

Prefabricated High-Strength Rebar Systems with High-Performance Concrete for Accelerated Construction of Nuclear Concrete Structures



The College of Engineering
at the University of Notre Dame



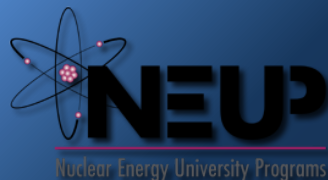
Primary Objective

Reduce field construction times and fabrication costs of reinforced concrete nuclear structures through:

- 1) High-strength reinforcing steel bars (rebar)
- 2) Prefabricated rebar assemblies, including headed anchorages
- 3) High-strength concrete

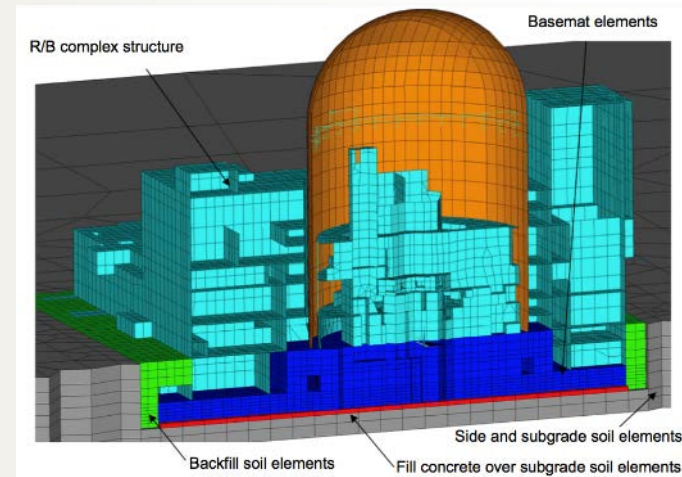


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Scope

- Explore effectiveness, code conformity, and viability of existing high-strength materials
- Focus on shear walls – most common lateral load resisting members in nuclear structures (pressure vessels not in scope)
- Aim to reduce complexities in rebar to improve construction quality and ease of inspection



US-APWR Design Control Doc.

Collaboration



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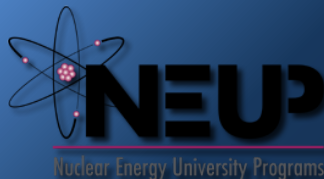
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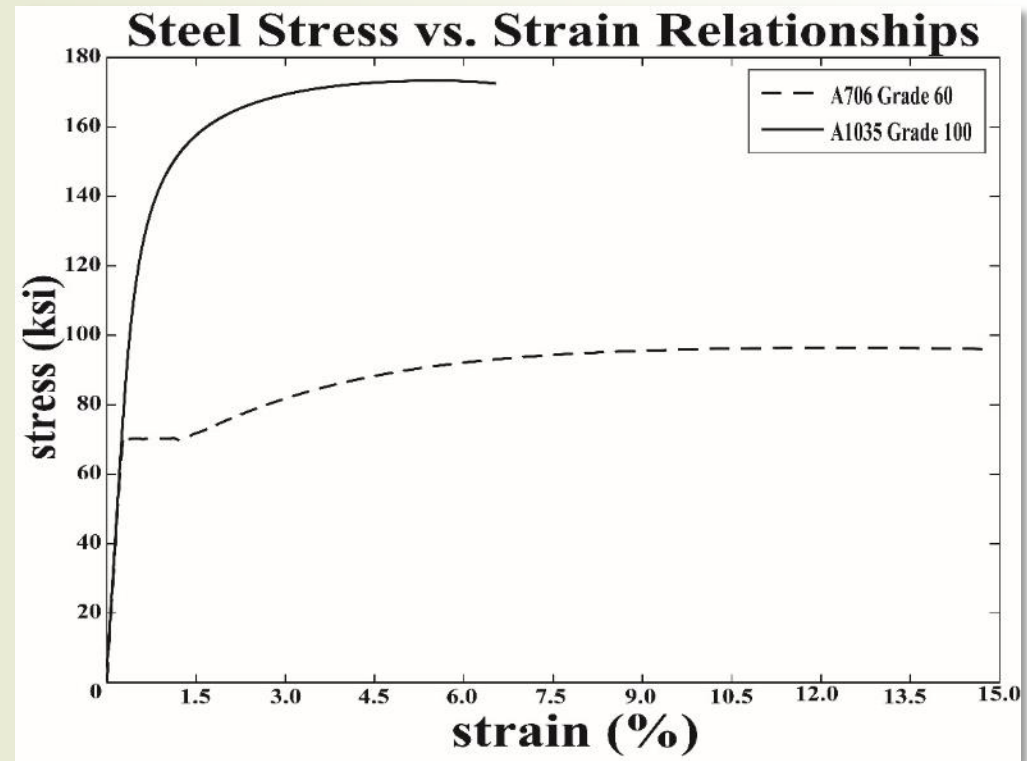


Project Tasks

1. Evaluation of High-Strength Materials
 - Limit-benefit Analysis
 - Cost-benefit Analysis
2. Evaluation of Prefabricated Rebar Cages
3. Optimization, Modeling, and Design
 - Pre-test Analyses
4. Experimental Testing
 - Deep Beam (Wall Slice) Specimens
 - Shear Wall Specimens
5. Design/Modeling/Construction Recommendations

1. High-Strength Materials: Scope

- High-strength rebar (up to Grade 120) with high-strength concrete (up to 15 ksi compressive strength)
- Concrete strength of 5 ksi typical in current practice
- ACI 349 limits headed bars and shear reinforcement to Grade 60



1. High-Strength Materials

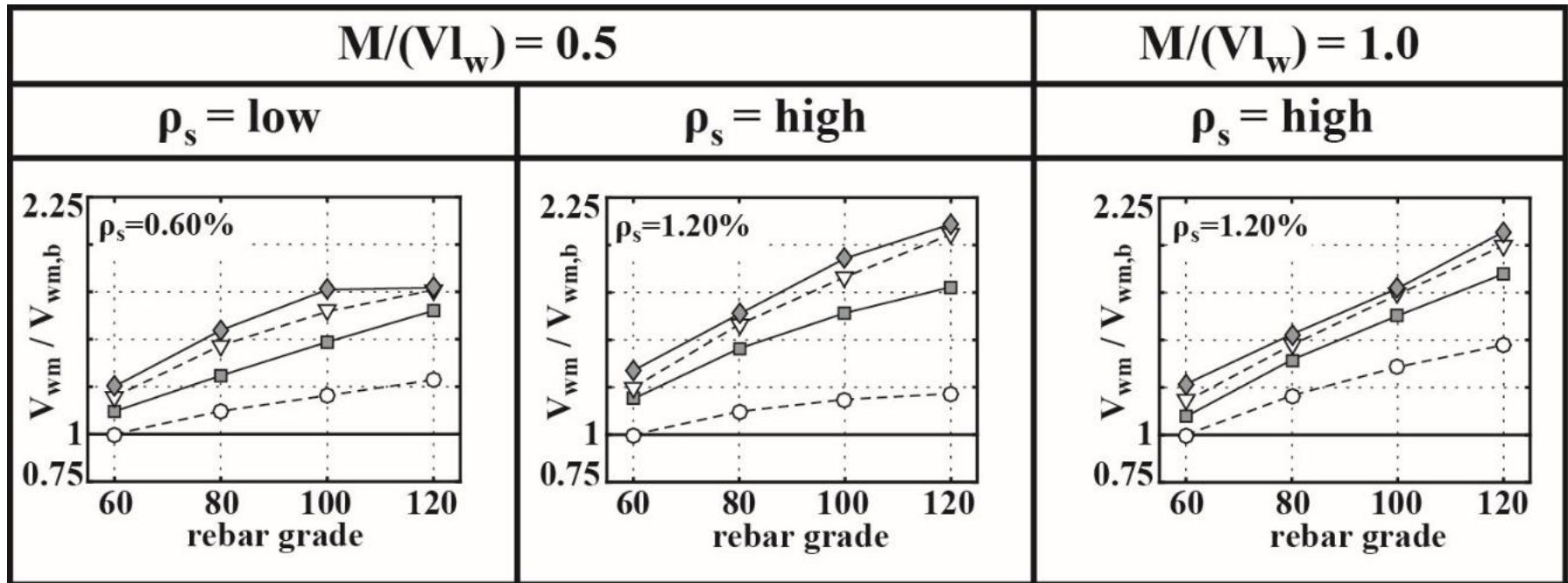
Numerical limit-benefit study to establish effects of high-strength materials on peak lateral strength of low-aspect-ratio shear walls:

- Parametric numerical investigation of 192 walls
- Peak strength predicted via finite element model

Parameter	Wall 1	Wall 2	Wall 3
length, l_w (ft)	20	60	120
height, h_w (ft)	40	120	120
thickness, t_w (in.)	15	45	45
moment to shear ratio, $M/(Vl_w)$	0.5, 1.0	0.5, 1.0	0.5, 1.0
concrete strength, f'_c (ksi)	5, 10, 15, 20	5, 10, 15, 20	5, 10, 15, 20
rebar strength, f_y (ksi)	60, 80, 100, 120	60, 80, 100, 120	60, 80, 100, 120
reinforcement ratio, ρ_s (%)	0.25, 0.50	0.60, 1.20	0.60, 1.20

1. High-Strength Materials

Results for Wall 2 (60 ft x 120 ft x 45 in.):



--○-- $f'_c = 5.00$ ksi

--□-- $f'_c = 10.0$ ksi

--▽-- $f'_c = 15.0$ ksi

--◇-- $f'_c = 20.0$ ksi

V_{wm} = Predicted peak lateral strength

$V_{wm,b}$ = Predicted peak lateral strength of “benchmark” with normal strength materials

1. High-Strength Materials

Summary of results of limit-benefit analysis

- Combination of high-strength rebar with high-strength concrete resulted in a higher-performing structure than with either high-strength material on its own
- Significant benefits by using concrete strength of $f'_c = 10$ ksi, with diminishing returns for higher strengths
- Greatest benefits of high-strength materials for walls with large rebar ratios, ρ_s

1. High-Strength Materials

Numerical cost-benefit study of economic effectiveness of high-strength materials for low-aspect-ratio shear walls:

- Parametric numerical investigation of 2304 walls
- Construction cost metric (Γ) includes rebar material cost, rebar labor cost, and concrete material cost (C_w), normalized by peak strength (V_{wm}): $\Gamma = \frac{C_w}{V_{wm}}$

Parameter	Wall 1	Wall 2	Wall 3
length, l_w (ft)	20	60	120
height, h_w (ft)	40	120	120
thickness, t_w (in.)	10, 15 , 20	30, 45 , 60	30, 45 , 60
moment to shear ratio, $M/(Vl_w)$	0.5 , 1.0	0.5 , 1.0	0.5 , 1.0
concrete strength, f'_c (ksi)	5 , 10, 15, 20	5 , 10, 15, 20	5 , 10, 15, 20
rebar strength, f_y (ksi)	60 , 80, 100, 120	60 , 80, 100, 120	60 , 80, 100, 120
reinforcement ratio, ρ_s (%)	low to high	low to high	low to high

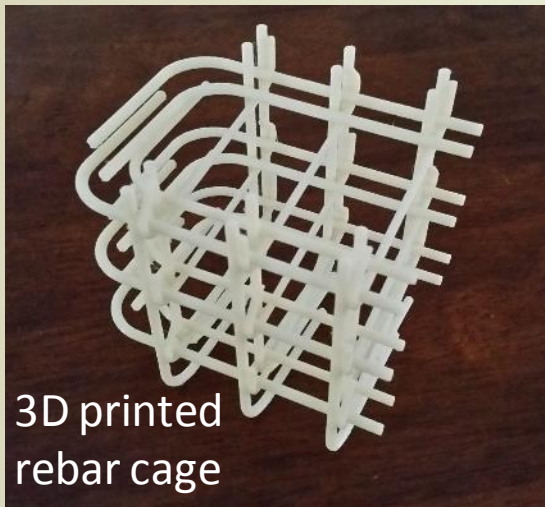
1. High-Strength Materials

Summary of results of cost-benefit analysis:

- Combination of high-strength rebar with high-strength concrete resulted in greatest economic benefits for walls with lower $M/(Vl_w)$ ratios and large reinforcement ratios, ρ_s
- A concrete strength of $f'_c = 10$ ksi showed the largest incremental reduction in construction cost
- Rebar grades greater than 100 can lead to decreased economic benefits due to the increased unit cost

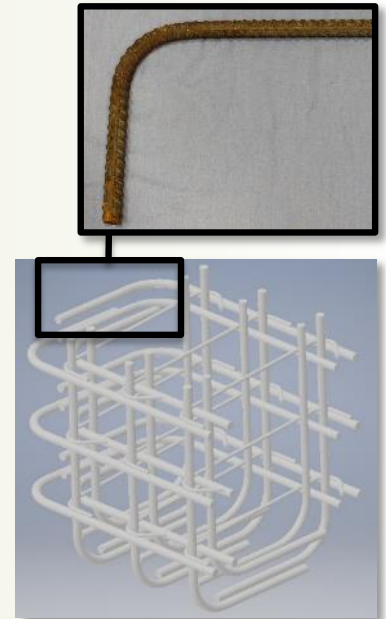
2. Prefabricated Rebar

- Evaluating prefab rebar cages for:
 - transportability
 - liftability
 - modularity
- Using mini-scale rapid prototyping



