
GABT-08	Tailings	х	X	opt	X	x			112.8	16.0%
	<b>T</b>			Compact to 90%						
	Transition	~	v	of max, 2% wet of	x	x	x	x	102.0	21.1%
GABT-09	Tailings	х	х	opt Compact to 95%	X	×	^	^	102.0	21.1%
	Transition			of max, 2% wet of						
GABT-10		х	v	opt			x	х	107.8	18.7%
GABT-10	rainings	^	^	Compact to 80%			<u>^</u>	<u></u>	107.0	10.770
	Slimes			of max, 2% wet of						
GABT-11		х	x	opt			x	x	96.0	27.8%
0,01-11	, annigo	~	~	Compact to 80%					23.0	
	Slimes			of max, 2% wet of						
		х	х	opt	x	х			101.6	22.5%
				Compact to 85%						
	Slimes			of max, 2% wet of						
GABT-13		х	х	opt			х	x	95.0	28.7%
				Compact to 80%						
	Slimes			of max, 2% wet of						
GABT-14	Tailings	х	х	opt	x	х		1	101.5	20.9%

Appendix C

Certificate of Analysis

February 15, 2007



Geotechnical Laboratory PO Box 4339 1570 Bear Creek Road Oak Ridge TN 37830 (865) 482-6497

### **CERTIFICATE OF ANALYSIS**

Erin Stanley General Engineering Laboratories 2040 Savage Road Charleston SC 29407 February 15, 2007

This is the Certificate of Analysis for the following samples:

Shaw Project ID: Shaw Project Number: Date Received by Lab: Number of Samples: Sample Type: **GEL – Moab** 109855.01410000 May 4, 2006 Fourteen (14) Soil

#### I. Introduction/Case Narrative

Fourteen soil samples were received by the Shaw Geotechnical Laboratory on May 4, 2006. Samples were submitted for determination of particle-size distribution, Atterberg limits, hydraulic conductivity, consolidated-undrained triaxial shear strength, one-dimensional consolidation properties, and capillary moisture relationships. Previous reports have distributed all sample results except for the CU triaxial test for sample GBABT-04, which is reported here.

Please see Appendix A, Sample Number Cross Reference List; Appendix B, Analysis Results; and Appendix C, Chain-of-Custody/Sample Receipt Records.

Reviewed and Approved:

Kulahle

Ralph Cole Laboratory Manager, Geotechnical Services

### II. <u>Analytical Results/Methodology</u>

REFERENCES: United Nations, *Recommendations on the Transport of Dangerous Goods, Manual of Tests and Criteria*, third ed. New York, 1999. United States Army Corps of Engineers (USACE), Engineer Manual 1110-2-1906, *Laboratory Soils Testing*, appendix II, 1970; United States Environmental Protection Agency, SW846, *Test Methods for Examining Solid Waste*, *Physical/Chemical Methods*, 3rd ed., Nov 1986 (EPA SW-846). Annual Book of ASTM Standards, Section 4, Construction, Volume 04.08, *Soil and Rock (I)*, and Volume 04.09, *Soil and Rock (II)*, 2006. Shaw Environmental and infrastructure, Standard Operating Procedures.

Consolidated-Undrained Triaxial Shear Strength ......ASTM D 4767

### III. Quality Control

Quality control checks such as duplicates and spikes (QC samples), are not normally applicable to geotechnical testing. This is due largely to the inability of obtaining samples with known characteristics, the heterogenous nature of the samples, and quality control procedures built-in to the analytical method.

QC measures to ensure accuracy and precision of test results include the following:

- 100% verification of all numerical results raw data entries, transcriptions and calculations entered by lab technicians are checked, recalculated and verified. Most data calculations are performed by computer programs.
- Data validation through test reasonableness summaries of all test results for individual reports are reviewed to determine the overall reasonableness of data and to determine the presence of any data that may be considered outliers.
- Quality control procedures are built into most standardized geotechnical procedures. For example, liquid limit and plastic limit analyses call for re-analyses and specify acceptance criteria.
- Routine instrument calibration instruments, gauges and equipment used in testing are calibrated on a routine basis. All instrument calibration follows ASTM or manufacturer guidelines.
- Maintenance of all past calibration records calibration records and certification documents of all instruments, gauges and equipment are updated routinely and maintained in the Quality Control Coordinators Quality/Operations files.
- Certified and trained personnel all technicians are trained in the application of standard laboratory procedures for geotechnical analyses as well as the quality assurance measures implemented by Shaw.