**Most Congested
(current)**

*Multiple layers
of hooked
Grade 60 bars*



*Fewer layers
of headed high-
strength bars*

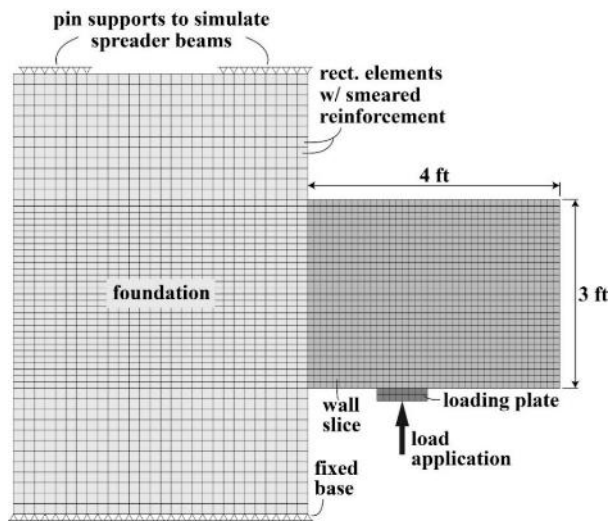


**Least Congested
(envisioned)**

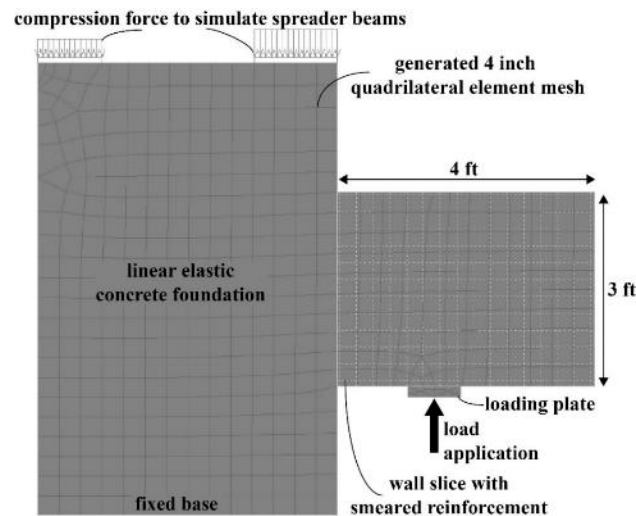


3. Optimization, Modeling and Design

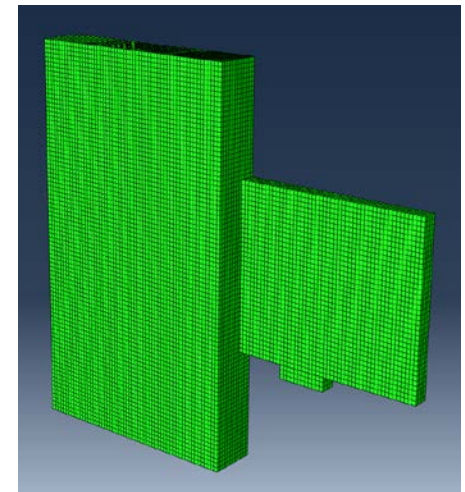
Pre-test Analyses of Deep Beam (Wall Slice) and Shear Wall Specimens in Vector2, ATENA, and ABAQUS



Vector2



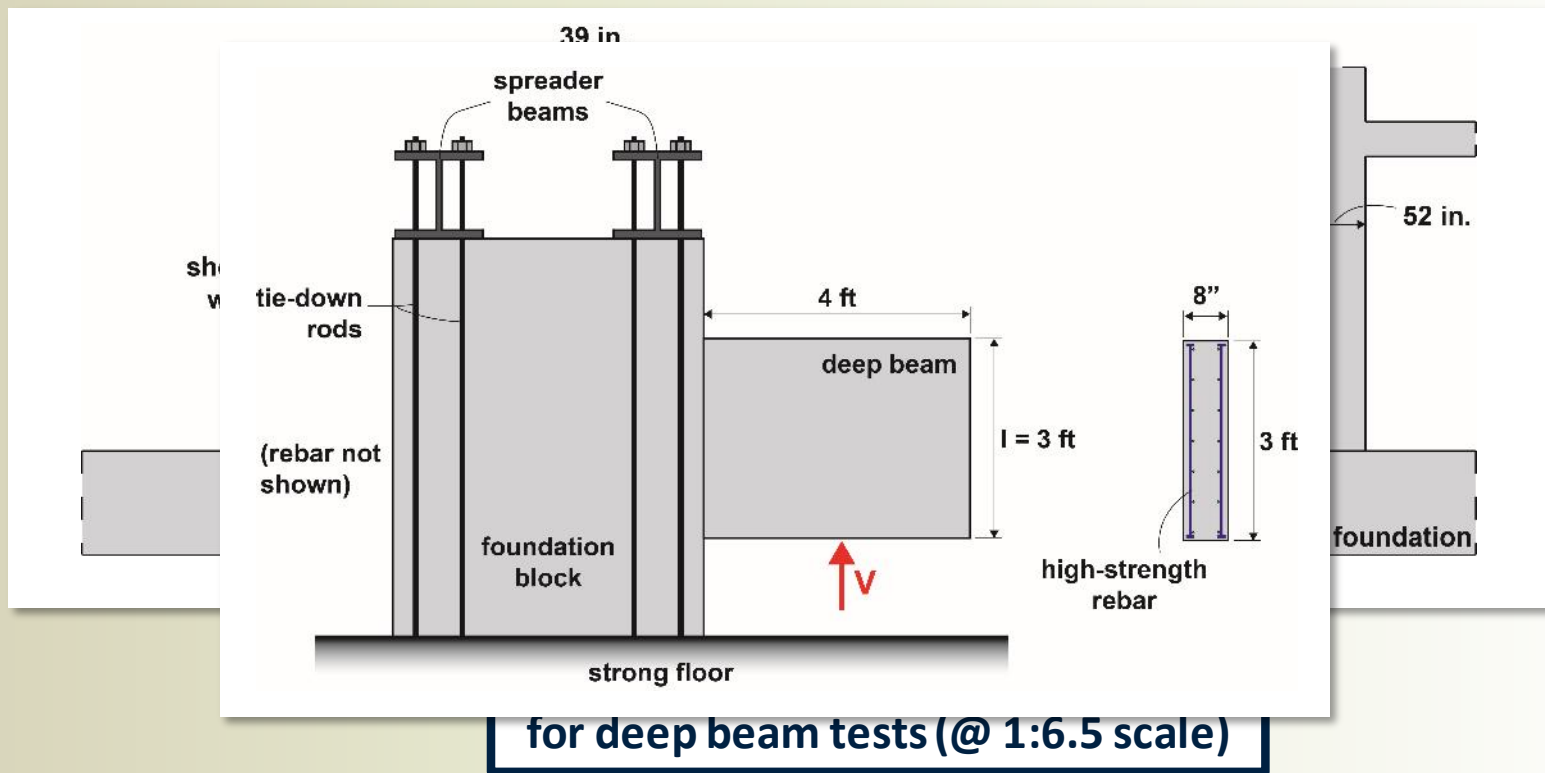
ATENA



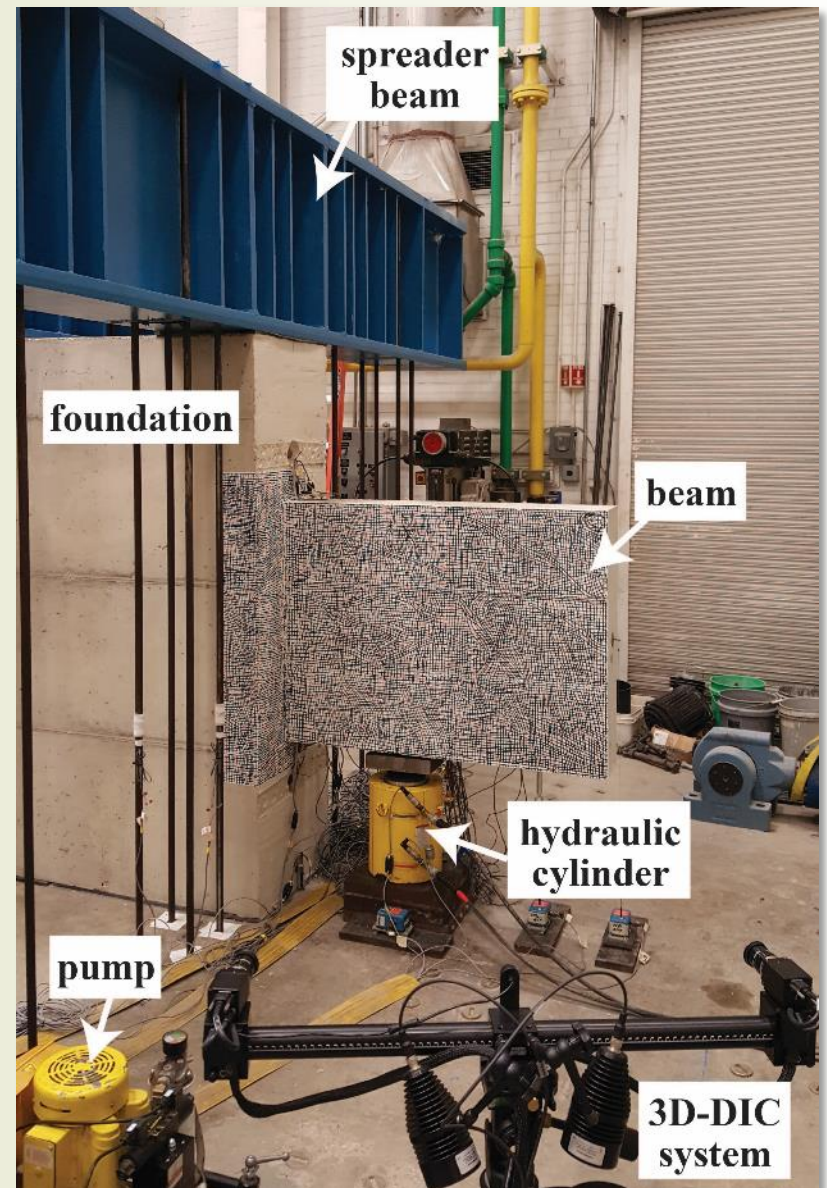
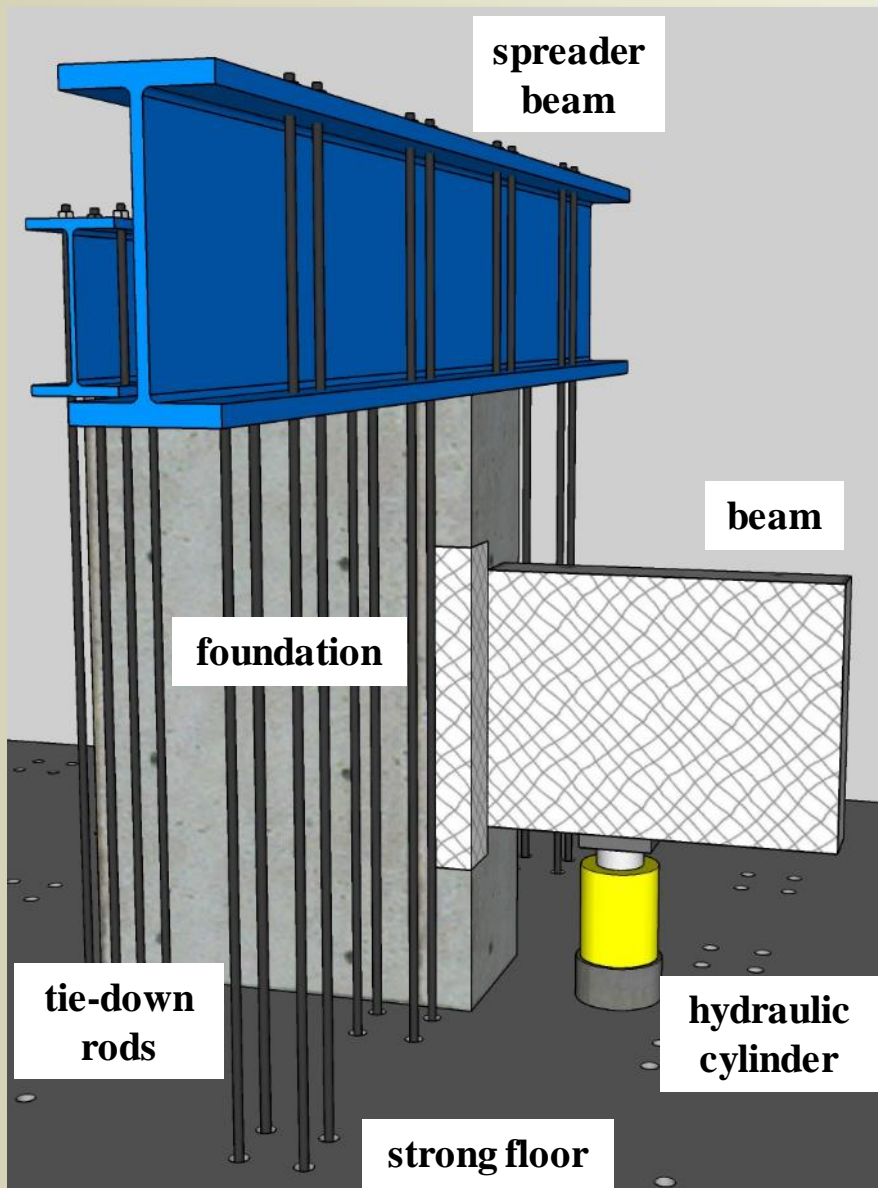
ABAQUS

4. Experimental Testing

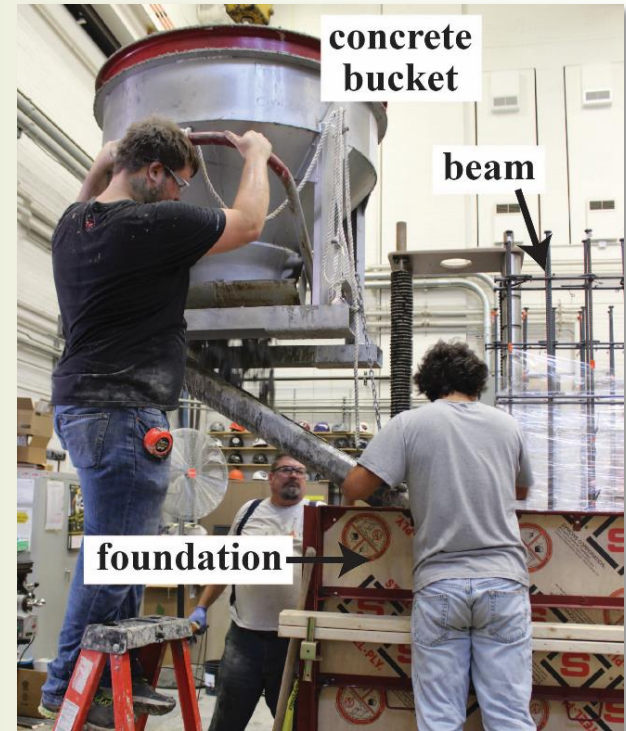
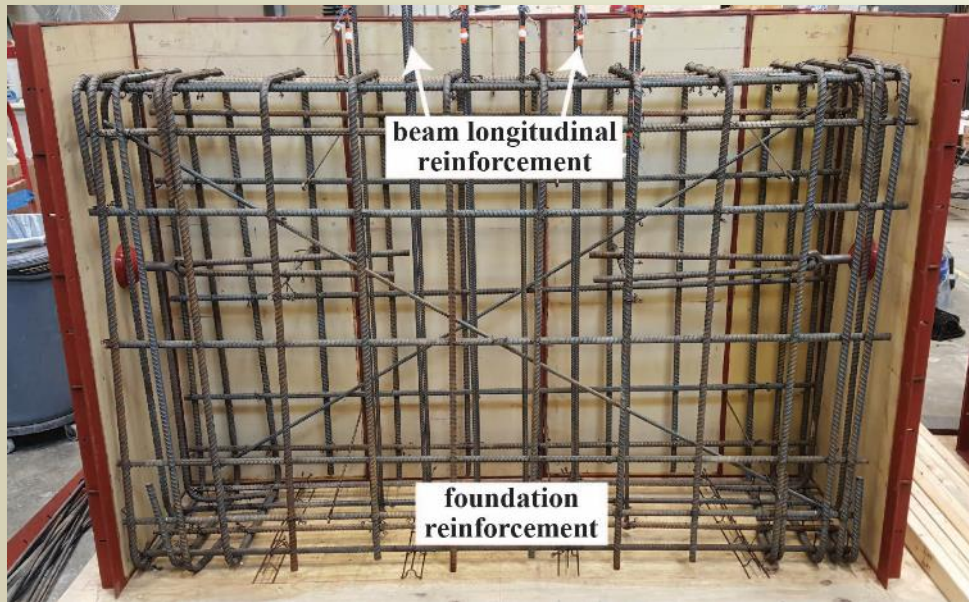
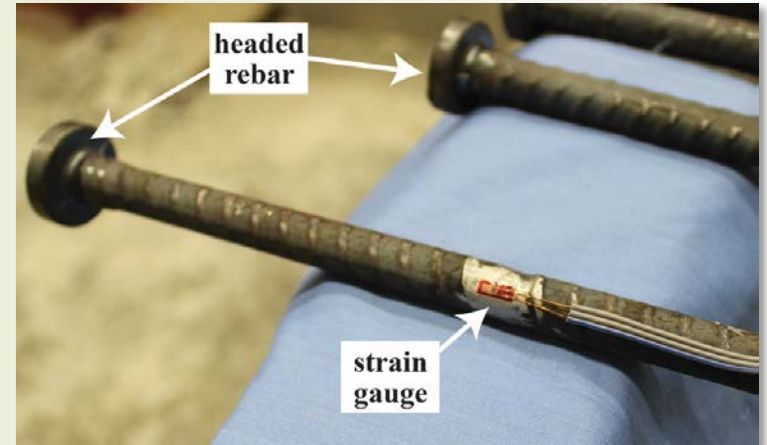
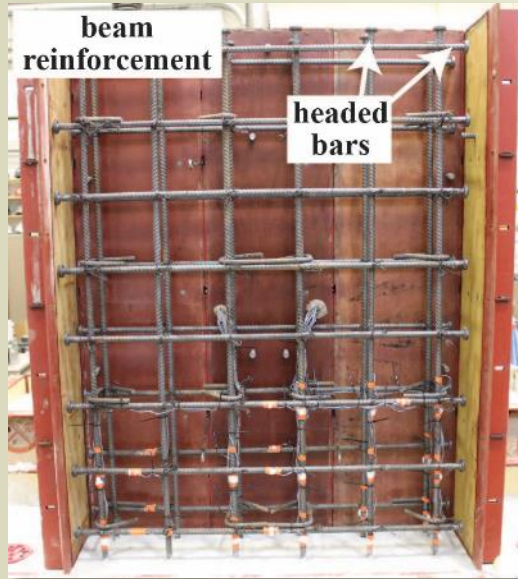
- “Generic wall” dimensions determined using publicly-available design control documents
- Provides basis for future deep beam and shear wall tests



Deep Beam Test Setup



Deep Beam Construction



Deep Beam Construction



Normal-Strength Concrete

$f'_c = 6500$ psi

slump = 8 in.



High-Strength Concrete

$f'_c = 14690$ psi

slump = 8.75 in.

Deep Beam Test Parameters

Specimen	f'_c (psi)	f_y (ksi)	ρ_s (%)	$M/(Vl_w)$
DB1	6500	70	0.833	0.5
DB2	6500	133	0.833	0.5
DB3	14960	70	0.833	0.5
DB4	14960	133	0.833	0.5

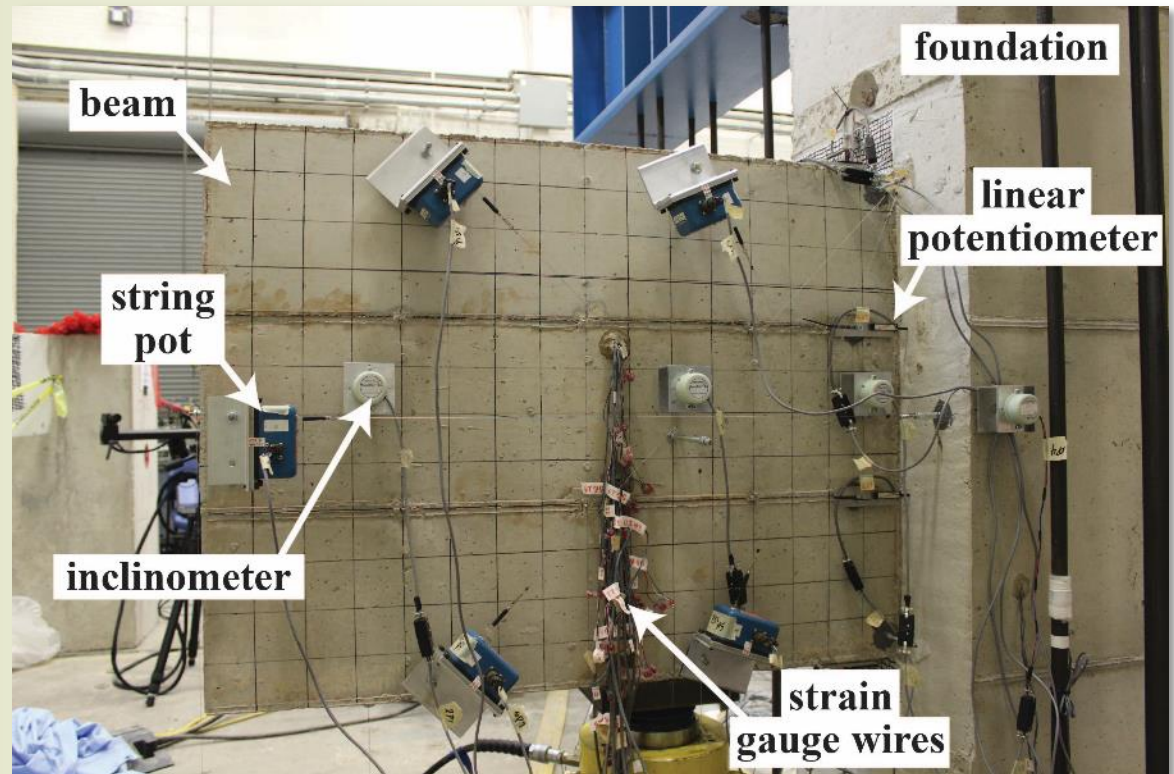
Definitions: f'_c – concrete 28 day compressive strength

f_y – rebar yield strength, determined by tensile tests and 0.2% offset method

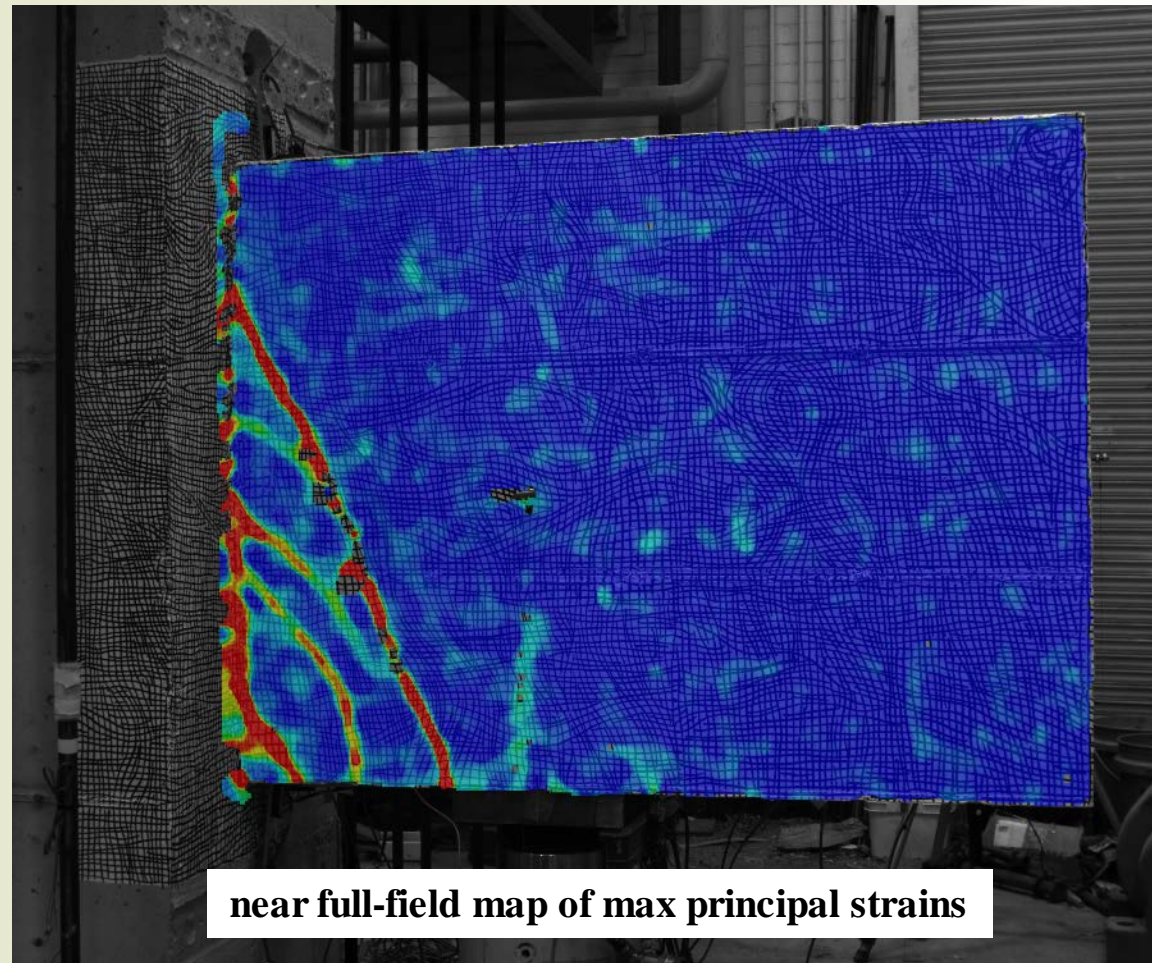
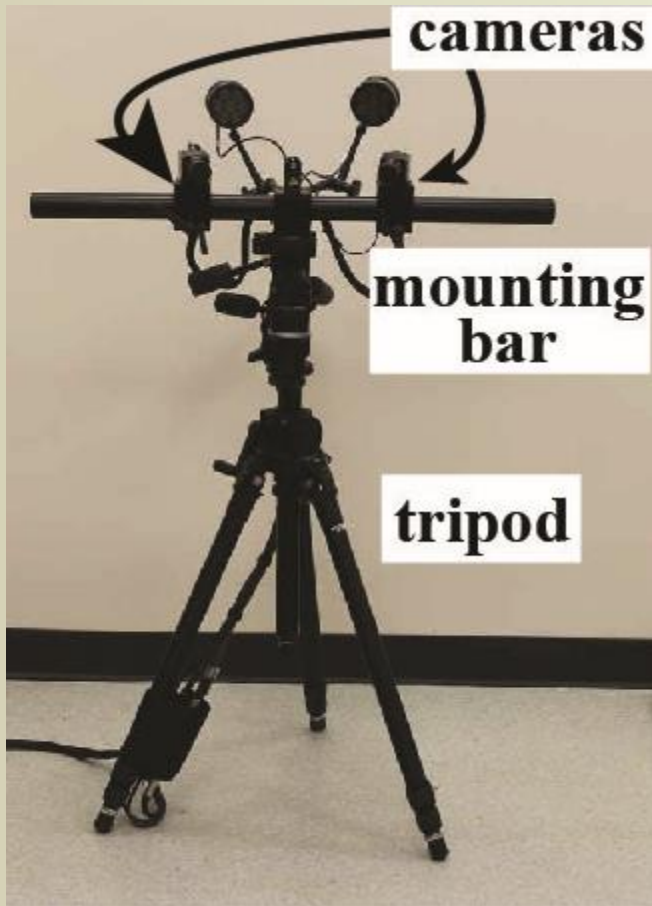
ρ_s – reinforcement ratio, symmetric for longitudinal and transverse rebar