Page 3 of 8 February 15, 2007 Erin Stanley General Engineering Laboratories Shaw Project Name: GEL – Moab Shaw Project No. 109855.01410000

 Quantitative analyses frequently used in geotechnical/physical testing programs do not use QC tools common to wet chemistry or radiochemistry laboratories. Measures not employed in the analysis of samples reported in this report include: laboratory control samples (LCS), blanks, matrix spikes (MS), duplicate analyses, dilutions, digestions, correction factors, surrogate sample analyses, detection limit determinations, control charts, and/or tentatively identified compounds (TICs).

#### IV. Data Qualification

A specific gravity value of 2.65 was assumed and used for certain calculations of ancillary supporting data, such as phase relations (degree of saturation, void ratio) reported in some test results. This value is an average of all natural earth materials and could vary markedly from the sample being tested.

Consolidated-undrained triaxial shear tests were performed by recompacting bulk material into test specimens. Density and moisture parameters supplied by the client. Three test specimens were prepared for each sample. The specimens were tested at 20 psi, 40 psi and 60 psi confining pressures.

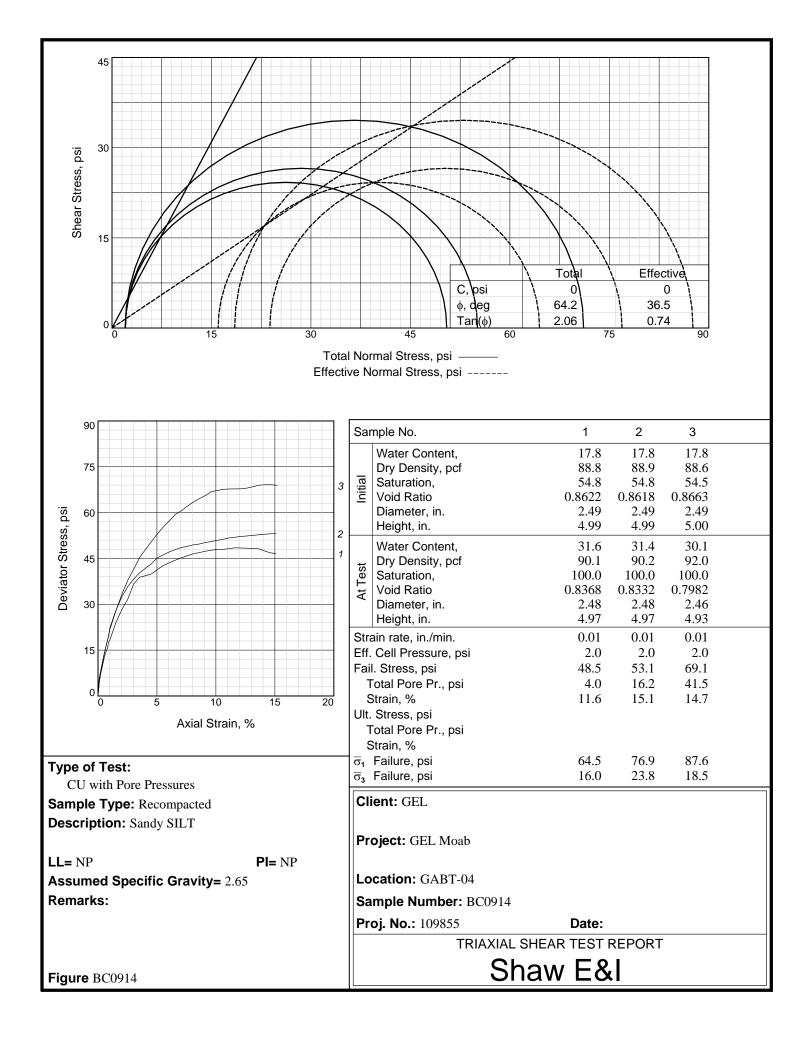
Cohesion intercepts for this sample were set to "zero" from negative intercepts calculated by the commercial software used to reduce data.