Deep Beam Instrumentation

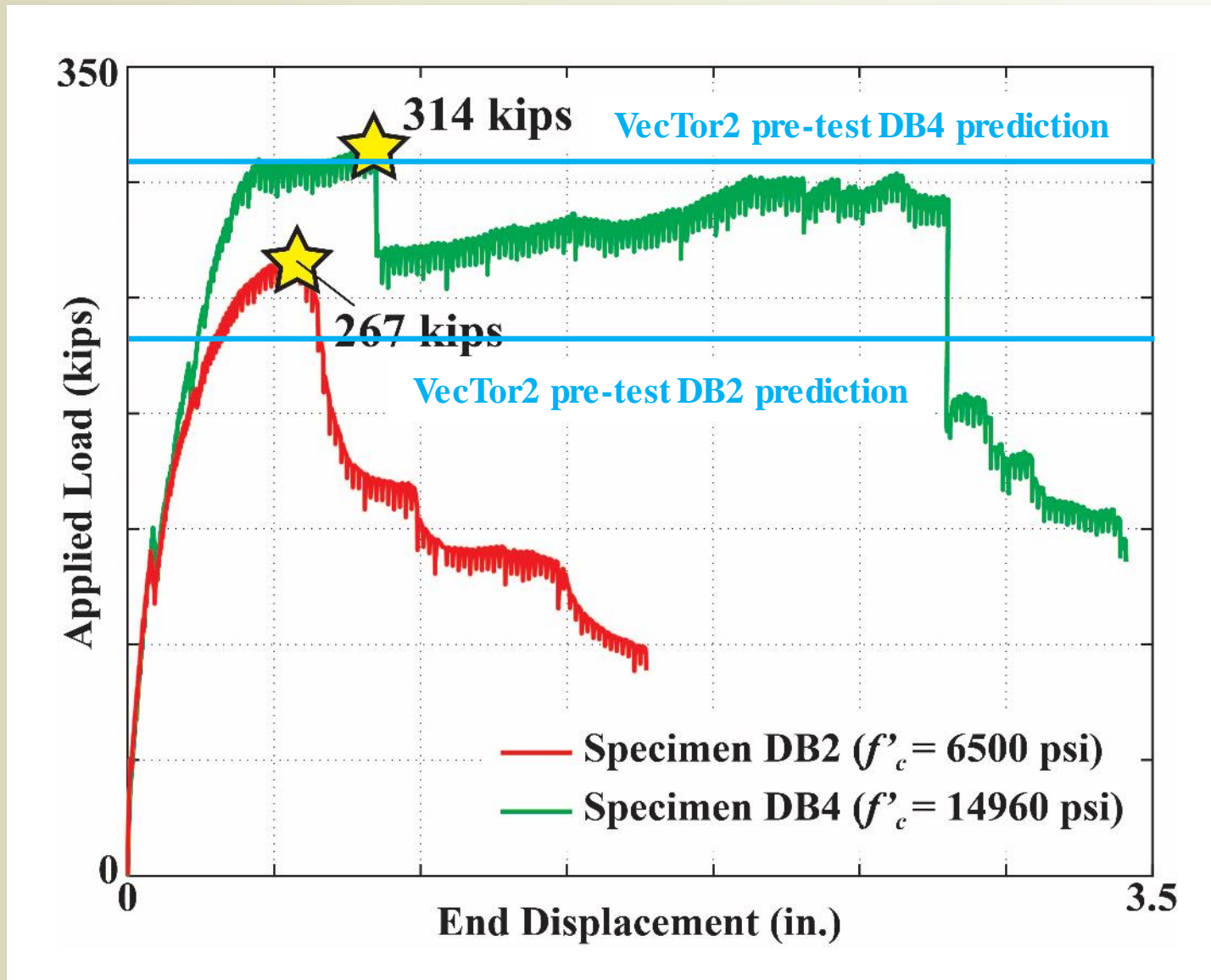
Type	Number
pressure transducer	2
string potentiometer	9
linear potentiometer	8
inclinometer	4
strain gauge	42
TOTAL	65



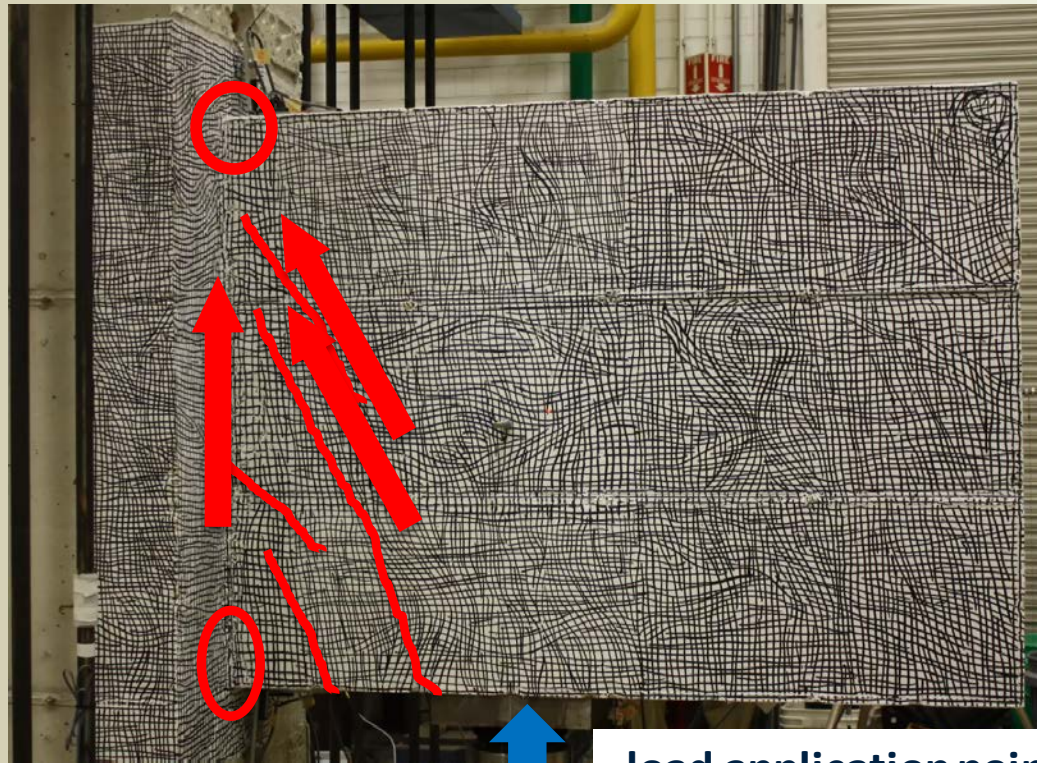
3D Digital Image Correlation



Specimens DB2 and DB4 Response

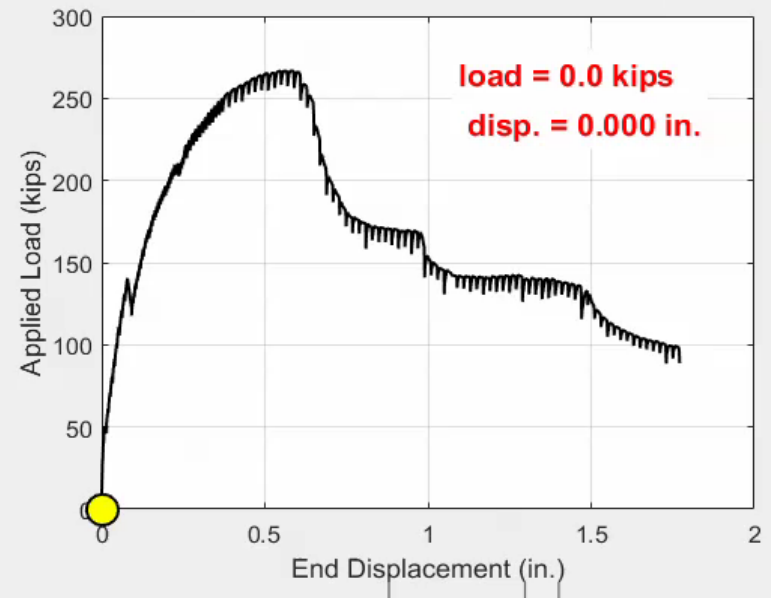


DB2 ($f'_c = 6500$ psi, $f_y = 133$ ksi)

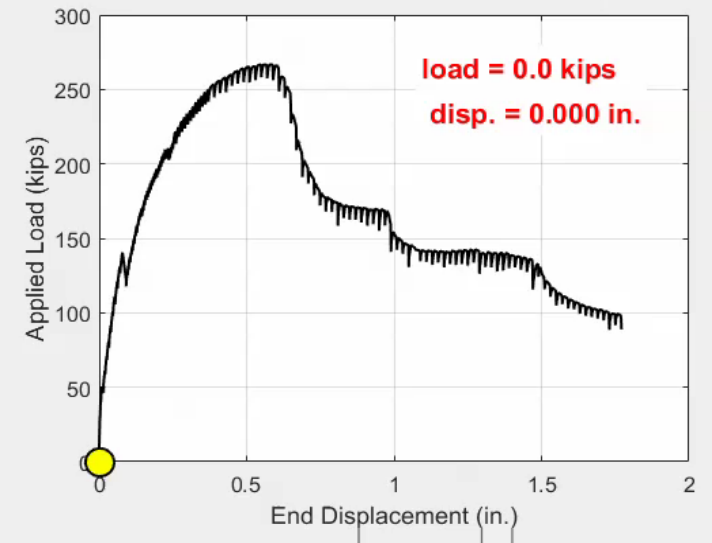
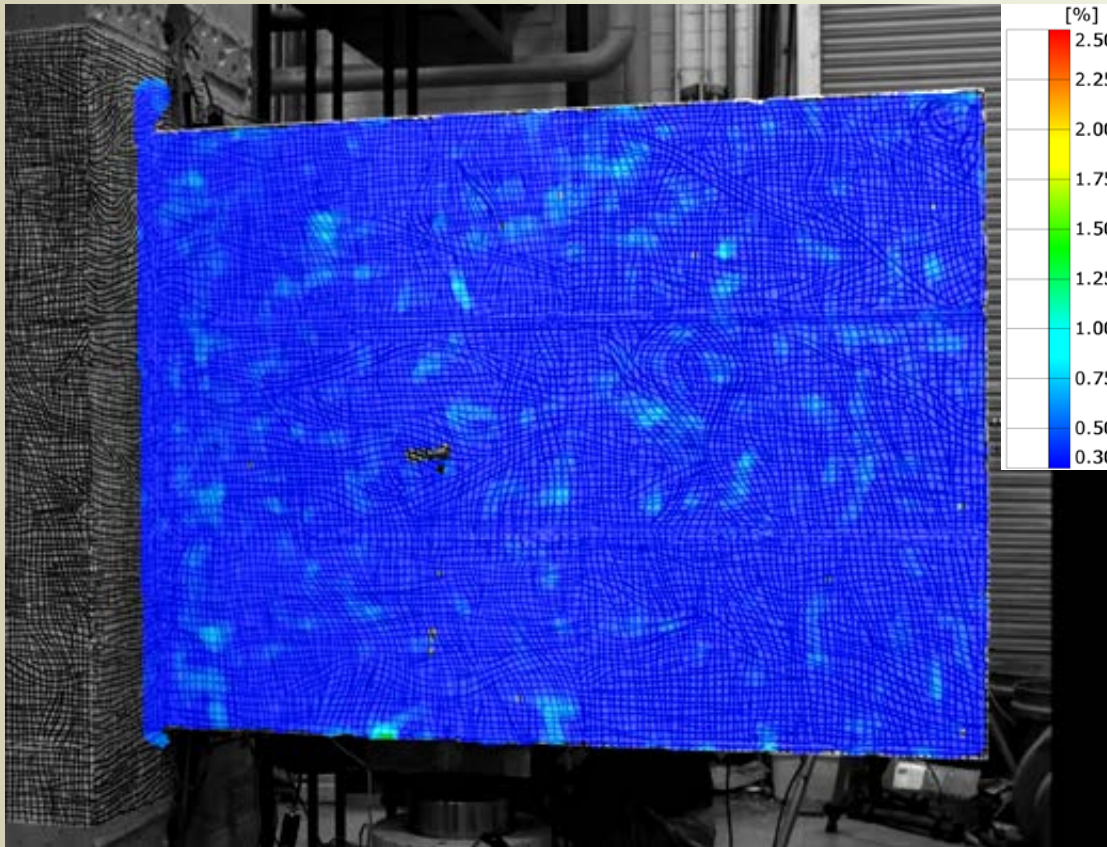


load application point

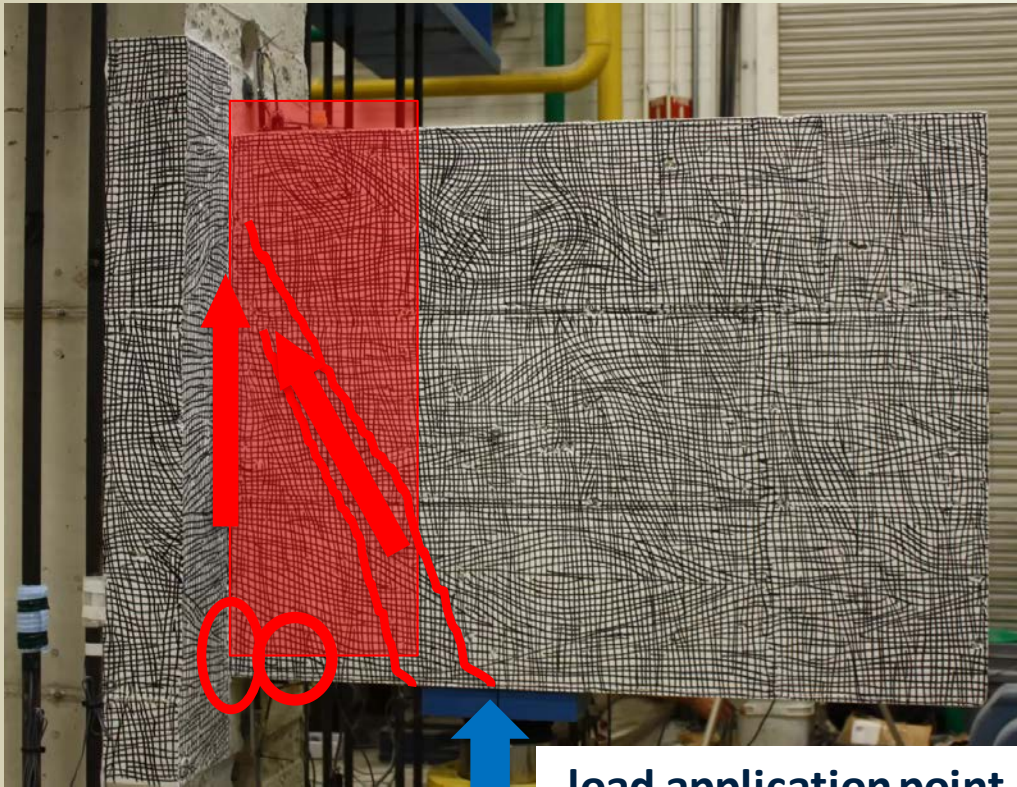
Slipping along 2nd diagonal crack



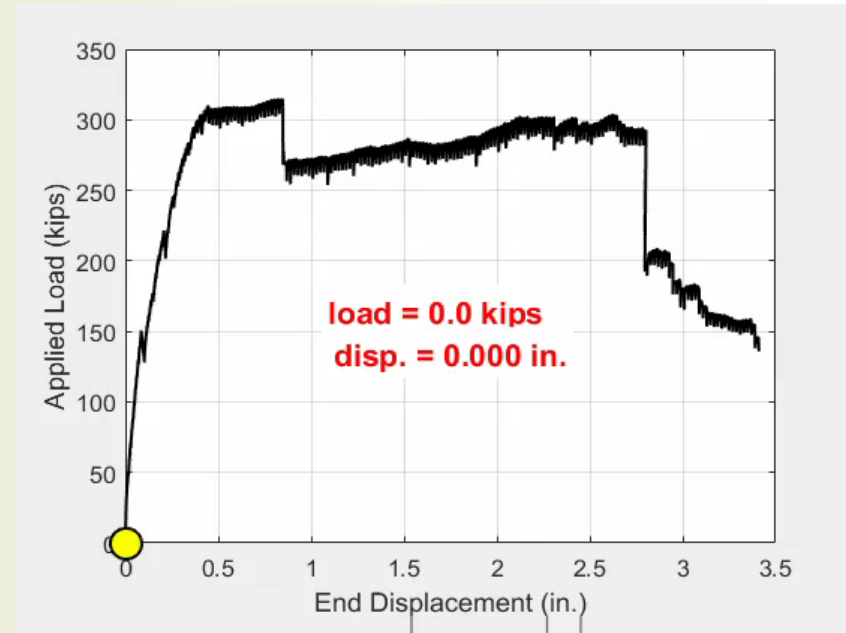
DB2 ($f'_c = 6500$ psi, $f_y = 133$ ksi)



DB4 ($f'_c = 14960$ psi, $f_y = 133$ ksi)

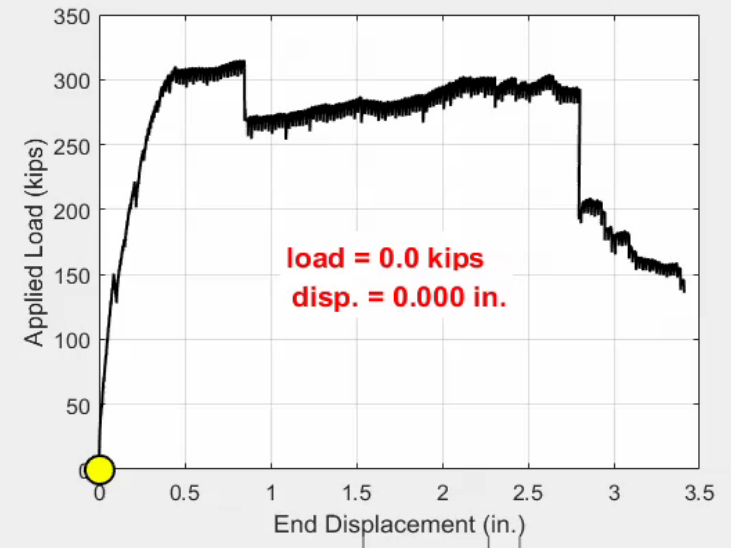
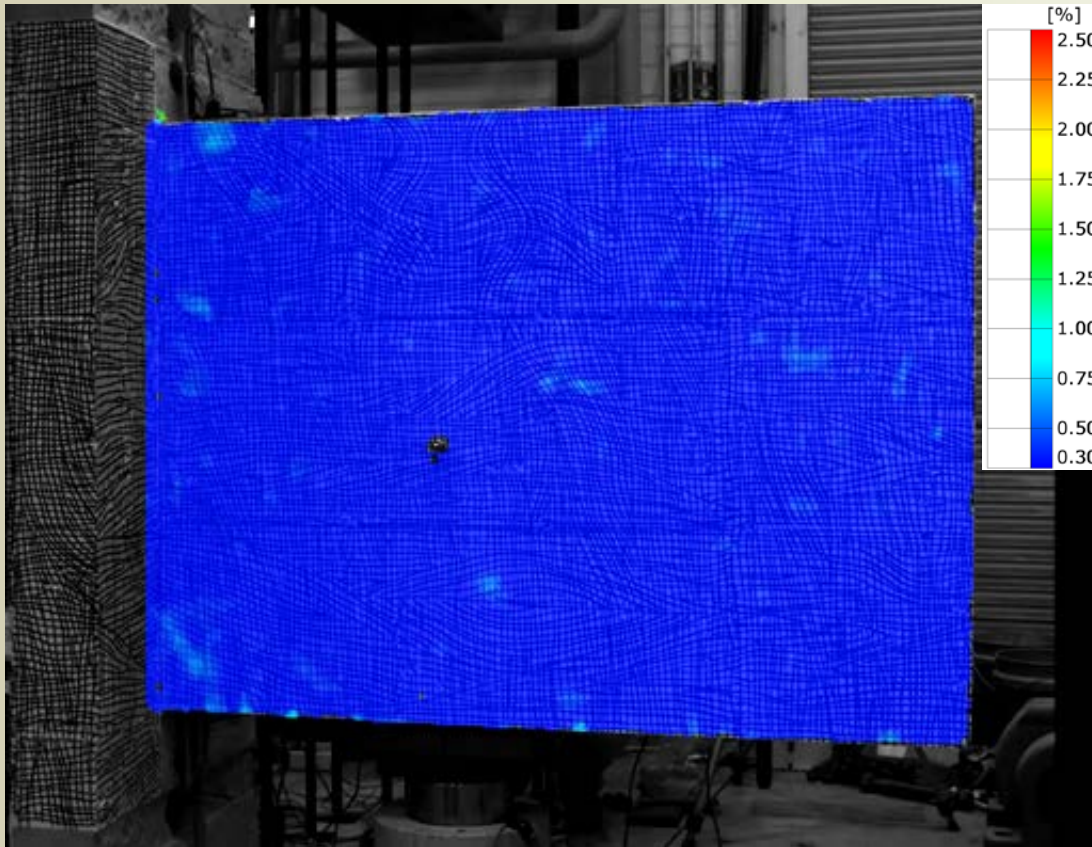


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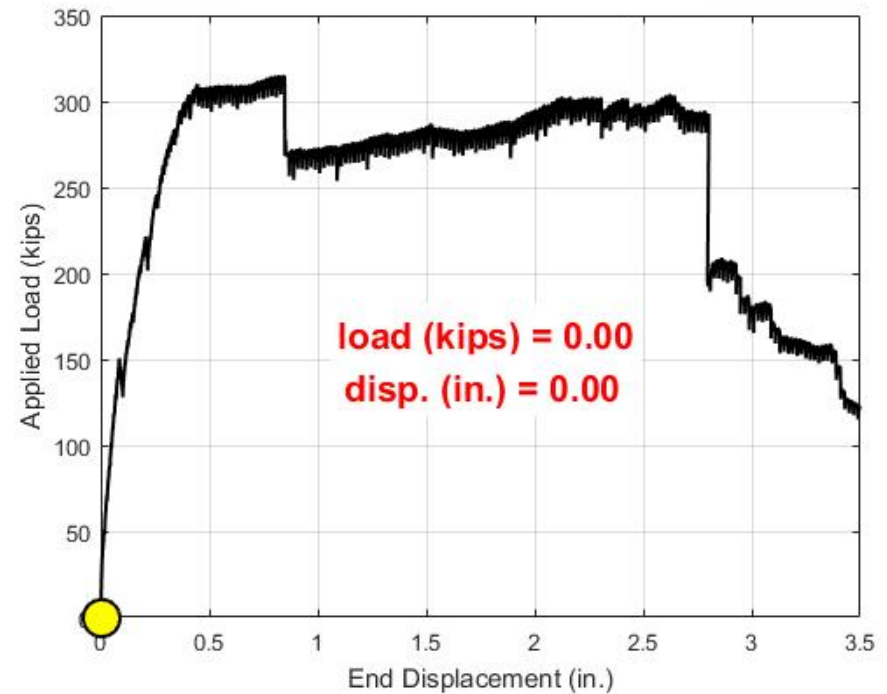
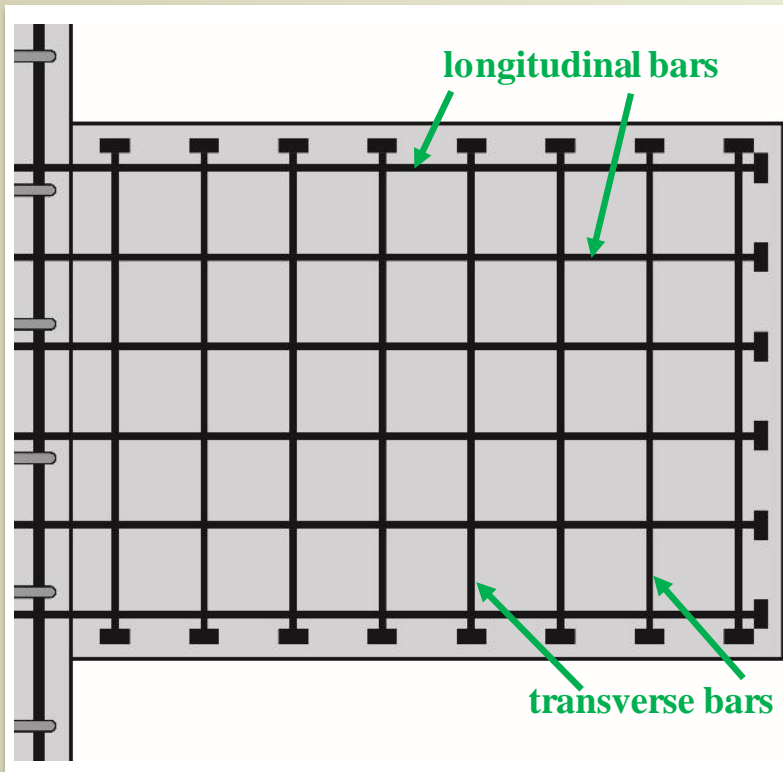