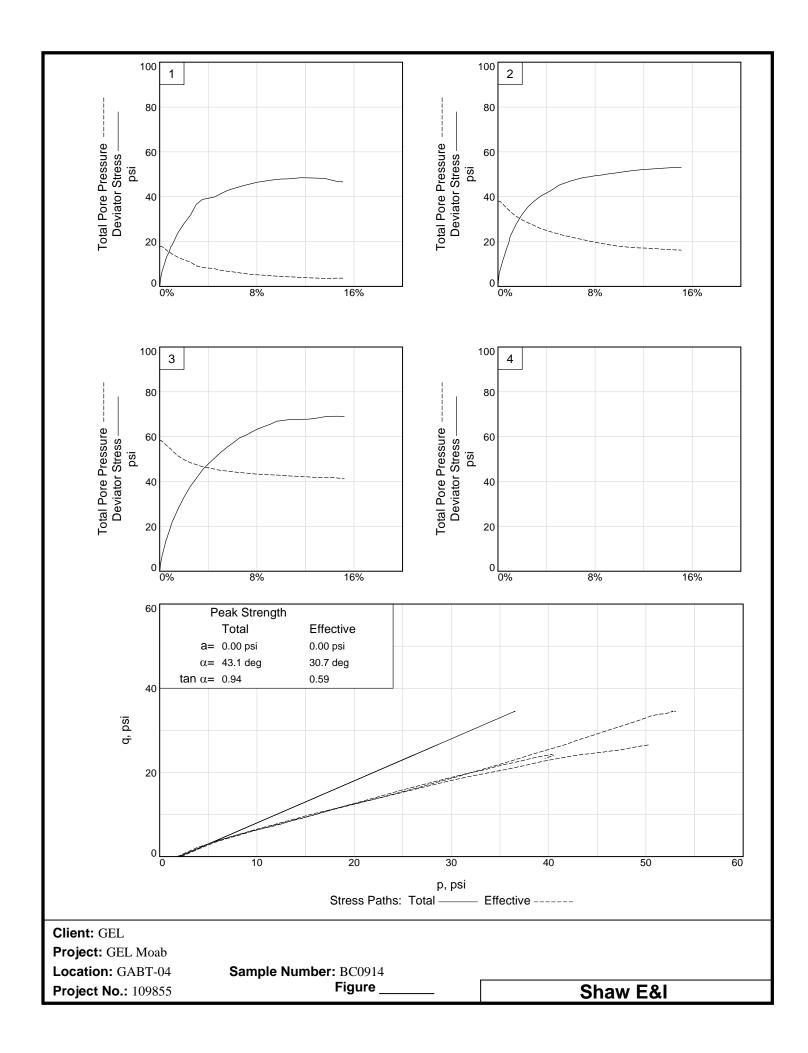
Appendix A Sample Cross-Reference List

### SAMPLE NUMBER CROSS-REFERENCE LIST (Cont'd)

LAB SAMPLE NO.	CLIENT SAMPLE NO.	MATRIX
BC0911	GABT-01	Soil
BC0912	GABT-02	Soil
BC0913	GABT-03	Soil
BC0914	GABT-04	Soil
BC0915	GABT-05	Soil
BC0916	GABT-06	Soil
BC0917	GABT-07	Soil
BC0918	GABT-08	Soil
BC0919	GABT-09	Soil
BC0920	GABT-10	Soil
BC0921	GABT-11	Soil
BC0922	GABT-12	Soil
BC0923	GABT-13	Soil
BC0924	GABT-14	Soil

Appendix B Data Results





Appendix C Chain of Custody Records <u>Stoller</u> Legacy Management Team

Stoller • Battelle • Source One

## Chain of Custody / Sample Submittal Form

RIN: 06040352 COC: 06040352.1.1 Sampler(s):

Project: MOAB CHARACTERIZATION Cost Number: 06-203-4-01-5-2-01 Purchase Order: 24316 Page 1 of 1 Laboratory: General Engineering Laboratories Address: 2040 Savage Road Charleston, SC 29407 Phone: 843.556.8171

Ticket	Sample Date	Time	Site	Location	Container	Preservation	Filtered	# Containsers	Analysis
	12/08/2005		MOA01	GABT-01	BUCKET	None		1	ODC,HC,TSS,CMR,Part Size,At Lim
	12/08/2005		MOA01	GABT-02	BUCKET	None		1	ODC,HC,TSS,CMR,Part Size,At Lim
	12/08/2005		MOA01	GABT-03	BUCKET	None		1	ODC,HC,TSS,CMR,Part Size,At Lim
	12/08/2005		MOA01	GABT-04	BUCKET	None		1	ODC,HC,TSS,CMR,Part Size,At Lim
	12/08/2005		MOA01	GABT-05	BUCKET	None		1	ODC,HC,TSS,CMR,Part Size,At Lim
	12/08/2005		MOA01	GABT-06	BUCKET	None		1	ODC,HC,TSS,CMR,Part Size,At Lim
	12/08/2005		MOA01	GABT-07	BUCKET	None		1	ODC,HC,TSS,CMR,Part Size,At Lim
	12/08/2005		MOA01	GABT-08	BUCKET	None		1	ODC,HC,TSS,CMR,Part Size,At Lim
	12/08/2005		MOA01	GABT-09	BAG	None		1	ODC,HC,TSS,CMR,ST,LRT,Part Size,At Lim
	12/08/2005		MOA01	GABT-10	BAG	None		1	ODC,HC,TSS,CMR,ST,LRT,Part Size,At Lim
	12/08/2005		MOA01	GABT-11	BAG	None		1	ODC,HC,TSS,CMR,ST,LRT,Part Size,At Lim
	12/08/2005		MOA01	GABT-12	BAG	None		1	ODC,HC,TSS,CMR,ST,LRT,Part Size,At Lim
	12/08/2005		MOA01	GABT-13	BAG	None		1	ODC,HC,TSS,CMR,ST,LRT,Part Size,At Lim
	12/08/2005		MOA01	GABT-14	BAG	None		1	ODC,HC,TSS,CMR,ST,LRT,Part Size,At Lim
			,						