Significant concrete degradation through beam depth

DB4 ($f'_c = 14960$ psi, $f_y = 133$ ksi)

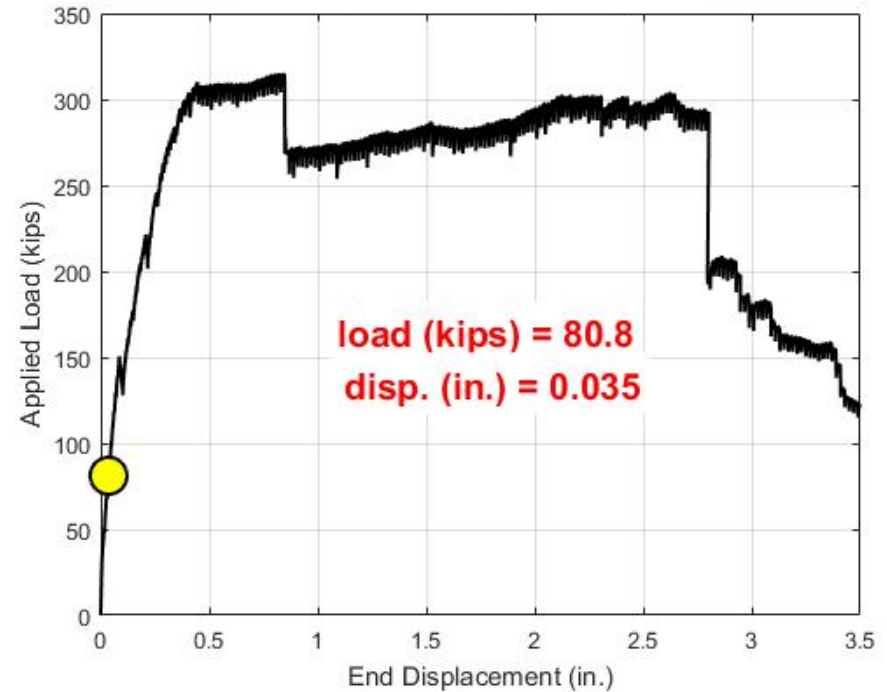
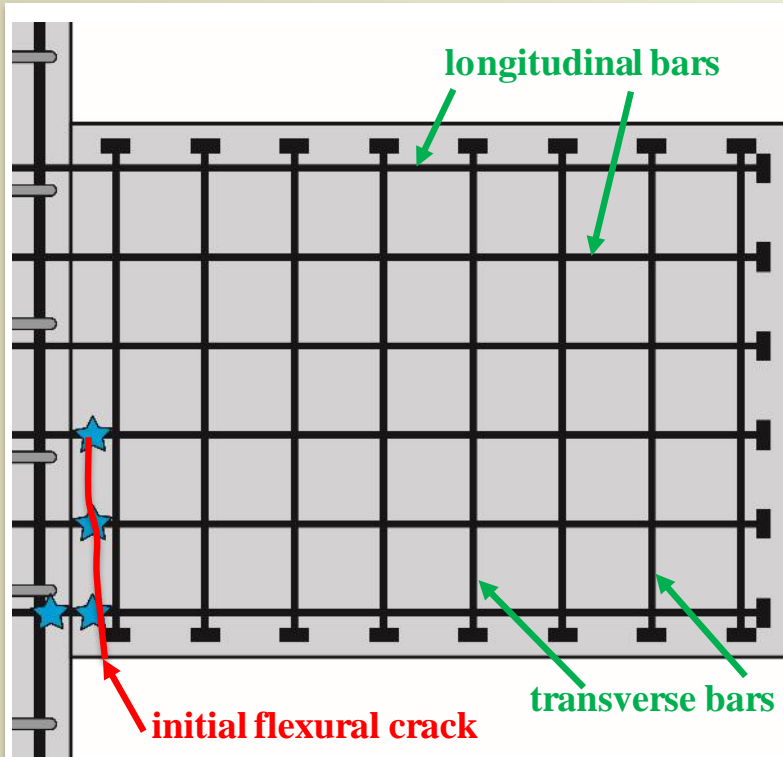


DB4 ($f'_c = 14960$ psi, $f_y = 133$ ksi)



- ★ active tension strain
- ★ tension yield (6.85 m ϵ)

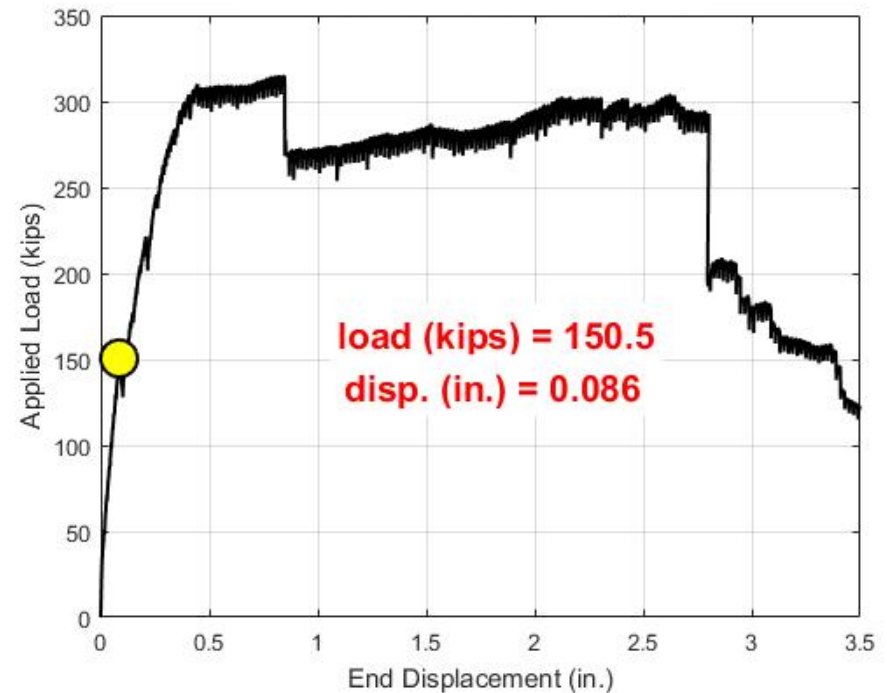
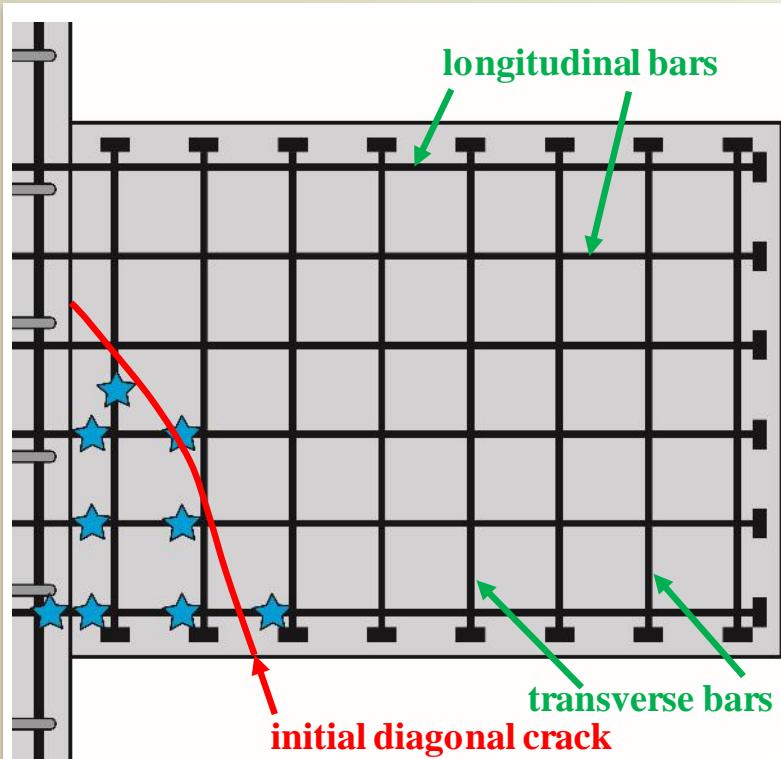
DB4 ($f'_c = 14960$ psi, $f_y = 133$ ksi)



- ★ active tension strain
- ★ tension yield (6.85 mε)

Initial flexural cracking, bottom three longitudinal layers active in tension

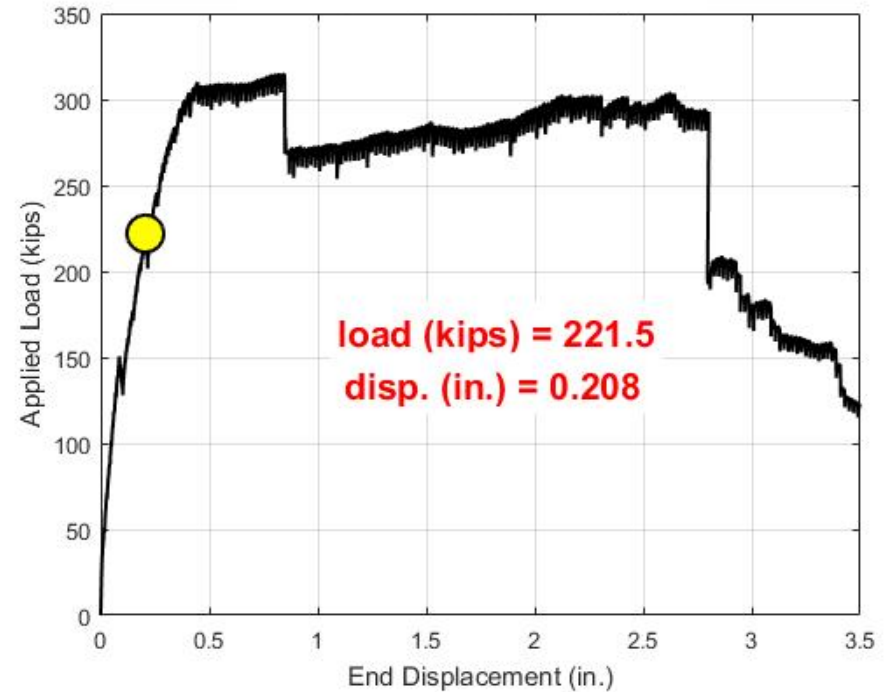
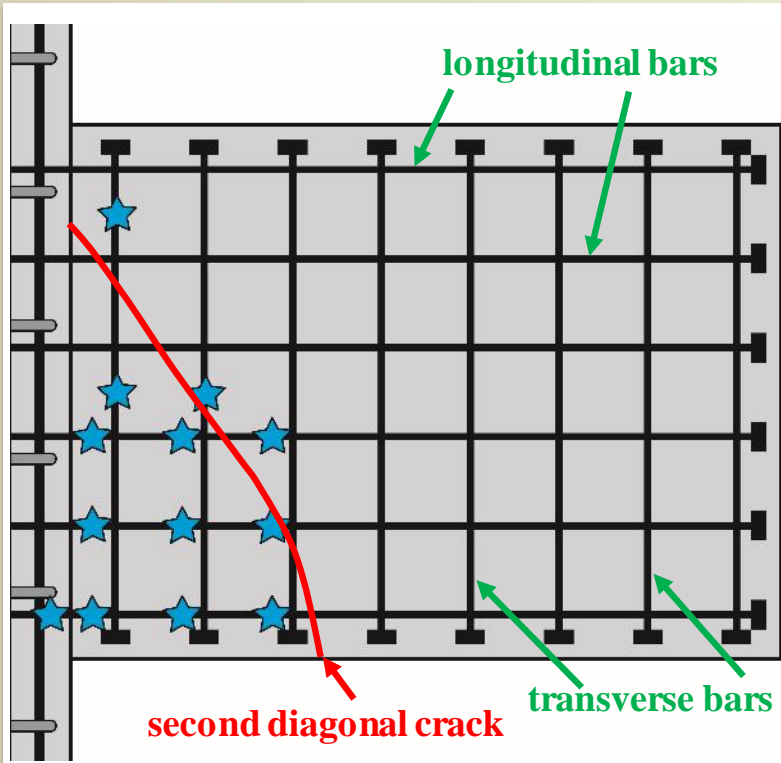
DB4 ($f'_c = 14960$ psi, $f_y = 133$ ksi)



- ★ active tension strain
- ★ tension yield (6.85 m ϵ)