ODC: Consolidation (ASTM D2435), HC: Hydraulic Conductivity (ASTM D5084), TSS: CU Triaxial Shear Strengh (ASTM D4767), CMR: Capillary Moisture Relationship (ASTM D2325) Part Size: Grain Size Distrubution (ASTM D422), At Lim: Atterburg Limits (ASTM D4318), ST: Shake Test (SOG TALGEO0002), LRT: Liquid Release Test (9095A)

Religuished by (signature)	Date	Time	Reliquished by (signature)	Date	Time
Erean	5-2-06	0915			·
Received by (signature)	Date	Time	Received by (signature)	Date	Time
Oon Kulling SHAPE+ I	5-4-06	0830			
				·	

Additional T	esting Reque	sts								
Bench Test Sample No.		Atterberg limits (LL/PL,PI) ASTM D 4318	Grain Size Distribution ASTM D422	Sample Prep	Hydraulic Conductivity using Flexible Wall Permeameter (ASTM D 5084) confining pressure = 20,40,60 psi	CU Triaxial Shear Strength with Pore Pressure Measurement (ASTM D 4767) confining pressure = 20,40,60 psi	Consolidation (ASTM D 2435) confining pressure = 15,30,60 psi	Capillary- Moisture Relationship (ASTM D 2325, D 3152, or D 5298)	Maximum Dry Density (pcf) (Standard Proctor)	Optimum Moisture Content (Standard Proctor)
Sample No.	. Son Type	A31MD 4310	7.0111 0422	Compact to 85%	- 20,40,00 poi	20,40,00 poi	10,00,00 por	01 D 5298/	FIGCIOI	FIGUIDI
				max, 2% wet of						
GABT-01	Cover Soil	x	x	opt	x				117.7	11.9%
GABT-02	Cover Soil	x	х						109.2	13.8%
				Compact to 85%						
	Sand			of max, 2% wet of						
GABT-03	Tailings	x	х	opt	x	х			106.3	12.7%
				Compact to 85%						
	Sand			of max, 2% wet of						
GABT-04	Tailings	x	x	opt	x	Х	х	Х	103.9	15.6%
				Compact to 90%						
	Transition			of max, 2% wet of		1				
GABT-05	Tailings	x	х	opt	X	x /			113.3	13.1%
				Compact to 95%						
	Sand			of max, 2% wet of						
GABT-06	Tailings	Х	Х	opt			Х	Х	107.3	14.6%
				Compact to 90%		,				
	Transition			of max, 2% wet of		/				
GABT-07	Tailings	Х	Х	opt	Х	X			107.3	18.4%
				Compact to 90%		1				
	Transition			of max, 2% wet of						
GABT-08	Tailings	Х	Х	opt	X	x			112.8	16.0%
				Compact to 90%						
	Transition			of max, 2% wet of						
GABT-09	Tailings	Х	Х	opt	X	х	Х	Х	102.0	21.1%
				Compact to 95%						
	Transition			of max, 2% wet of						
GABT-10	Tailings	х	Х	opt			Х	х	107.8	18.7%
				Compact to 80%						
0.0T ()	Slimes	v	V.	of max, 2% wet of			v	v	06.0	07.00/
GABT-11	Tailings	х	х	opt			х	x	96.0	27.8%
	0.			Compact to 80%						
	Slimes	v	v	of max, 2% wet of	v	v			101.0	22.5%
GABT-12	Tailings	x	х	opt	X	X			101.6	22.5%
	Olimon			Compact to 85%						
	Slimes	v	V	of max, 2% wet of			x	x	95.0	28.7%
GABT-13	Tailings	x	х	opt Compact to 80%			^	^	95.0	20.1%
	Slimon			of max, 2% wet of						
CART 44	Slimes Tailings	х	x		x	x		÷	101.5	20.9%
GABT-14	railings	^	^	lobr	^	^			101.0	20.070