Bottom three longitudinal layers and closest transverse layer to foundation strain to arrest diagonal crack

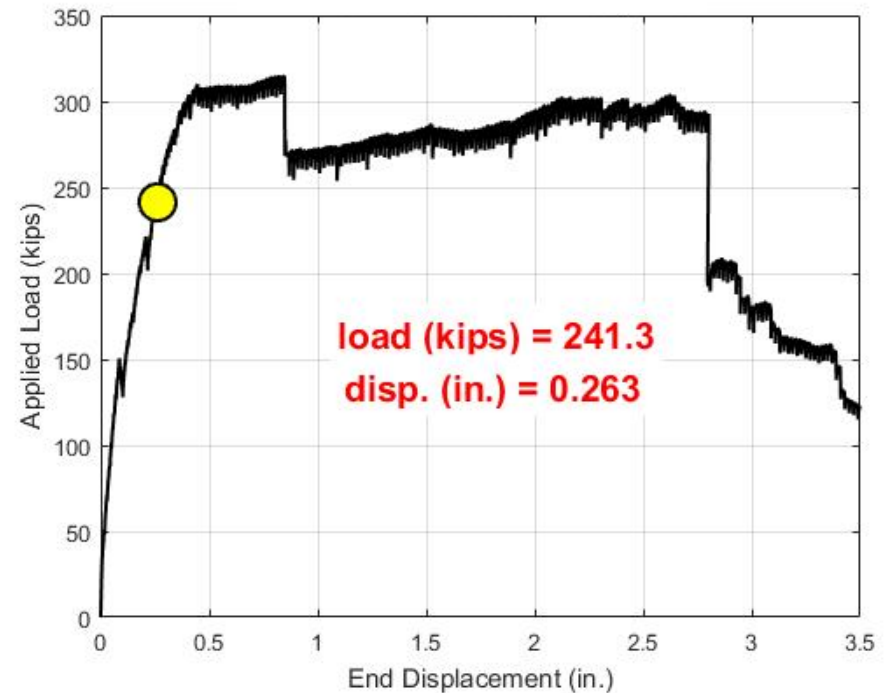
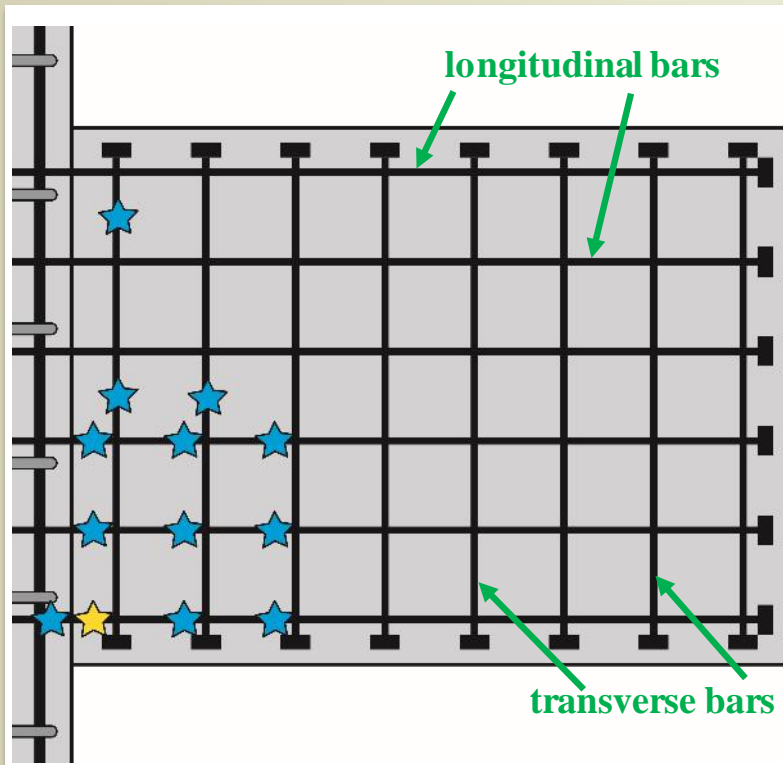
DB4 ($f'_c = 14960$ psi, $f_y = 133$ ksi)



- ★ active tension strain
- ★ tension yield (6.85 m ϵ)

Two transverse bar layers and two longitudinal bar layers above the bottom experience strain increase

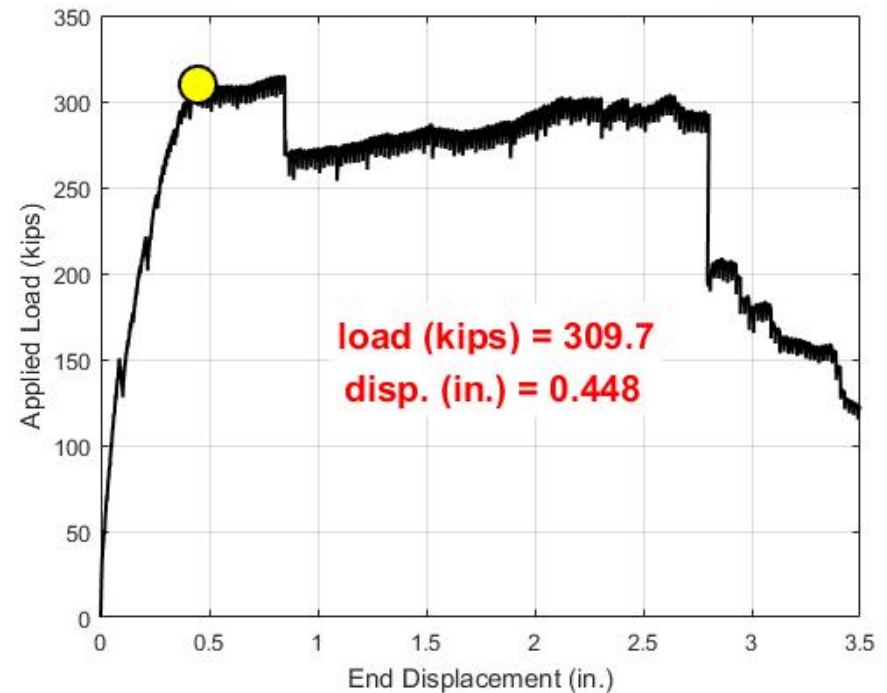
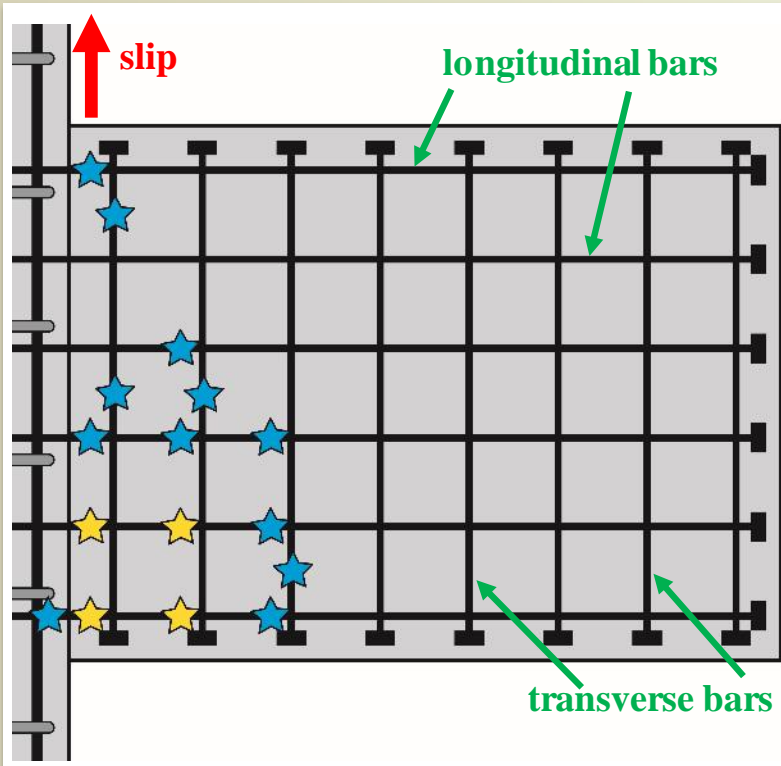
DB4 ($f'_c = 14960$ psi, $f_y = 133$ ksi)



- ★ active tension strain
- ★ tension yield (6.85 $m\epsilon$)

Initiation of longitudinal reinforcement yielding

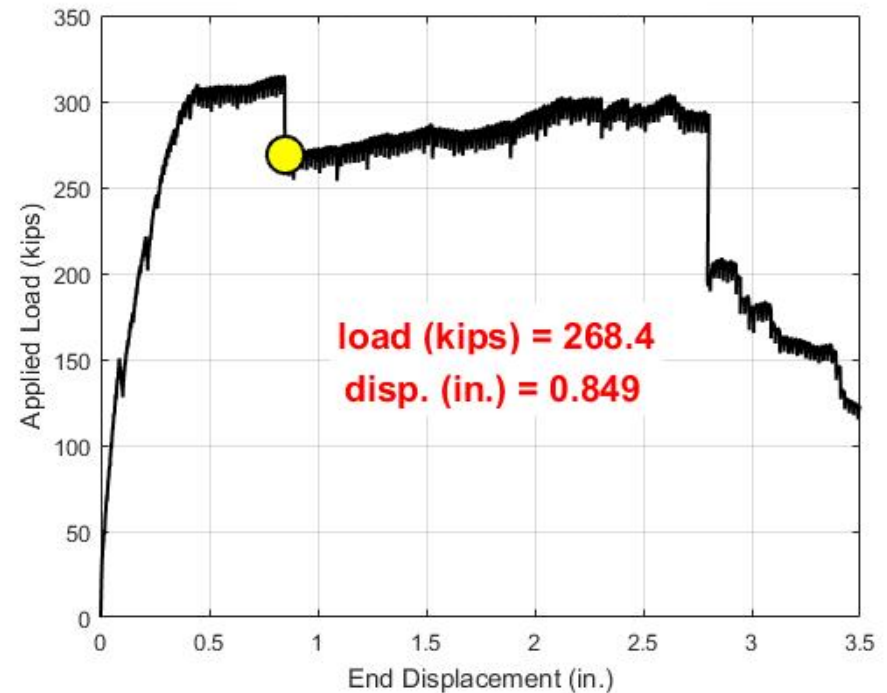
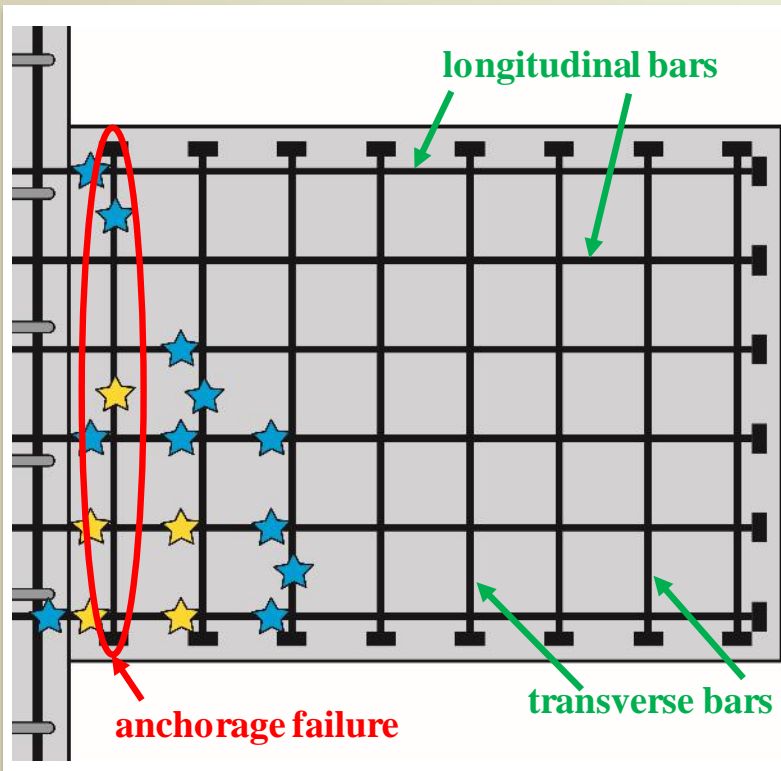
DB4 ($f'_c = 14960$ psi, $f_y = 133$ ksi)



- ★ active tension strain
- ★ tension yield (6.85 mε)

Slip at foundation interface
Extensive yielding of longitudinal reinforcement

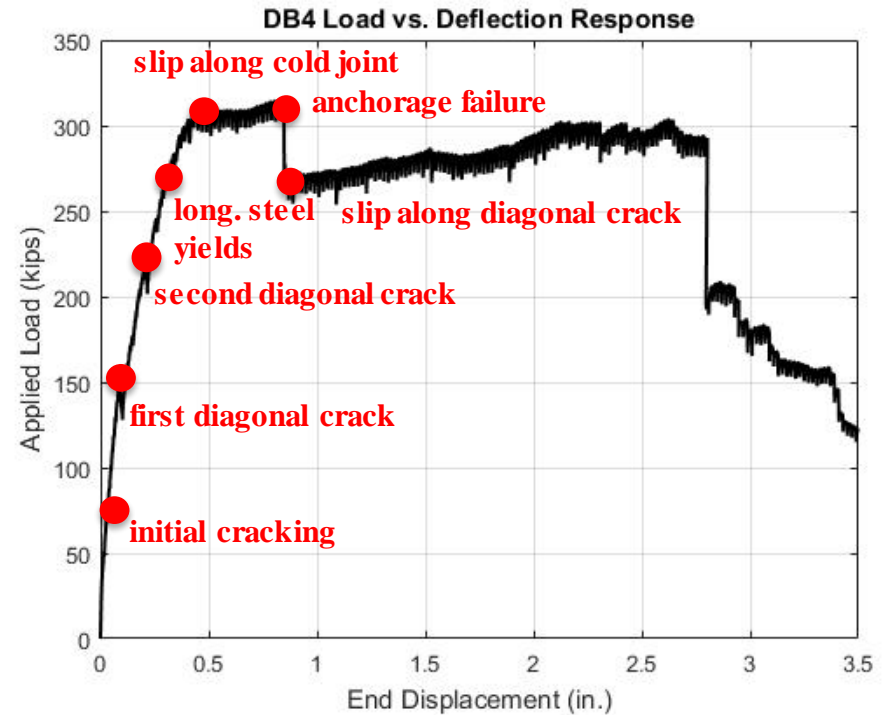
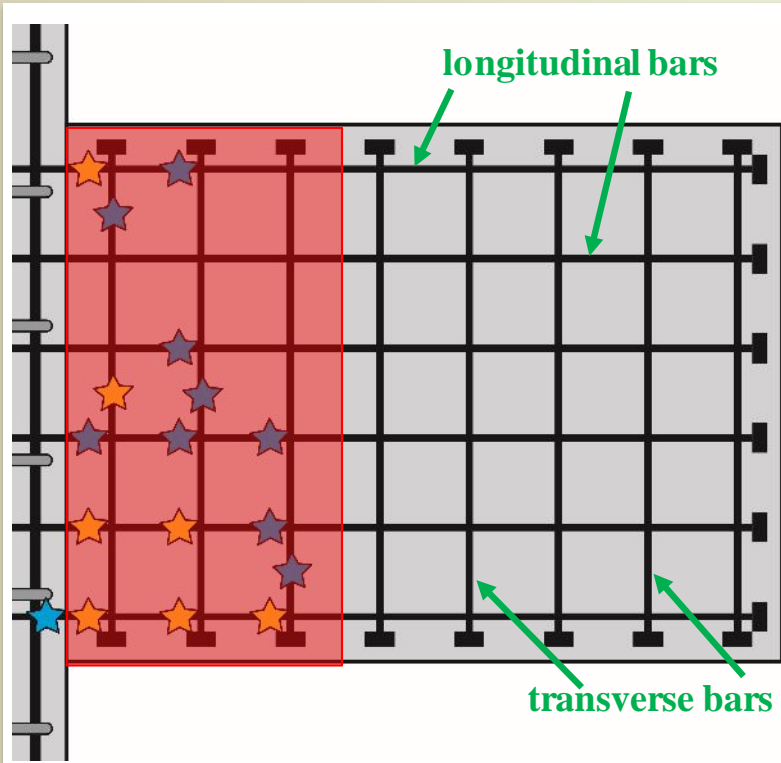
DB4 ($f'_c = 14960$ psi, $f_y = 133$ ksi)



- ★ active tension strain
- ★ tension yield ($6.85 m\epsilon$)

Anchorage failure of first transverse bar after yielding to arrest diagonal cracks

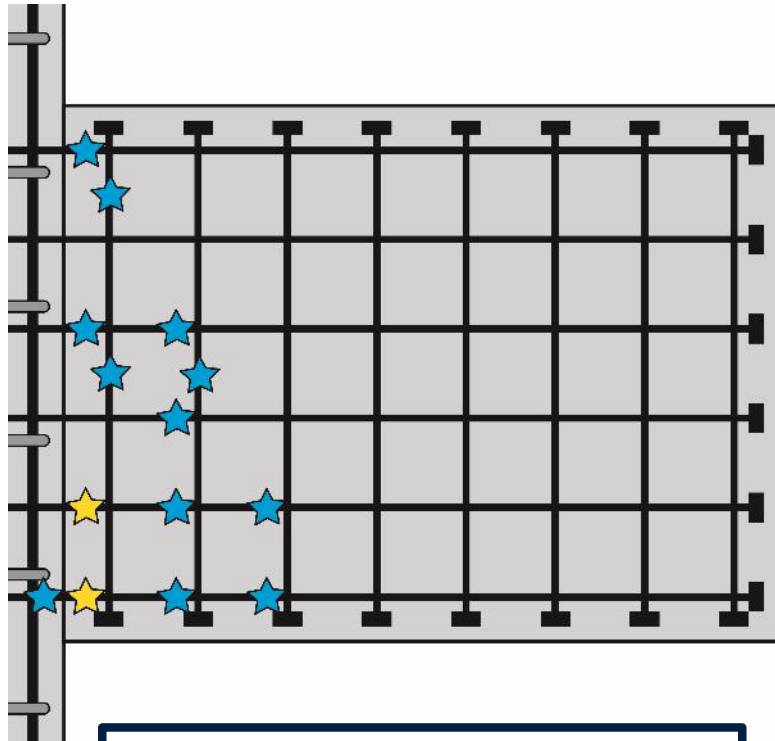
DB4 ($f'_c = 14960$ psi, $f_y = 133$ ksi)



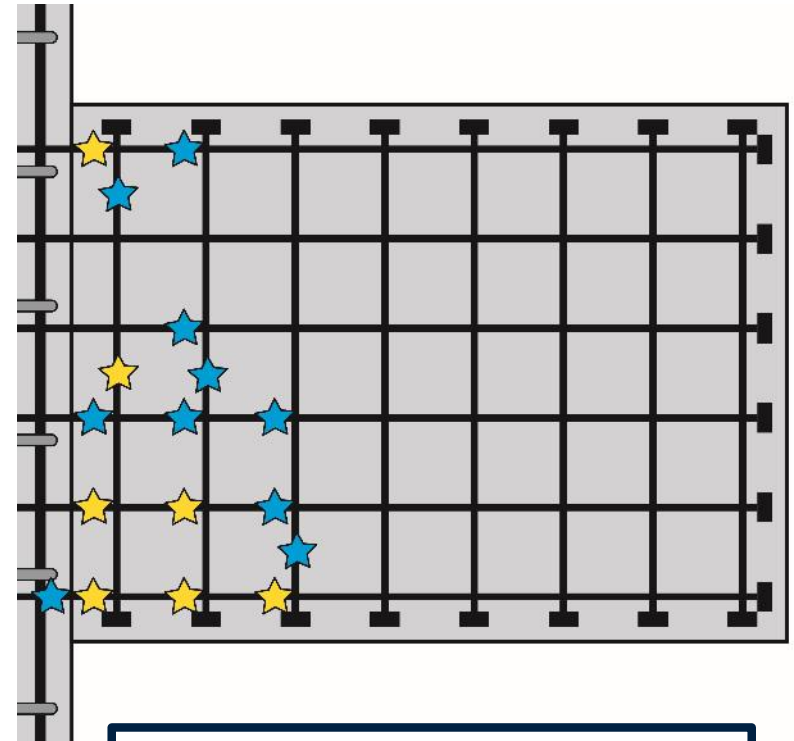
- ★ active tension strain
- ★ tension yield (6.85 mε)

Extensive concrete degradation

DB2 and DB4 Strain Comparisons



DB2 $f'_c = 6500$ psi $f_y = 133$ ksi



DB4 $f'_c = 14960$ psi $f_y = 133$ ksi

- ★ active tension strain
- ★ tension yield (6.85 mε)

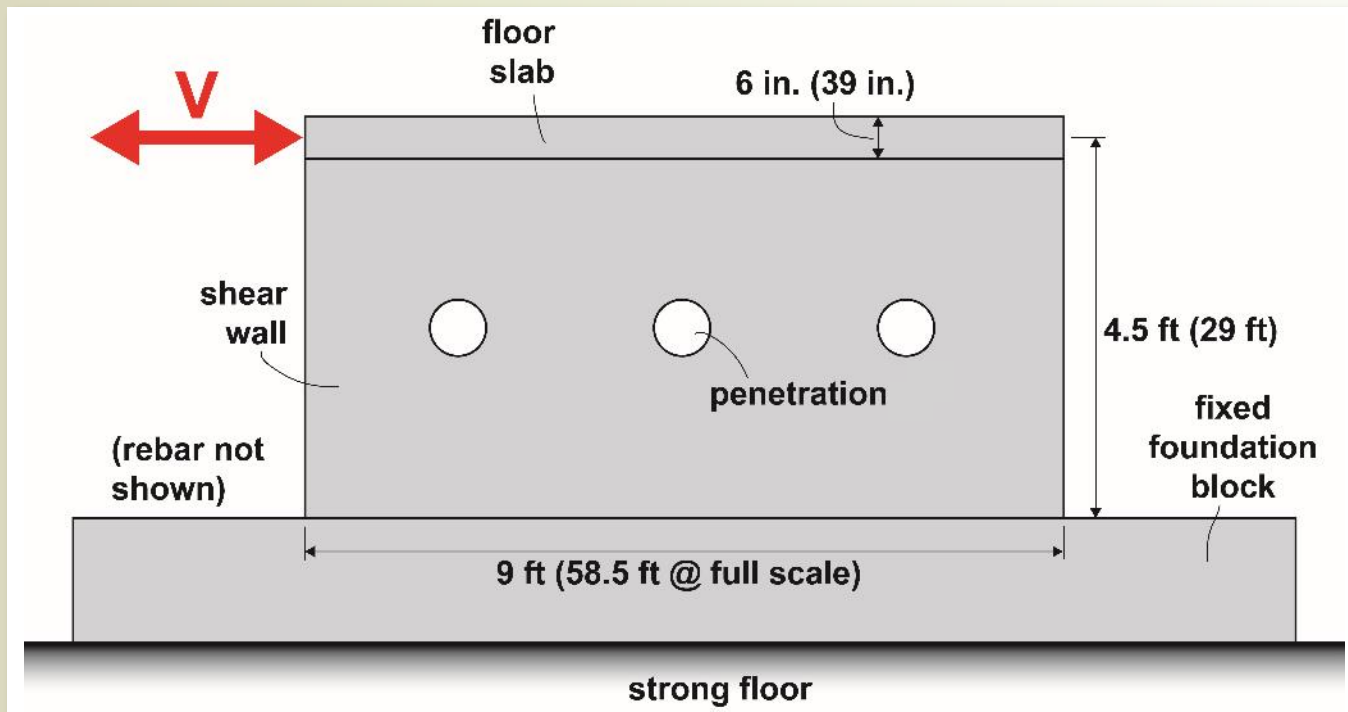
High-strength concrete able to better take advantage of higher yield strengths of reinforcement

Summary of Deep Beam Tests

- 17.6% increase in peak shear strength when increasing f'_c from 6500 psi to 14960 psi
- Significant increase in ductility due to increase in f'_c
- Pre-test analyses provided reasonable predictions for peak strength

Future Reduced-Scale Shear Wall Tests

- 1:6.5 scale of “generic wall”
- $M/(Vl_w) = 0.50$
- Tested under cyclic and accidental thermal loads
- High-strength steel and concrete



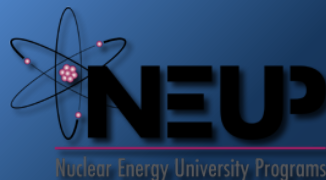
Conclusions to Date

- High-strength steel more effective when combined with high-strength concrete
 - Numerically demonstrated (economics and peak strength)
 - Measured experimentally
- Greatest benefit for walls with low moment-to-shear ratios and large reinforcement ratios; typical of nuclear concrete shear walls
- Largest economic and structural benefits when using Grade 100 rebar together with 10 ksi concrete
- Project tasks on schedule

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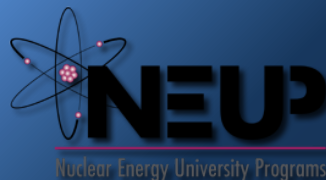


Research Products

- Journal Paper (submitted):
 - “Effect of High-Strength Materials on Lateral Strength of Shear-Critical Reinforced Concrete Walls,” *ACI Structural Journal*.
- Presentations:
 - Presentation, 2015 Fall ACI Convention, Denver, CO.
 - Poster, 2015 Energy Week, Center for Sustainable Energy, University of Notre Dame, Notre Dame, IN.
 - Presentation, 2016 Fall ACI Convention, Philadelphia, PA.
 - Presentation, 2016 American Nuclear Society Winter Meeting and Nuclear Technology Expo, Las Vegas, NV.



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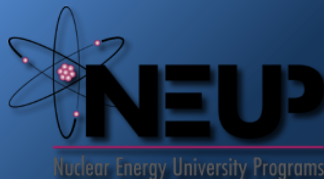


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- Material/Fabrication Donations:
 - MMFX Steel
 - Dayton Superior Corp.
 - HRC, Inc.
 - Sika Corp. U.S.



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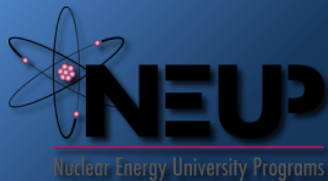
Questions?

<http://phsrc-nuclearwalls.nd.edu>

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